



VILLAGE OF WILLIAMS BAY

250 Williams Street | PO Box 580 | Williams Bay | WI | 53191 | vi.williamsbay.wi.gov

Phone: 262-245-2700

NOTICE

VILLAGE BOARD OF TRUSTEES MEETING

MONDAY, MAY 19, 2025 AT 6:30 PM

Village Hall Council Room

250 Williams Street

Williams Bay, WI 53191

AGENDA

The following agenda items may be considered for Discussion, Consideration, or Action

I. Call to Order

II. Roll Call

III. Pledge of Allegiance

IV. Meeting Decorum

- A.** Meeting Decorum. *According to Robert's Rules of Order, meeting decorum means maintaining respectful and orderly behavior during a discussion, including addressing the chair to speak, staying relevant to the topic at hand, avoiding personal attacks, and generally adhering to courteous conduct while debating issues; essentially, ensuring that discussions are focused and civil, with only one person speaking at a time when recognized by the chair. Violators of meeting decorum will be excused from the meeting.*

V. Minutes

- A.** Village Board Meeting Minutes of May 5, 2025

VI. Consent Agenda

- A.** Resolution R-32-25 Denying Use of Golf Cart as Low Speed Vehicle
- B.** Resolution R-33-25 Establishing A Policy On Rural Type Mailboxes
- C.** Resolution R-34-25 Authorizing A Memorandum of Understanding Between the Walworth County Sheriff's Office and Village of Williams Bay for Storm Siren Upgrades

VII. Presentation of accounts and petitions

- A.** Payroll ending 05-02-2025 in the amount of \$55,973.13
- B.** Accounts Payable Prepays
1. dated 05/06/2025 in the amount of \$30.00
 2. dated 05/07/2025 in the amount of \$150.00
- C.** Accounts Payable Unpays dated 05-16-2025 in the amount of \$81,744.06
- D.** Library Accounts Payable Unpays dated 05-15-2025 in the amount of \$2,654.44
- E.** March 2025 Financial Statement

VIII. Plan Commission

- A.** APPLICANT: Dancing Dudes, LLC (Owner)
TAX KEYS: WOP 00088
STREET ADDRESS: 77 N. Walworth Ave, Williams Bay, WI 53191

APPLICATION. The applicant requests a recommendation to the Village Board for approval for architectural design alteration to remove the window shutters and to have black metal awnings on the upper level and black cloth awnings on the lower level.

IX. Public Comments

- A.** Public Comments Responses - *Responses to Public Comments from the previous Village Board Meeting are included in the Meeting Packet Materials Under this Agenda Item*
- B.** *Individual public comments will be limited to two (2) minutes maximum. If your comments align with other comments before you, please consider stating that you are in agreement with those comments. The public may speak on any item that is not included on this agenda as a "Public Hearing or Forum." Anyone who wishes to comment should identify himself or herself and provide his or her local address. Board members may discuss any matter raised by the public. However, the Board will refrain from extensive dialogue and should not take action on matters raised by the public during a public comment period. Referrals to committees or staff members might be made. Members may ask the commenter to clarify his or her remarks. The Board President or the presiding officer will maintain control of the meeting.*

X. President's Remarks

XI. Ordinances and Resolutions

- A.** Ordinance 2025-05 Repealing Section 281-12 of the Code of Ordinances of the Village of Williams Bay Regarding Launch Area Use (*Second Reading and Adoption*)

XII. Committee Reports

- A. Protective Services, Chair - Trustee Vlach**

- 1. Original, Renewal, or Temporary Operator License Applications

XIII. Public Comments

- A.** *Individual public comments will be limited to two (2) minutes maximum. If your comments align with other comments before you, please consider stating that you are in agreement with those comments. The public may speak on any item that is not included on this agenda as a "Public Hearing or Forum." Anyone who wishes to comment should identify himself or herself and provide his or her local address. Board members may discuss any matter raised by the public. However, the Board will refrain from extensive dialogue and should not take action on matters raised by the public during a public comment period. Referrals to committees or staff members might be made. Members may ask the commenter to clarify his or her remarks. The Board President or the presiding officer will maintain control of the meeting.*

XIV. Other Items for Discussion, Consideration, or Action

- A.** Emergency Medical Services Week Proclamation
- B.** PUBLIC HEARING. Motion to Open Public Hearing Re Hanson's Addition Vacation of Right of Way - East Half of Hanson Street and All of 16.5 Foot Alley Shown on Hanson's Addition - All Interested Parties are Invited to Attend
- C.** APPLICANT: Steve Greenberg, SG Investments, LLC (Owner).
TAX KEYS: WHA00003; WHA 00002; WHA 00001; WHA00004; WHA00005; WHA00006
STREET ADDRESS: North Side of E. Geneva St, Between Willabay Drive and Potawatomi Street,

Williams Bay, WI 53191

APPLICATION: Right of Way Vacation for a lot line adjustment, to decrease the number of lots on the Land from fourteen (14) to five (5) and to develop those lots for single-family homes

- D. Motion to Close Public Hearing
- E. Consideration and Possible Action on Resolution R-35-25 Final Resolution Vacating the East Half of Hanson Street and All of 16.5 Foot Alley Shown on Hanson's Addition to Williams Bay

XV. Adjournment

Requests from persons with disabilities, who need assistance to participate in this meeting or hearing, should be made to the Village Clerk's office in advance so the appropriate accommodations can be made.

Posted: 05/16/2025 5:00 PM



VILLAGE OF WILLIAMS BAY

250 Williams Street | PO Box 580 | Williams Bay | WI | 53191 | vi.williamsbay.wi.gov

Phone: 262-245-2700

UNOFFICIAL MINUTES VILLAGE BOARD MEETING 5/5/2025 MEETING MONDAY, MAY 5, 2025 AT 6:30 PM VILLAGE HALL COUNCIL ROOM 250 WILLIAMS STREET WILLIAMS BAY, WI 53191

I. Call to Order

President Jaramillo called the meeting to order at 06:30pm.

II. Roll Call

Present: President Jaramillo, Trustees Vlach, Wright, Umans, Russell, Bartholomew, Franzen

Also Present: Administrator Lothspeich, Police Chief Timm, Luutenant Kostock, Village Engineer Snyder, Williams Bay Fire Chief Smith, Public Works Director Edwards, Library Director Schnupp, Recreation Director Knop, Village Code Enforcement Schwark, Treasurer Wiese, Clerk Kolls

III. Pledge of Allegiance

The Pledge of Allegiance was recited by all.

IV. Meeting Decorum

- A. Meeting Decorum. *According to Robert's Rules of Order, meeting decorum means maintaining respectful and orderly behavior during a discussion, including addressing the chair to speak, staying relevant to the topic at hand, avoiding personal attacks, and generally adhering to courteous conduct while debating issues; essentially, ensuring that discussions are focused and civil, with only one person speaking at a time when recognized by the chair. Violators of meeting decorum will be excused from the meeting.*

V. Minutes

- A. Village Board Meeting Minutes of April 21, 2025

The motion to approve the Village Board Meeting Minutes of April 21, 2025 was initiated by Trustee Umans and seconded by Trustee Vlach. Unanimously carried.

VI. Presentation of accounts and petitions

- A. Payroll ending 04-18-2025 in the amount of \$51,499.11

The motion to approve the Payroll ending 04-18-2025 in the amount of \$51,499.11 was initiated by Trustee Wright and seconded by Trustee Vlach. Unanimously carried.

- B. Accounts Payable Unpays dated 05-01-2025 in the amount of \$526,442.28

The motion to approve the Accounts Payable Unpays dated 05-01-2025 in the amount of \$526,442.28 was initiated by Trustee Wright and seconded by Trustee Umans. Unanimously carried.

- C. Accounts Payable Prepays

- 1. dated 04-22-2025 in the amount of \$3,808.00

The motion to approve the accounts payable prepaid dated 04-22-2025 in the amount of \$3,808.00 was initiated by Trustee Wright and seconded by Trustee Umans. Unanimously carried.

2. dated 4-25-2025 in the amount of \$15, 540.31

The motion to approve the Accounts payable prepaids dated 4-25-2025 in the amount of \$15, 540.31 was initiated by Trustee Wright and seconded by Trustee Umans. Unanimously carried.

VII. Plan Commission

- A.** RECONSIDERATION OF VILLAGE BOARD ACTION TAKEN ON APRIL 21, 2025
APPLICANT: Dancing Dudes, LLC (Owner), Attorney Christina Green (Agent)
TAX KEYS: WOP 00088

STREET ADDRESS: 77 N. Walworth Ave, Williams Bay, WI 53191

Applicant requests a recommendation to the Village Board for approval of revised awning signage. The applicant also requests approval for an architectural design alteration to remove the upper floor awnings.

The motion to rescind the motion made at the previous Village Board meeting April 21, 2025 was initiated by President Jaramillo and seconded by Trustee Russell. Unanimously Carried.

The motion to send this item back to the Plan Commission for review was initiated by President Jaramillo and seconded by Trustee Bartholomew.

Votes:

Yes: President Jaramillo, Trustees Bartholomew, Umans, Wright, Vlach, Russell, Franzen

No: None

Abstain: None

Result: Passes

VIII. Public Comments

Pamela Russell, 24 Lower Loch Vista, Spoke regarding a dangerous bridge on the shore path near the Lake Geneva Youth Camp

- A.** *Individual public comments will be limited to two (2) minutes maximum. If your comments align with other comments before you, please consider stating that you are in agreement with those comments. The public may speak on any item that is not included on this agenda as a "Public Hearing or Forum." Anyone who wishes to comment should identify himself or herself and provide his or her local address. Board members may discuss any matter raised by the public. However, the Board will refrain from extensive dialogue and should not take action on matters raised by the public during a public comment period. Referrals to committees or staff members might be made. Members may ask the commenter to clarify his or her remarks. The Board President or the presiding officer will maintain control of the meeting.*

IX. President's Remarks

- A.** Arbor Day tree plantings

President Jaramillo spoke regarding the Arbor Day Tree Plantings at Kishwaukee and the Lions Park.

- B.** Congratulations to all involved on a successful Truck-a-palooza

President Jaramillo gave congratulations to all involved with a successful Truck-a-palooza explaining that there were over 1,000 in attendance.

- C.** Congratulations to WBBA and all involved on the Cinco de Mayo event

President Jaramillo gave congratulations to the WBBA on a successful Cinco de Mayo event.

- D. Request to Schedule Joint Village of Fontana and Village of Williams Bay Board Meeting Re: EMS Services Agreement

President Jaramillo wanted to inform the Village Trustees and residents that there was a request to schedule a Joint Village of Fontana and Village of Williams Bay Board Meeting regarding EMS Services Agreement.

X. Ordinances and Resolutions

- A. An Ordinance Rescinding Village Ordinance Chapter 281 Public Waters and Beaches Article I General Regulations 281-12 Re Commercial Use Launch Area.

The motion to approve the Ordinance Repealing Village Ordinance Chapter 281 Public Waters and Beaches Article I General Regulations 281-12 Re Commercial Use Launch Area was initiated by Committee, no second required. Unanimously carried.

XI. Committee Reports

- A. Protective Services, Chair - Trustee Vlach

- 1. Original, Renewal, or Temporary Operator License Applications

The motion to approve the Original, Renewal, or Temporary Operator License Applications was initiated by Trustee Vlach and seconded by Trustee Umans. Unanimously carried.

XII. Public Comments

Connie Gluth, 95 Lackey, spoke regarding traffic problems near the boat launch. Gluth gave several suggestions, such as a staging area where they could telephone in to check congestion or a buzzer system letting them know it was their turn.

Scott Christian, Thorpe & Christian Attorneys At Law, spoke in opposition to the Pier Permit request for 100 Birch Walnut on behalf of Cedar Point.

Tim Halas, 134 Elmhurst Ct, spoke regarding the need to keep tight control on Topography, and requested that we request a calander of events and major milestones from them to help with this.

- A. *Individual public comments will be limited to two (2) minutes maximum. If your comments align with other comments before you, please consider stating that you are in agreement with those comments. The public may speak on any item that is not included on this agenda as a "Public Hearing or Forum." Anyone who wishes to comment should identify himself or herself and provide his or her local address. Board members may discuss any matter raised by the public. However, the Board will refrain from extensive dialogue and should not take action on matters raised by the public during a public comment period. Referrals to committees or staff members might be made. Members may ask the commenter to clarify his or her remarks. The Board President or the presiding officer will maintain control of the meeting.*

XIII. Other Items for Discussion, Consideration, or Action

- A. 50th Anniversary of the Williams Bay Fine Art Fest - Sandra Johnson

Sandra Johnson spoke regarding the Williams Bay Fine Art Fest and all they do for the community. Johnson plans to step down and is asking for someone to step up.

- B. Police Appreciation Proclamation

President Jaramillo read the Police Appreciation Proclamation.

- C. Clerk's Week Proclamation

President Jaramillo read the Clerk's Week Proclamation.

- D. Resolution Recognizing Harold Friestad as Chairman Emeritus of Kishwauketoe Nature Conservancy

President Jaramillo moved this item to just after section V. Minutes.

The motion to approve the Resolution Recognizing Harold Friestad as Chairman Emeritus of Kishwauketoe Nature

Conservancy was initiated by Trustee Vlach and seconded by Trustee Franzen. Unanimously carried.

Village President Jaramillo and Kishwaukee Chair Skalla recognized Harold Friestad as Chairman Emeritus of Kishwaukee Nature Conservancy.

- E.** Approval of Zoning Administration and Land Division Services Contract Between the Village of Williams Bay and Municipal Code Enforcement, LLC

The motion to approve the Administration and Land Division Services Contract Between the Village of Williams Bay and Municipal Code Enforcement, LLC was initiated by Trustee Wright and seconded by Trustee Franzen. Unanimously carried.

- F.** Recommendation of the Appointment of Municipal Code Enforcement, LLC as the Village Zoning Administrator for a One (1) Year Term.

The motion to approve the Appointment of Municipal Code Enforcement, LLC as the Village Zoning Administrator for a One (1) Year Term was initiated by President Jaramillo and seconded by Trustee Umans. Unanimously carried.

- G.** Recommendation of the Appointment of Halina Marra as a Citizen Member to the Parks and Memorial Advisory Committee for a Three (3) Year Term

The motion to approve the Appointment of Halina Marra as a Citizen Member to the Parks and Memorial Advisory Committee for a Three (3) Year Term was initiated by President Jaramillo and seconded by Trustee Umans. Unanimously carried.

- H.** Consideration and Possible Action on Pier Permit for 254 Circle Pkwy

The motion to approve the Pier Permit for 254 Circle Pkwy was initiated by President Jaramillo and seconded by Trustee Russell. Unanimously carried.

Votes:

Yes: President Jaramillo, Trustees Bartholomew, Umans, Wright, Vlach, Russell, Franzen

No: None

Abstain: None

Result: Passes

The motion to approve the pier permit for 254 Circle Pkwy was amended to include that it is contingent upon the receipt of an as-built survey which must be provided within 90 days of the completion of the installation of the pier was initiated by Trustee Wright and seconded by Trustee Russell. Unanimously Carried.

- I.** Consideration and Possible Action on Pier Permit for 100 Birch Walnut

The motion to approve the Pier Permit for 100 Birch Walnut contingent upon the receipt of an as-built survey which must be provided within 90 days of the completion of the installation of the pier was initiated by Trustee Russell and seconded by Trustee Vlach.

Votes:

Yes: President Jaramillo, Trustees Bartholomew, Umans, Wright, Vlach, Russell, and Franzen

No: None

Abstain: None

Result: Passes

XIV. Adjournment

The motion to adjourn was initiated by Trustee Umans and seconded by Trustee Wright at 07:25pm. Unanimously carried.

/s/ Tina Kolls, Village Clerk

**VILLAGE OF WILLIAMS BAY
RESOLUTION NO. R-32-25**

A RESOLUTION DENYING REQUEST TO OPERATE GOLF CART ON VILLAGE STREETS

WHEREAS, the Village of Williams Bay (the “Village”) is a Wisconsin municipality; and

WHEREAS, Wisconsin State Law and Village Code permit low-speed vehicles (LSV) to operate on roads with speed limits of 35 mph or less; and

WHEREAS, under Wisconsin State Law A LSV must meet the National Highway Traffic Safety Administration's (NHTSA) Federal Motor Vehicle Safety Standards, with a maximum speed of 25 mph, and must be titled, registered, and insured, and

WHEREAS, the Wisconsin Department of Transportation has determined that “*Golf carts, all-terrain vehicles, and utility terrain vehicles are not considered LSVs because these vehicles are not designed and manufactured for on-road use, and because they lack the equipment and certification needed to comply with federal motor vehicle safety standards, they cannot be registered for use on public roads in Wisconsin*”; and

WHEREAS, Williams Bay Resident Thomas F. Wachs submitted a request for permission to operate a golf cart on Village streets; and

WHEREAS, in furtherance of its duty to manage facilities of the Village responsibly and protect life and safety on May 5, 2025, the Village Streets & Highway Committee recommended denial of Mr. Wachs request to operate a golf cart on Village streets.

WHEREAS, on May 19, 2025, the Village of Williams Village Board, as recommended by the Streets & Highways Committee, has determined that it is in the best interests of the Village and its residents to deny Mr. Wach’s request.

NOW, THEREFORE, the Village Board of the Village of Williams Bay, County of Walworth, State of Wisconsin, do hereby ordain as follows:

Section I: Recitals. The foregoing recitals are hereby incorporated herein as findings of the Village Board of Trustees.

Section II: Denial. The Board of Trustees hereby denies the request by Mr. Wachs to operate a golf cart on Village streets, therefore.
Approved by the Village Board of the Village of Williams Bay this 19th day of May 2025.

VILLAGE OF WILLIAMS BAY

By: _____
Adam Jaramillo, President

Attest: _____
Tina Kolls, Clerk



Outlook

Fw: Form submission from: Comments

From Justin Timm <chief@vi.williamsbay.wi.gov>

Date Wed 4/23/2025 3:05 PM

To Tina Kolls <clerk@vi.williamsbay.wi.gov>

Ms. Kolls,

The village resident, Thomas Wachs would like to have this brought up to the board for a possible variance on the LSV/ golf cart ordinance.

Would we be able to get this on an agenda in the near future?

Respectfully,

Justin P Timm CPM

Chief of Police

Williams Bay Police Department

262.245.2710 (office)

262.581.3061 (mobile)

SMIP Session 75

FBI Trilogy Recipient

WCELEA President

From: Cynthia Stanek <Assistant@vi.williamsbay.wi.gov>

Sent: Monday, April 21, 2025 08:20

To: Justin Timm <chief@vi.williamsbay.wi.gov>

Subject: Fw: Form submission from: Comments

Chief Timm,

Please see the message below.

Warm regards,

Cynthia Stanek

Utility Clerk/ Administrative Assistant

262-245-2700 Option 1

Please note my email address has changed to Assistant@vi.williamsbay.wi.gov

From: Williams Bay, WI via Williams Bay, WI <cmsmailer@civicplus.com>

Sent: Saturday, April 19, 2025 9:45 AM

To: Cynthia Stanek <Assistant@vi.williamsbay.wi.gov>

Subject: Form submission from: Comments

Submitted on Saturday, April 19, 2025 - 9:45am

Submitted by anonymous user: 172.15.89.124

Submitted values are:

==Your Message Details:==

Subject: Use of Golf Cart by a Handicapped Resident

Message: In my infinite wisdom, I bought a maxi scooter upstate, and when I headed home, I almost made it, and dumped it on the corner of Cherry and Hill last July. My wife had banned me from buying any motorcycles, but I didn't listen, and I have been parked with a cast and now a brace, and would like to get something tamer, like a golf cart, I found the following on line...In Wisconsin, a handicapped person may be able to operate a golf cart on city streets, but it depends on local ordinances and specific regulations. Generally, individuals with a valid handicapped driver's certificate and a valid driver's license may be exempt from fees or other requirements for operating a golf cart on city streets. However, it's crucial to check local ordinances in Williams Bay, as they may have specific rules regarding golf cart operation, including restrictions on sidewalks, parks, and grassy areas... So, I do understand that it would be restricted usage, but it will allow me to go to Willie's to get a burger, get my mail at the PO, see neighborhood friends without having my wife drive me everywhere, When I am able I could use my or my wife's car both carrying handicapped Plates, but the use of a golf cart would be much simpler.

==Your Contact Information:==

Full Name: Thomas F Wachs

E-mail Address: cheddarkopf@gmail.com

Phone Number: 9282829775

==Address:==

Street: 363 Forest Drive

City: Williams Bay

State: Wisconsin

Zipcode: 53191

The results of this submission may be viewed at:

<https://www.williamsbay.org/node/2/submission/14826>

**VILLAGE OF WILLIAMS BAY
RESOLUTION NO. R-33-25**

A RESOLUTION ESTABLISHING A RURAL TYPE MAILBOX REPLACEMENT POLICY

WHEREAS, the Village of Williams Bay (the “Village”) is a Wisconsin municipality; and

WHEREAS, in furtherance of its duty to manage facilities of the Village responsibly; and

WHEREAS, on May 2, 2025, the Village Streets & Highways Committee recommended approval for the proposed Rural Type Mailbox Replacement Policy; and

WHEREAS, on May 19, 2025, the Village of Williams Village Board, as recommended by the Streets & Highways Committee, has determined that it is in the best interests of the Village and its residents to approve the Rural Type Mailbox Replacement Policy as set forth in Exhibit A to this Resolution.

NOW, THEREFORE, the Village Board of the Village of Williams Bay, County of Walworth, State of Wisconsin, do hereby ordain as follows:

Section I: Recitals. The foregoing recitals are hereby incorporated herein as findings of the Village Board of Trustees.

Section II: Approval. The Board of Trustees hereby approves the recommended Rural Type Mailbox Replacement Policy, therefore.

Approved by the Village Board of the Village of Williams Bay this 19th day of May 2025.

VILLAGE OF WILLIAMS BAY

By: _____
Adam Jaramillo, President

Attest: _____
Tina Kolls, Clerk

EXHIBIT A

VILLAGE RURAL TYPE MAILBOX POLICY

DRAFT



VILLAGE OF WILLIAMS BAY

Policy On Rural Type Mailboxes

Rural type mailboxes have been and are allowed within the road right-of way in the Village of Williams Bay as a matter of convenience and practicality to the property owners or occupants and not as a matter of right.

Encroachments upon the road right-of-way are governed by Section 86.04 of the Wisconsin Statutes which states in part as follows:

86.04 (1) ORDER FOR REMOVAL. If any highway right-of-way shall be encroached upon, under or over by any fence, stand, building or any other structure or object, and including encroachments caused by acquisition by the public of new or increased widths of highway right-of-way, the department, in case of a state trunk highway, the county highway committee, in case of a county trunk highway, or the city council, village or town board, in case of a street or highway maintained by or under the authority of any city, village or town, may order the occupant or owner of the land through or by which the highway runs, and to which the encroachment shall be appurtenant, to remove the encroachment beyond the limits of the highway within 30 days. The order shall specify the extent and location of the encroachment with reasonable certainty and shall be served upon the occupant or owner of the land through or by which the highway runs, and to which the encroachment shall be appurtenant.

Thus, it is the intention of this policy that if a mailbox is accidentally damaged in the process of any Village roadway or utility operations or maintenance, it is the property owner or occupants' obligation to repair or replace the mailbox. The property owner or occupant is not entitled to replacement or compensation.

It is further the intention that the above policy shall also be applicable to tubes or boxes for newspaper delivery and such tubes or boxes should be located adjacent to the mailbox.

**VILLAGE OF WILLIAMS BAY
RESOLUTION NO. R-34-25**

**A RESOLUTION AUTHORIZING MEMORANDUM OF UNDERSTANDING WITH WALWORTH
COUNTY SHERIFF RE: STORM SIRENS UPGRADES**

WHEREAS, the Village of Williams Bay (the “Village”) is a Wisconsin municipality; and

WHEREAS, in furtherance of its duty to manage facilities of the Village responsibly and protect life and safety; and

WHEREAS, on May 5, 2025, the Village Protective Services Committee and Finance & Personnel Committee recommended approval for the proposed Memorandum (MOU) between the Walworth County Sheriff’s Department and the Village of Williams Bay (among other Walworth County municipalities); and

WHEREAS, on May 19, 2025, the Village of Williams Village Board, as recommended by the Streets & Highways Committee, has determined that it is in the best interests of the Village and its residents to approve the Memorandum of Understanding with the Walworth County Sheriff: re: Storm Sirens Upgrades as set forth in Exhibit A to this Resolution.

NOW, THEREFORE, the Village Board of the Village of Williams Bay, County of Walworth, State of Wisconsin, do hereby ordain as follows:

Section I: Recitals. The foregoing recitals are hereby incorporated herein as findings of the Village Board of Trustees.

Section II: Approval. The Board of Trustees hereby authorizes the MOU, therefore.

Approved by the Village Board of the Village of Williams Bay this 19th day of May 2025.

VILLAGE OF WILLIAMS BAY

By: _____
Adam Jaramillo, President

Attest: _____
Tina Kolls, Clerk

EXHIBIT A

**MEMORANDUM OF UNDERSTANDING WITH WALWORTH COUNTY SHERIFF
RE: STORM SIRENS UPGRADES**

DRAFT

Memorandum of Understanding Between the Walworth County Sheriff's Office and Walworth County Entities for Storm Siren Upgrades

PARTICIPANTS

This Memorandum of Understanding (hereinafter referred to as "MOU") involves the Walworth County Sheriff's Office (hereinafter referred to as "WCSO") and all Walworth County municipalities/entities (hereinafter referred to as "the Entities") who currently have storm siren sites that need to be upgraded. The specific fifteen entities subject to this agreement include:

- Village of Darien
- Town of Delavan
- City of Delavan
- Village of East Troy
- City of Elkhorn
- Village of Fontana
- City of Lake Geneva
- Village of Genoa City
- Town of Linn
- Country Estates Association
- Town of Geneva
- Village of Sharon
- City of Whitewater
- Village of Walworth
- Village of Williams Bay

PROJECT PURPOSE AND KEY DETAILS

The purpose of this MOU is to set guidelines related to funding and facilitating an upgrade to emergency storm sirens throughout Walworth County and to clarify future responsibilities for siren sites post-upgrade.

The Walworth County Sheriff's Office has funds available as part of the 911 Center Radio System Replacement Project (hereinafter "the Radio System Project"). The Radio System Project will enhance interoperability and continuity of operations between police, fire, and emergency medical services in Walworth County. The project will enhance overall safety.

The storm siren upgrade will implement the use of the same technology countywide to make all Walworth County sirens compatible with the existing warning systems in use by Delavan, Lake Geneva, and Whitewater. The system queries every siren in the county daily. The query asks about battery health, AC power, and provides a report. The same technology then communicates with the National Weather Service when warnings are issued. When warnings are received, the system controller determines which sirens fall within the areas to be warned and activates those sirens. This is an automated siren activation. This siren upgrade will enhance repair identification, ensure correct siren activation in designated areas, and improve the overall system and community safety.

WCSO proposes to pay the cost of outfitting the sirens with the compatible equipment. To finance the upgrade, WCSO has \$452,198.77 available. The cost per siren site is approximately \$7,300.00. The Entities have no financial responsibility for the storm siren upgrade. Through this Memorandum of Understanding, WCSO proposes to facilitate and fund the improvement of 59 emergency siren sites and 3 base stations in Walworth County, as well as to upgrade the central control system and backup system for the sirens. For this project, WCSO will facilitate, manage, and fund the countywide emergency siren site updates, central control system and backup system updates. During the upgrade, the Entities will maintain any insurance coverage related to their respective siren sites.

Siren Services, the company who services the Federal Signal Commander Systems in Lake Geneva, Delavan, and Whitewater, will be the company conducting the upgrade. Lieutenant Jason Rowland (hereinafter "Lt. Rowland"), from the Walworth County Sheriff's Office, is the project coordinator for the storm siren project. Lt. Rowland will manage the storm siren upgrade and will be the primary point of contact for Siren Services and will also be the point of contact for the Entities during the upgrade.

After the storm siren upgrade is complete, the Entities will resume all responsibility for maintenance, communication, and improvements to the siren sites located in their respective jurisdictions.

AGREEMENT PERIOD

The term of the agreement for this MOU shall begin when the upgrade of the shared siren system starts in the Spring of 2025 and will end upon completion of the installation of the upgrades to the siren sites, central control system, and backup system. Once the upgrade is complete, WCSO will no longer provide any funding for the siren sites, central control system or backup system, or any other components, nor will WCSO be responsible for any maintenance or service fees, nor will WCSO be responsible for any future communication or contracts regarding the Entities siren sites, unless a future agreement is entered into.

PARTY INDEPENDENCE

Each party to this MOU is a separate and independent entity. As such, each party retains its own identity in providing services, and each entity is responsible for establishing its own policies and financing its own projects and activities beyond this agreement.

FUTURE RESPONSIBILITY FOR MAINTENANCE

Following the completion of the countywide siren upgrade pursuant to this MOU, each entity will resume all responsibility for their own sirens, as well as any other prior obligations related to the sirens. Resuming responsibility for the sirens, siren sites, and systems includes any operation, maintenance, communication, and financial responsibility.

The current yearly recurring cost for maintaining the overall system will be \$16,411.00. Each of the sites will therefore be responsible for approximately \$280.00 per site for yearly maintenance and subscription costs. Yearly maintenance costs will be billed directly to the municipality/entity by Siren Services. Following the completion of the term of this MOU, WCSO will not be involved in anyway in the payment of yearly maintenance costs and all communications regarding the siren sites will be directly between the Entities and Siren Services.

COOPERATION

Each of the above listed Entities will cooperate with Lt. Rowland, Siren Services, and any other person who needs information or access to the siren system equipment and components to facilitate the storm siren upgrade.

Lt. Rowland will inform the Entities upon completion of the storm siren upgrade. Siren Services will notify the Entities when they will be responsible for payment of the yearly maintenance and subscription costs.

INDEMNIFICATION

The Entities are responsible for the insurance, operation, maintenance, and costs of their sirens, siren sites, and any equipment beyond the improvements proposed by this agreement. The Entities agree to fully indemnify and hold harmless Walworth County and the WCSO from all claims related to the siren sites and the siren site upgrades during the term of the MOU.

AMENDMENTS

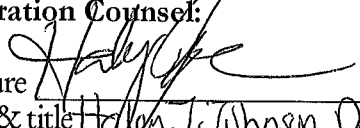
This agreement may be supplemented, amended, or revised in writing by agreement of the parties.

Dated this _____ of _____, 2025

Representative from Walworth County

Signature _____
Name & title _____
Date _____

Approved as to form by Walworth County Corporation Counsel:

Signature 
Name & title Helen J. Johnson, Deputy
Date 4/4/2025 Corporation
Counsel

Representative from Village of Darien

Signature _____
Name & title _____
Date _____

Representative from Village of Genoa City

Signature _____
Name & title _____
Date _____

Representative from Town of Linn

Signature _____
Name & title _____
Date _____

Representative from Country Estates Association

Signature _____
Name & title _____
Date _____

Representative from Town of Delavan

Signature _____
Name & title _____
Date _____

Representative from City of Delavan

Signature _____
Name & title _____
Date _____

Representative from Village of East Troy

Signature _____
Name & title _____
Date _____

Representative from City of Elkhorn

Signature _____
Name & title _____
Date _____

Representative from Village of Fontana

Signature _____
Name & title _____
Date _____

Representative from City of Lake Geneva

Signature _____
Name & title _____
Date _____

Representative from Town of Geneva

Signature _____
Name & title _____
Date _____

Representative from Village of Sharon

Signature _____
Name & title _____
Date _____

Representative from City of Whitewater

Signature _____
Name & title _____
Date _____

Representative from Village of Walworth

Signature _____
Name & title _____
Date _____

Representative from Village of Williams Bay

Signature _____
Name & title _____
Date _____

04/19/2025 - 05/02/20205

Pay Date:

5/9/2025

Department	Gross Regular Wages	Gross OT Wages	Payroll Taxes & Deductions	Net Wages
Crossing Guard	\$824.85	\$0.00	\$69.20	\$755.65
General Administration	\$12,292.06	\$0.00	\$5,823.65	\$6,468.41
KNC	\$3,180.08	\$0.00	\$874.89	\$2,305.19
Lakefront/Beach	\$2,306.24	\$79.92	\$474.81	\$1,911.35
Library	\$7,581.56	\$0.00	\$2,986.47	\$4,595.09
Municipal Court	\$1,166.08	\$0.00	\$150.58	\$1,015.50
Parks	\$1,426.36	\$0.00	\$161.23	\$1,265.13
Police	\$29,481.94	\$2,489.83	\$10,534.81	\$21,436.96
Protective Services (F&R)	\$6,630.53	\$0.00	\$840.43	\$5,790.10
Public Works/W&S	\$12,692.58	\$106.20	\$5,037.99	\$7,760.79
Recreation Department	\$3,861.54	\$0.00	\$1,192.58	\$2,668.96
Village Board	\$0.00	\$0.00	\$0.00	\$0.00
Total Net Wages				\$55,973.13

GL Account and Title	Description	Amount	Date Paid	GL Period
WALWORTH COUNTY REGISTER OF DEEDS				
100-51670-000 MISC MUNI SERVICES COST RECOV	RECORDING DRAINAGE	30.00		525
Total WALWORTH COUNTY REGISTER OF DEEDS:		30.00		
Grand Totals:		30.00		

Village Board Approval Date: _____

GL Account and Title	Description	Amount	Date Paid	GL Period
PETTY CASH				
100-44622 LAUNCH REVENUE	CHANGE BAG FFOR LAU	150.00		525
Total PETTY CASH:		150.00		
Grand Totals:		150.00		

Village Board Approval Date: _____

GL Account and Title	Description	Amount	GL Period
425			
ABSOLUTE MOBILE TESTING LLC			
200-57930-130 WATER DRUG TESTING EXPENSE	RANDOM DRUG TESTING	111.00	425
Total ABSOLUTE MOBILE TESTING LLC:		111.00	
ADAMS PUBLISHING GROUP			
100-51400-000 RECRUITING FEES	BEACH AND LAUNCH ATTENDA	1,676.00	425
Total ADAMS PUBLISHING GROUP:		1,676.00	
BANDT COMMUNICATIONS INC.			
100-52360-150 RESCUE DEPT REPAIRS/MAINT	AMBULANCE RADIO REPAIR	312.50	425
Total BANDT COMMUNICATIONS INC.:		312.50	
BROOKS TRACTOR			
100-54310-150 STREETS EQUIP REPAIRS/MAINT	PARTS/FILTERS	208.00	425
Total BROOKS TRACTOR:		208.00	
BURLINGTON WASTEWATER UTILITY			
200-57623-170 WATER TESTING	3 DIST SAMPLES	120.00	425
Total BURLINGTON WASTEWATER UTILITY:		120.00	
CITY OF LAKE GENEVA			
100-51412-000 ELECTION EXPENSE	ELECTION PUBLICATION PUBLI	9.03	425
Total CITY OF LAKE GENEVA:		9.03	
COLUMN SOFTWARE PBC			
100-51410-210 GEN ADMIN PUBLICATIONS	TOPOGRAPHY LIQUOR LICENS	39.77	425
100-51670-000 MISC MUNI SERVICES COST RECOV	PETITION TO VACATE HANSON	208.64	425
Total COLUMN SOFTWARE PBC:		248.41	
CORE & MAIN			
200-57346-100 METER REPLACEMENT PROGRAM	WATER METER SUPPLIES	13,615.64	425
Total CORE & MAIN:		13,615.64	
EAGLE EYE TREE SERVICE			
100-56130-000 TREE ENHANCEMENT	STUMP GRINDING	932.50	425
Total EAGLE EYE TREE SERVICE:		932.50	
ELKHORN NAPA AUTO PARTS			
100-52120-150 POLICE REPAIRS/MAINT	BLOWER MOTOR FOR SRO VE	67.53	425
100-52120-150 POLICE REPAIRS/MAINT	BLOWER MOTOR RESISTOR F	89.42	425
Total ELKHORN NAPA AUTO PARTS:		156.95	
HALVERSON DOOR LLC			
100-54310-150 STREETS EQUIP REPAIRS/MAINT	SHOP DOOR SPRING REPAIR	787.06	425
Total HALVERSON DOOR LLC:		787.06	

GL Account and Title	Description	Amount	GL Period
HEYER TRUE VALUE HARDWARE			
100-55410-150 PARKS REPAIRS/MAINT	SUPPLIES	32.72	425
Total HEYER TRUE VALUE HARDWARE:		32.72	
HYDRO CORP			
200-57656-150 CROSS CONNECTION INSPECTION	CROSS CONNECTION PROGRA	1,351.00	425
200-57656-150 CROSS CONNECTION INSPECTION	CROSS CONNECTION PROGRA	459.00	425
Total HYDRO CORP:		1,810.00	
MID-STATE EQUIPMENT			
500-54310-140 STREETS/HIGHWAYS	ASPHALT ROLLER	17,900.00	425
Total MID-STATE EQUIPMENT:		17,900.00	
PATS SERVICES INC			
200-57631-160 WATER TREATMENT CHEMICALS	LIME SLUDGE	200.00	425
200-57631-160 WATER TREATMENT CHEMICALS	LIME SLUDGE	200.00	425
200-57631-160 WATER TREATMENT CHEMICALS	LIME SLUDGE	250.00	425
200-57631-160 WATER TREATMENT CHEMICALS	LIME SLUDGE	250.00	425
200-57631-160 WATER TREATMENT CHEMICALS	LIME SLUDGE	250.00	425
Total PATS SERVICES INC:		1,150.00	
PREMIUM WATERS INC			
100-51730-160 VH BLDG SUPPLIES	DRINKING WATER VILLAGE HA	24.49	425
Total PREMIUM WATERS INC:		24.49	
SAFEBUILT LLC			
100-53100-211 BLDG INSPECTION CONTRACT	APRIL BUILDING PERMIT INPSE	11,960.53	425
Total SAFEBUILT LLC:		11,960.53	
WELDERS SUPPLY CO.			
100-52360-160 RESCUE DEPT SUPPLIES	OXYGEN	427.60	425
Total WELDERS SUPPLY CO.:		427.60	
WHITING, FRANK			
100-55210-275 REC DEPT PROGRAM EXPENSES	CHAIR YOGA 4/3/25	48.00	425
100-55210-275 REC DEPT PROGRAM EXPENSES	CHAIR YOGA 4/17/25	40.00	425
100-55210-275 REC DEPT PROGRAM EXPENSES	CHAIR YOGA 4/24/25	40.00	425
Total WHITING, FRANK:		128.00	
WILLIAMS BAY AUTOMOTIVE			
100-52120-150 POLICE REPAIRS/MAINT	OIL CHANGE & FILTER	95.79	425
100-52120-150 POLICE REPAIRS/MAINT	BRAKES 294	1,135.36	425
Total WILLIAMS BAY AUTOMOTIVE:		1,231.15	
WISCONSIN PROFESSIONAL POLICE			
100-21262 UNION DUES PAYABLE	MAY UNION DUES	274.20	425
Total WISCONSIN PROFESSIONAL POLICE:		274.20	

GL Account and Title	Description	Amount	GL Period
WISCONSIN STATE LABORATORY OF HYGIENE			
200-57623-170 WATER TESTING	FLUORIDE TEST	31.00	425
Total WISCONSIN STATE LABORATORY OF HYGIENE:		31.00	
Total 425:		53,146.78	
525			
A & A RESORATIONS			
100-55411-150 LAKEFRONT REPAIRS/MAINT	40% DOWN PIER PAINTING	1,994.00	525
Total A & A RESORATIONS:		1,994.00	
ACC PUBLISHING LLC			
100-55411-160 LAKEFRONT SUPPLIES	RESIDENT DECALS	193.29	525
Total ACC PUBLISHING LLC:		193.29	
ACCURATE APPRAISAL			
100-51520-000 ASSESSOR CONTRACT	ASSESSMENT SERVICES	2,500.00	525
Total ACCURATE APPRAISAL:		2,500.00	
AMAZON CAPITAL SERVICES			
200-57935-150 WATER PLANT REPAIRS/MAINT	TABLET HOLDER	23.95	525
200-57935-150 WATER PLANT REPAIRS/MAINT	GPS POLE	63.99	525
200-57641-160 WATER DISTRIBUTION SUPPLIES	HOSE BIB VACCUM BREAKER	209.60	525
200-57641-160 WATER DISTRIBUTION SUPPLIES	HOSE BIB VACCUM BREAKER	326.80	525
100-55411-150 LAKEFRONT REPAIRS/MAINT	LIFE RING AND BRACKET	139.98	525
200-57641-160 WATER DISTRIBUTION SUPPLIES	VACCUM BREAKER OUTDOOR	352.60	525
200-57641-160 WATER DISTRIBUTION SUPPLIES	VACCUM BREAKER OUTDOOR	8.60	525
100-55410-150 PARKS REPAIRS/MAINT	KOHLER URINAL GASKET	13.98	525
100-54310-150 STREETS EQUIP REPAIRS/MAINT	MILWAUKEE TOOL POWER CO	12.91	525
100-54310-150 STREETS EQUIP REPAIRS/MAINT	MILWAUKEE CORD PROTECTO	9.99	525
200-57930-140 WATER DIGGERS HOTLINE EXPENSE	PAINT SHAKER	36.95	525
100-54310-150 STREETS EQUIP REPAIRS/MAINT	SHIPPING AND HANDLING	5.79	525
100-51410-160 GEN ADMIN SUPPLIES	VILLAGE HALL OFFICE SUPPLI	142.69	525
100-52320-160 FIRE DEPT SUPPLIES	FIRE DEPT FANS	59.94	525
Total AMAZON CAPITAL SERVICES:		1,407.77	
AURORA UNIVERSITY			
001-15100 UTILITY CASH CLEARING	REFUND OVERPAYMENT	56.76	525
001-15100 UTILITY CASH CLEARING	REFUND OVERPAYMENT	5,681.75	525
001-15100 UTILITY CASH CLEARING	REFUND OVERPAYMENT	434.54	525
001-15100 UTILITY CASH CLEARING	REFUND OVERPAYMENT	434.54	525
Total AURORA UNIVERSITY:		6,607.59	
COMPOST MANAGEMENT			
100-55410-150 PARKS REPAIRS/MAINT	BROWN MULCH	94.50	525
100-55410-150 PARKS REPAIRS/MAINT	BROWN MULCH	94.50	525
100-55410-150 PARKS REPAIRS/MAINT	BROWN MULCH	94.50	525
Total COMPOST MANAGEMENT:		283.50	
DADDINO, CHRISTOPHER & NAHLAH			
001-15100 UTILITY CASH CLEARING	REFUND OVERPAYMENT	160.45	525

GL Account and Title	Description	Amount	GL Period
Total DADDINO, CHRISTOPHER & NAHLAH:		160.45	
DJS SCUBA LOCKER INC			
100-52340-145 DIVE TEAM TANK MAINTENANCE	HYDRO TEST, VIP & AIR FILL	165.00	525
Total DJS SCUBA LOCKER INC:		165.00	
DOMINION VOTING SYSTEMS			
100-51412-000 ELECTION EXPENSE	ICE ANNUAL FIRMWARE LICEN	241.89	525
100-51412-000 ELECTION EXPENSE	ICE ANNUAL HARDWARE WAR	249.31	525
Total DOMINION VOTING SYSTEMS:		491.20	
FASTENAL			
100-54310-150 STREETS EQUIP REPAIRS/MAINT	FLOW BOLTS AND NUTS	59.32	525
Total FASTENAL:		59.32	
FRIGO, DINA MARIE			
100-55210-275 REC DEPT PROGRAM EXPENSES	RECREATION INSTRUCTOR	179.13	525
Total FRIGO, DINA MARIE:		179.13	
GATEWAY TECHNICAL COLLEGE			
100-52320-190 FIRE DEPT TRAINING/CONFERENCES	FIRFIGHTER 1 TEST AND BOOK	279.75	525
Total GATEWAY TECHNICAL COLLEGE:		279.75	
GORDON FLESCH			
100-55210-130 REC DEPT TECHNOLOGY/IT	REC DEPT COPIER LEASE	48.46	525
100-51410-162 GEN ADMIN COPIER EXPENSE	VH COPIER LEASE	181.47	525
Total GORDON FLESCH:		229.93	
LAKESIDE INTERNATIONAL TRUCKS			
100-54310-150 STREETS EQUIP REPAIRS/MAINT	TRUCK BRAKE COMBO	305.90	525
Total LAKESIDE INTERNATIONAL TRUCKS:		305.90	
LASER ELECTRIC SUPPLY			
100-55411-150 LAKEFRONT REPAIRS/MAINT	PIER LIGHT PLUGS/ ELECTRIC	122.22	525
Total LASER ELECTRIC SUPPLY:		122.22	
LASER WORKS UNLIMITED			
100-52320-160 FIRE DEPT SUPPLIES	ACCOUNTABILITY TAGS	21.00	525
Total LASER WORKS UNLIMITED:		21.00	
LEE RECREATION LLC			
100-55410-150 PARKS REPAIRS/MAINT	PLAYGROUND PARTS	220.60	525
Total LEE RECREATION LLC:		220.60	
MUNICIPAL CODE ENFORCEMENT LLC			
100-53100-215 CODE ENFORCEMENT CONTRACT	CODE ENFORCEMENT APRIL 2	3,073.20	525

GL Account and Title	Description	Amount	GL Period
Total MUNICIPAL CODE ENFORCEMENT LLC:		3,073.20	
ODLING CONSTRUCTION INC.			
100-54310-175 STREETS ROAD MAINTENANCE	SIDEWALK REPAIRFROMN SIN	2,470.20	525
200-57651-150 WATER MAINS REPAIRS/MAINT	CURB REPAIR FROM WM BREA	2,470.20	525
Total ODLING CONSTRUCTION INC.:		4,940.40	
PENGUIN MANAGEMENT INC			
100-52320-160 FIRE DEPT SUPPLIES	E DISPATCH	972.00	525
Total PENGUIN MANAGEMENT INC:		972.00	
PETE'S TIRE ELKHORN LLC			
100-55410-150 PARKS REPAIRS/MAINT	NEW MOWER DECK TIRE	61.45	525
100-55410-150 PARKS REPAIRS/MAINT	NEW MOWER TIRE	211.99	525
Total PETE'S TIRE ELKHORN LLC:		273.44	
SCHAEFFER MUNICIPAL SERVICES LLC			
100-53100-210 ZONING INSPECTION CONTRACT	ZONING CONTRACT FEES	1,524.90	525
Total SCHAEFFER MUNICIPAL SERVICES LLC:		1,524.90	
SECURIAN FINANCIAL GROUP INC			
100-52120-124 POLICE LIFE INSURANCE	JUNE LIFE INS POLICE	101.22	525
100-54100-123 DPW ADMIN HEALTH INSURANCE	JUNE LIFE INS DPW	6.64	525
100-54310-124 STREETS LIFE INSURANCE	JUNE LIFE INS STREETS	17.47	525
100-55410-124 PARKS LIFE INSURANCE	JUNE LIFE INS PARKS	13.24	525
200-57630-124 WATER TREATMENT - LIFE INS	JUNE LIFE INS WATER TREATM	4.21	525
200-57640-124 WATER DISTRIBUTION - LIFE INS	JUNE LIFE INS ATER DIST	4.21	525
200-57902-124 WATER ACCOUNTING - LIFE INS	JUNE LIFE INS WATER ACCT	11.32	525
230-58100-124 KNC LIFE INSURANCE	JUNE LIFE INS KNC	5.00	525
200-57920-124 WATER ADMIN - LIFE INS	JUNE LIFE INS WATER ADMIN	12.33	525
300-58964-124 SEWER ACCOUNTING - LIFE INS	JUNE LIFE INS SEWER ACCT	11.29	525
300-58965-124 SEWER ADMIN - LIFE INS	JUNE LIFE INS SEWER ADMIN	12.32	525
100-51410-124 GEN ADMIN LIFE INSURANCE	JUNE LIFE INS GEN ADMIN	60.70	525
100-55210-124 REC DEPT LIFE INSURANCE	JUNE LIFE INS REC DEPT	6.05	525
400-58100-124 LIFE INSURANCE	JUNE LIFE INS LIBRARY	49.39	525
100-21257 LIFE INSURANCE PAYABLE	EMPLOYEE CONTRIBUTION	358.66	525
Total SECURIAN FINANCIAL GROUP INC:		674.05	
SYLVESTER-KNUDTSON, AMY			
100-55210-275 REC DEPT PROGRAM EXPENSES	APRIL INSTRUCTOR HOURS	324.14	525
Total SYLVESTER-KNUDTSON, AMY:		324.14	
WALWORTH COUNTY			
225-54635-160 RECYCLING PROGRAM SUPPLIES	2025 CLEAN SWEEEP DONATIO	500.00	525
Total WALWORTH COUNTY:		500.00	
WITTE SUPPLY COMPANY			
100-55410-150 PARKS REPAIRS/MAINT	PLAYGROUND MULCH	1,000.00	525
100-55410-150 PARKS REPAIRS/MAINT	TOPSOIL	94.50	525

GL Account and Title	Description	Amount	GL Period
Total WITTE SUPPLY COMPANY:		1,094.50	
Total 525:		28,597.28	
Grand Totals:		81,744.06	

Village Board Approval Date: _____

GL Account and Title	Description	Amount	GL Period
525			
BAKER & TAYLOR			
400-58200-000 ADULT PRINT	ADULT PRINT/ BOOK	922.38	525
400-58201-000 CHILDREN PRINT	CHILDREN'S BOOKS	200.85	525
410-58200-102 BOOKS, DONOR DESIGNATED	BOOKS DONOR DESIGNATED	290.24	525
Total BAKER & TAYLOR:		1,413.47	
CAROL PAUI			
400-58201-000 CHILDREN PRINT	CHILDREN'S PRINT	11.99	525
Total CAROL PAUI:		11.99	
DEMCO			
400-58100-160 LIBRARY SUPPLIES	SUPPLIES	105.98	525
Total DEMCO:		105.98	
FONTANA PUBLIC LIBRARY			
400-58200-000 ADULT PRINT	ADULT PRINT	31.00	525
Total FONTANA PUBLIC LIBRARY:		31.00	
HECKMAN, NANCY			
400-58232-000 LIBRARY CRAFTS	CRAFTS	50.57	525
Total HECKMAN, NANCY:		50.57	
MIDWEST TAPE			
400-58240-000 LIBRARY MEDIA	MEDIA	91.46	525
Total MIDWEST TAPE:		91.46	
MILWAUKEE JOURNAL SENTINEL			
410-58250-000 LIBRARY CHAPIN EXPENDITURES	CHAPIN	517.97	525
Total MILWAUKEE JOURNAL SENTINEL:		517.97	
SOUTHERN LAKES NEWSPAPERS LLC			
400-58280-000 LIBRARY PERIODICALS	PERIODICALS	58.00	525
Total SOUTHERN LAKES NEWSPAPERS LLC:		58.00	
UNIQUE SERVICES			
400-58310-000 IT/LICENSES/CONTRACTED SERVICE	IT/LICENSES/CONTRACTED SE	50.00	525
Total UNIQUE SERVICES:		50.00	
WALWORTH COUNTY SECURITY ALARMS			
400-58310-000 IT/LICENSES/CONTRACTED SERVICE	IT/LICENSES/CONTRACTED SE	324.00	525
Total WALWORTH COUNTY SECURITY ALARMS:		324.00	
Total 525:		2,654.44	
Grand Totals:		2,654.44	

GL Account and Title	Description	Amount	GL Period
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Village Board Approval Date: _____

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

GENERAL FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEARNED	PCNT
<u>TAXES</u>					
100-41100 TAX LEVY-GENERAL	1,192,093.84	1,192,093.84	1,793,544.00	601,450.16	66.5
100-41101 DELINQUENT PP TAX	.00	.00	50.00	50.00	.0
100-41104 UTILITY TAX EQUIVALENT	.00	.00	77,000.00	77,000.00	.0
100-41105 ROOM TAX	(150.00)	12,896.12	75,000.00	62,103.88	17.2
100-41106 CABLE FRANCHISE FEES	.00	13,303.07	51,000.00	37,696.93	26.1
100-41110 MERCY PILOT	.00	.00	56,000.00	56,000.00	.0
100-41111 WLC PILOT	.00	.00	37,500.00	37,500.00	.0
100-41200 COUNTY AND MUNICIPAL AID	.00	.00	30,600.00	30,600.00	.0
100-41201 SUPPLEMENTAL CTY & MUNICPL AID	.00	.00	82,000.00	82,000.00	.0
TOTAL TAXES	1,191,943.84	1,218,293.03	2,202,694.00	984,400.97	55.3
<u>INTERGOVERNMENTAL</u>					
100-42001 EXEMPT COMPUTER AID	.00	.00	500.00	500.00	.0
100-42002 STATE SHARED REVENUE	.00	.00	64,000.00	64,000.00	.0
100-42006 TRANSPORTATION AID	.00	55,188.02	210,000.00	154,811.98	26.3
100-42007 POLICE TRAINING FROM STATE	.00	.00	1,400.00	1,400.00	.0
100-42008 OTHER STATE AIDS	.00	.00	24,000.00	24,000.00	.0
100-42009 POLICE GRANT FROM CTY	340.00	1,479.84	3,000.00	1,520.16	49.3
100-42010 POLICE SRO REVENUE	.00	29,406.45	67,000.00	37,593.55	43.9
100-42012 MISC GRANTS	6,403.12	37,312.44	.00	(37,312.44)	.0
100-42013 POLICE FEDERAL/DOJ GRANTS	.00	518.00	.00	(518.00)	.0
TOTAL INTERGOVERNMENTAL	6,743.12	123,904.75	369,900.00	245,995.25	33.5
<u>LICENSES & PERMITS</u>					
100-43001 LIQUOR/BEER LICENSE	.00	.00	6,300.00	6,300.00	.0
100-43002 OPERATOR LICENSE	80.00	250.00	2,200.00	1,950.00	11.4
100-43006 BUILDING PERMITS	.00	6,625.36	80,000.00	73,374.64	8.3
100-43007 ELECTRICAL PERMITS	.00	2,520.10	17,000.00	14,479.90	14.8
100-43008 PLUMBING PERMITS	.00	2,268.86	20,000.00	17,731.14	11.3
100-43009 ROOM TAX PERMIT	175.00	350.00	600.00	250.00	58.3
100-43014 CIGARETTE LICENSE	.00	.00	300.00	300.00	.0
100-43015 ZONING AND PLANNING FEES	2,100.00	3,945.00	20,000.00	16,055.00	19.7
100-43016 TREE PERMIT	200.00	895.00	6,000.00	5,105.00	14.9
100-43018 DOG LICENSE	345.00	795.00	1,000.00	205.00	79.5
100-43021 TOURIST ROOMING HOUSE PERMIT	.00	5,400.00	8,100.00	2,700.00	66.7
100-43022 SHORT TERM RENTAL PERMIT	.00	.00	19,000.00	19,000.00	.0
100-43025 TRANSIENT MERCHANT PERMIT	.00	350.00	150.00	(200.00)	233.3
100-43090 INVOICED SERVICE PAYMENTS	10,487.50	15,361.18	124,000.00	108,638.82	12.4
TOTAL LICENSES & PERMITS	13,387.50	38,760.50	304,650.00	265,889.50	12.7

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

GENERAL FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEARNED	PCNT
<u>PUBLIC CHARGES FOR SERVICE</u>						
100-44040	POLICE GEN REVENUE	6.00	51.00	250.00	199.00	20.4
100-44049	SPECIAL ASSESSMENT LETTERS	360.00	760.00	4,000.00	3,240.00	19.0
100-44060	STREET OPENING PERMIT	150.00	250.00	1,000.00	750.00	25.0
100-44090	DONATION/SPONSORSHIP REVENUE	600.00	600.00	6,000.00	5,400.00	10.0
100-44092	RECREATION MISC REVENUE	623.26	1,119.62	3,700.00	2,580.38	30.3
100-44094	ATHLETIC PROGRAM REVENUE	.00	270.00	12,000.00	11,730.00	2.3
100-44098	REC DEPT PROGRAM REVENUE	(2,845.26)	(5,069.26)	15,000.00	20,069.26	(33.8)
100-44101	REC DEPT EVENTS/TRIPS	.00	.00	500.00	500.00	.0
100-44102	MISC REC REVENUE	(120.00)	.00	.00	.00	.0
100-44106	DONATION FOR REC DEPT-MISC	(600.00)	(600.00)	.00	600.00	.0
100-44107	FACILITY RENTAL REVENUE	1,190.00	2,500.00	9,300.00	6,800.00	26.9
100-44301	WATERWAY MARKERS	.00	.00	6,300.00	6,300.00	.0
100-44620	LAKEFRONT/ShORE INCOME	876.00	1,260.00	16,000.00	14,740.00	7.9
100-44621	BEACH REVENUE	2,005.00	2,190.00	85,000.00	82,810.00	2.6
100-44622	LAUNCH REVENUE	5,701.25	5,838.75	210,000.00	204,161.25	2.8
100-44623	HORVATH DRY STORAGE REVENUE	4,557.60	4,507.60	25,300.00	20,792.40	17.8
100-44625	TOWN OF LINN BEACH REVENUE	.00	.00	15,000.00	15,000.00	.0
100-44630	KAYAK/PADDLEBOARD RENTAL FEES	.00	.00	15,000.00	15,000.00	.0
TOTAL PUBLIC CHARGES FOR SERVICE		12,503.85	13,677.71	424,350.00	410,672.29	3.2
<u>FINES & FORFEITURES</u>						
100-45001	COURT FINE REVENUE	1,851.60	4,878.97	26,000.00	21,121.03	18.8
100-45002	PARKING TICKET REVENUE	400.00	900.00	8,000.00	7,100.00	11.3
TOTAL FINES & FORFEITURES		2,251.60	5,778.97	34,000.00	28,221.03	17.0
<u>SOURCE 46</u>						
100-46000	WEED AND NUISANCE CONTROL	750.00	750.00	750.00	.00	100.0
TOTAL SOURCE 46		750.00	750.00	750.00	.00	100.0
<u>COMMERCIAL</u>						
100-48004	INTEREST ON INVESTMENTS	.00	64,907.04	48,680.00	(16,227.04)	133.3
100-48005	PROPERTY RENTAL INCOME	(13,395.00)	.00	.00	.00	.0
100-48013	BOAT SLIP RENTAL	219,183.24	242,644.24	286,000.00	43,355.76	84.8
100-48015	GENEVA LK LAW ENFORCEMENT AGCY	.00	.00	32,000.00	32,000.00	.0
100-48016	MUNICIPAL BUSINESS LEASE PYMT	.00	.00	1,000.00	1,000.00	.0
100-48020	GLEA CBCW GRANT	.00	.00	2,400.00	2,400.00	.0
TOTAL COMMERCIAL		205,788.24	307,551.28	370,080.00	62,528.72	83.1

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

GENERAL FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEARNED	PCNT
	<u>MISCELLANEOUS</u>					
100-49002	INSURANCE PAYMENTS/REBATE	10,000.00	10,000.00	2,500.00	(7,500.00)	400.0
100-49003	SALE OF VGE ASSET	.00	.00	1,000.00	1,000.00	.0
100-49009	GENERAL MISC UNCLASS	175.88	315.83	500.00	184.17	63.2
	TOTAL MISCELLANEOUS	10,175.88	10,315.83	4,000.00	(6,315.83)	257.9
	TOTAL FUND REVENUE	1,443,544.03	1,719,032.07	3,710,424.00	1,991,391.93	46.3

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

GENERAL FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>GENERAL GOVERNMENT</u>					
100-51110-110 VILLAGE BOARD WAGES	5,512.50	5,512.50	22,100.00	16,587.50	24.9
100-51110-121 VILLAGE BOARD FICA	421.72	421.72	1,700.00	1,278.28	24.8
100-51110-130 VILLAGE BOARD OTHER EXPENSE	119.82	206.87	1,000.00	793.13	20.7
100-51210-110 MUNICIPAL COURT WAGES	2,914.70	8,744.10	9,300.00	555.90	94.0
100-51210-121 MUNICIPAL COURT FICA	222.98	668.94	770.00	101.06	86.9
100-51210-150 MUNICIPAL COURT IT FEES	37.09	4,214.27	1,500.00	(2,714.27)	281.0
100-51405-000 BILLABLE SERVICES	149.29	15,674.18	109,000.00	93,325.82	14.4
100-51410-110 GEN ADMIN WAGES	13,057.84	39,153.25	170,346.00	131,192.75	23.0
100-51410-112 GEN ADMIN OT WAGES	20.58	68.60	.00	(68.60)	.0
100-51410-121 GEN ADMIN FICA	941.09	2,822.22	13,091.00	10,268.78	21.6
100-51410-122 GEN ADMIN RETIREMENT	908.96	2,725.94	11,938.00	9,212.06	22.8
100-51410-123 GEN ADMIN HEALTH INSURANCE	2,985.89	8,963.67	34,976.00	26,012.33	25.6
100-51410-124 GEN ADMIN LIFE INSURANCE	60.70	182.10	750.00	567.90	24.3
100-51410-127 GEN ADMIN HSA FUNDING	604.16	1,208.34	7,250.00	6,041.66	16.7
100-51410-130 GEN ADMIN IT EXPENSE	3,334.99	3,494.97	4,000.00	505.03	87.4
100-51410-160 GEN ADMIN SUPPLIES	904.75	1,789.33	10,000.00	8,210.67	17.9
100-51410-161 GEN ADMIN POSTAGE	188.81	188.81	6,500.00	6,311.19	2.9
100-51410-162 GEN ADMIN COPIER EXPENSE	.00	623.18	3,600.00	2,976.82	17.3
100-51410-190 GEN ADMIN TRAINING	499.00	2,414.88	7,000.00	4,585.12	34.5
100-51410-200 GEN ADMIN TELEPHONE	468.19	1,403.01	4,000.00	2,596.99	35.1
100-51410-210 GEN ADMIN PUBLICATIONS	194.18	458.23	3,500.00	3,041.77	13.1
100-51410-300 GEN ADMIN CODIFICATION	.00	1,728.00	6,200.00	4,472.00	27.9
100-51412-000 ELECTION EXPENSE	62.31	62.31	3,000.00	2,937.69	2.1
100-51412-110 ELECTION WAGES	.00	2,553.80	7,000.00	4,446.20	36.5
100-51412-121 ELECTION FICA	.00	.00	30.00	30.00	.0
100-51414-000 SOFTWARE LICENSE & IT SUPPORT	845.46	11,297.83	30,000.00	18,702.17	37.7
100-51414-100 LASERFICHE EXPENSE	.00	.00	3,100.00	3,100.00	.0
100-51415-000 LEAGUE EXPENSES/DUES	.00	.00	3,000.00	3,000.00	.0
100-51510-000 INSURANCE EXPENSE	.00	61,181.00	66,000.00	4,819.00	92.7
100-51520-000 ASSESSOR CONTRACT	2,500.00	7,500.00	30,000.00	22,500.00	25.0
100-51560-000 CONTINGENCY	.00	.00	34,326.00	34,326.00	.0
100-51570-000 AUDIT EXPENSE	.00	700.00	28,150.00	27,450.00	2.5
100-51610-000 LEGAL SERVICES	.00	.00	35,000.00	35,000.00	.0
100-51720-150 LIONS FIELD HOUSE REPAIR/MAINT	.00	394.18	2,000.00	1,605.82	19.7
100-51720-160 LIONS FIELD HOUSE SUPPLIES	.00	54.16	700.00	645.84	7.7
100-51720-170 LIONS FIELD HOUSE GAS	374.95	1,021.85	1,800.00	778.15	56.8
100-51720-171 LIONS FIELD HOUSE ELECTRIC	158.03	308.99	1,800.00	1,491.01	17.2
100-51720-173 LIONS FIELD HOUSE W&S	.00	168.22	1,000.00	831.78	16.8
100-51720-175 JANITORIAL SERVICES	.00	871.48	5,800.00	4,928.52	15.0
100-51730-150 VH BLDG REPAIRS/MAINT	179.61	503.61	5,000.00	4,496.39	10.1
100-51730-160 VH BLDG SUPPLIES	159.24	271.07	2,000.00	1,728.93	13.6
100-51730-170 VH BLDG GAS	471.84	1,239.20	2,500.00	1,260.80	49.6
100-51730-171 VH BLDG ELECTRIC	991.56	1,954.38	8,000.00	6,045.62	24.4
100-51730-173 VH BLDG WATER & SEWER	.00	424.17	2,000.00	1,575.83	21.2
100-51730-175 JANITORIAL SERVICES	.00	1,350.00	8,000.00	6,650.00	16.9
100-51965-000 WMS BAY BUSINESS ASSOC	.00	8,828.14	52,500.00	43,671.86	16.8
100-51970-000 SHORT TERM RENTAL ADMIN	.00	.00	6,600.00	6,600.00	.0
TOTAL GENERAL GOVERNMENT	39,290.24	203,351.50	767,827.00	564,475.50	26.5

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

GENERAL FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>PUBLIC SAFETY</u>					
100-52120-110 POLICE WAGES	57,871.74	169,309.20	726,500.00	557,190.80	23.3
100-52120-111 POLICE PT SHIFT PREMIUM	.00	.00	500.00	500.00	.0
100-52120-112 POLICE OT WAGES	3,518.17	11,491.02	60,000.00	48,508.98	19.2
100-52120-121 POLICE FICA	4,570.56	13,454.18	61,000.00	47,545.82	22.1
100-52120-122 POLICE RETIREMENT	8,387.99	25,513.03	116,000.00	90,486.97	22.0
100-52120-123 POLICE HEALTH INSURANCE	15,169.06	47,510.34	164,000.00	116,489.66	29.0
100-52120-124 POLICE LIFE INSURANCE	101.22	303.66	1,000.00	696.34	30.4
100-52120-125 POLICE UNIFORMS	652.55	1,101.07	10,000.00	8,898.93	11.0
100-52120-127 POLICE HSA FUNDING	3,124.95	6,249.90	35,000.00	28,750.10	17.9
100-52120-130 POLICE IT EXPENSE	382.90	28,073.75	58,766.00	30,692.25	47.8
100-52120-150 POLICE REPAIRS/MAINT (	93.10)	5,167.51	11,665.00	6,497.49	44.3
100-52120-160 POLICE SUPPLIES	351.69	380.82	5,300.00	4,919.18	7.2
100-52120-161 POLICE POSTAGE/CONFINEMENT	12.00	12.00	650.00	638.00	1.9
100-52120-180 POLICE FUEL	1,543.29	4,947.83	21,000.00	16,052.17	23.6
100-52120-190 POLICE TRAINING	1,413.89	4,583.89	23,451.00	18,867.11	19.6
100-52120-200 POLICE TELEPHONE	385.70	1,157.51	6,500.00	5,342.49	17.8
100-52120-210 POLICE COMMUNITY PROGRAMS	.00	.00	3,000.00	3,000.00	.0
100-52120-220 POLICE LICENSE SUPSESION FEE	.00	3.00	.00	(3.00)	.0
100-52130-110 WATER SAFETY PATROL	.00	.00	33,000.00	33,000.00	.0
100-52130-120 GENEVA LAKE ENVIRONMENTAL AGCY	.00	.00	45,000.00	45,000.00	.0
100-52130-130 GENEVA LAKE LAW ENFORCEMENT	.00	75,000.00	75,000.00	.00	100.0
100-52130-150 EMERGENCY MANAGEMENT	.00	.00	1,200.00	1,200.00	.0
100-52320-106 FIRE DEPT OFFICER PAY	2,083.36	6,189.39	25,700.00	19,510.61	24.1
100-52320-107 FIRE DEPT MEETING PAY	307.20	847.40	3,700.00	2,852.60	22.9
100-52320-108 FIRE DEPT DRILL PAY	598.29	1,668.07	10,260.00	8,591.93	16.3
100-52320-109 FIRE DEPT CALLS PAY	888.37	2,169.35	16,000.00	13,830.65	13.6
100-52320-120 FIRE DEPT TRAINING PAY	.00	.00	1,250.00	1,250.00	.0
100-52320-121 FIRE DEPT FICA	283.81	803.56	4,350.00	3,546.44	18.5
100-52320-122 FIRE DEPT RETIREMENT	72.37	198.07	850.00	651.93	23.3
100-52320-130 FIRE DEPT IT	361.42	377.17	1,200.00	822.83	31.4
100-52320-150 FIRE DEPT REPAIRS/MAINT	47.62	142.86	16,000.00	15,857.14	.9
100-52320-160 FIRE DEPT SUPPLIES	.00	5,930.52	6,500.00	569.48	91.2
100-52320-170 FIRE DEPT GAS	.00	.00	5,300.00	5,300.00	.0
100-52320-171 FIRE DEPT ELECTRIC	175.62	362.50	2,000.00	1,637.50	18.1
100-52320-173 FIRE DEPT WATER & SEWER	.00	160.75	800.00	639.25	20.1
100-52320-180 FIRE DEPT FUEL	.00	44.63	1,000.00	955.37	4.5
100-52320-190 FIRE DEPT TRAINING/CONFERENCES	.00	.00	1,000.00	1,000.00	.0
100-52320-200 FIRE DEPT TELEPHONE	311.37	971.22	3,500.00	2,528.78	27.8
100-52340-106 DIVE TEAM OFFICER PAY	.00	.00	960.00	960.00	.0
100-52340-107 DIVE TEAM MEETING PAY	9.58	9.58	640.00	630.42	1.5
100-52340-108 DIVE TEAM DRILL PAY	31.94	31.94	.00	(31.94)	.0
100-52340-121 DIVE TEAM FICA	3.17	3.17	130.00	126.83	2.4
100-52340-145 DIVE TEAM TANK MAINTENANCE	.00	.00	4,260.00	4,260.00	.0
100-52340-150 DIVE TEAM REPAIRS/MAINT	.00	.00	300.00	300.00	.0
100-52340-190 DIVE TEAM TRAINING	.00	.00	800.00	800.00	.0
100-52360-106 RESCUE DEPT OFFICER PAY	373.12	1,108.49	5,400.00	4,291.51	20.5
100-52360-107 RESCUE DEPT MEETING PAY	28.74	151.32	700.00	548.68	21.6
100-52360-108 RESCUE DEPT TRAINING PAY	127.70	480.08	1,000.00	519.92	48.0
100-52360-109 RESCUE DEPT CALLS PAY	.00	1,708.44	4,000.00	2,291.56	42.7
100-52360-110 RESCUE DEPT UNEMPLOYMENT	.00	1.00	.00	(1.00)	.0
100-52360-121 RESCUE DEPT FICA	35.15	130.65	900.00	769.35	14.5
100-52360-122 RESCUE DEPT RETIREMENT	.00	.00	100.00	100.00	.0

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

GENERAL FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
100-52360-130 RESCUE DEPT IT EXPENSE	.00	12.00	300.00	288.00	4.0
100-52360-160 RESCUE DEPT SUPPLIES	.00	35.52	200.00	164.48	17.8
100-52360-180 RESCUE DEPT FUEL	.00	.00	300.00	300.00	.0
100-52360-190 RESCUE DEPT TRAINING	.00	.00	500.00	500.00	.0
100-52360-200 RESCUE DEPT TELEPHONE	97.05	291.14	600.00	308.86	48.5
TOTAL PUBLIC SAFETY	103,228.49	418,087.53	1,579,032.00	1,160,944.47	26.5

PUBLIC SAFETY

100-53100-160 BLDG INSP SUPPLIES	210.80	210.80	500.00	289.20	42.2
100-53100-210 ZONING INSPECTION CONTRACT	1,645.50	2,125.50	25,000.00	22,874.50	8.5
100-53100-211 BLDG INSPECTION CONTRACT	.00	8,561.88	93,600.00	85,038.12	9.2
100-53100-215 CODE ENFORCEMENT CONTRACT	3,281.20	6,619.60	33,000.00	26,380.40	20.1
100-53100-220 STR ENFORCEMENT CONTRACT	.00	.00	7,000.00	7,000.00	.0
TOTAL PUBLIC SAFETY	5,137.50	17,517.78	159,100.00	141,582.22	11.0

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

GENERAL FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>PUBLIC WORKS</u>					
100-54100-110 DPW ADMIN WAGES	3,593.36	10,769.77	47,500.00	36,730.23	22.7
100-54100-121 DPW ADMIN FICA	239.50	717.73	3,600.00	2,882.27	19.9
100-54100-122 DPW ADMIN RETIREMENT	247.80	742.68	3,300.00	2,557.32	22.5
100-54100-123 DPW ADMIN HEALTH INSURANCE	1,046.39	3,139.17	12,600.00	9,460.83	24.9
100-54100-124 DPW ADMIN LIFE INSURANCE	.00	.00	100.00	100.00	.0
100-54100-127 DPW HSA FUNDING	208.33	416.66	2,500.00	2,083.34	16.7
100-54100-300 ENGINEERING	.00	.00	6,000.00	6,000.00	.0
100-54310-110 STREETS WAGES	7,203.26	21,108.36	89,500.00	68,391.64	23.6
100-54310-112 STREETS OT WAGES	47.57	587.60	1,700.00	1,112.40	34.6
100-54310-113 STREETS DBL OT WAGES	84.19	84.19	3,700.00	3,615.81	2.3
100-54310-121 STREETS FICA	542.37	1,607.65	7,000.00	5,392.35	23.0
100-54310-122 STREETS RETIREMENT	499.72	1,513.09	6,400.00	4,886.91	23.6
100-54310-123 STREETS HEALTH INSURANCE	1,596.12	4,788.36	19,400.00	14,611.64	24.7
100-54310-124 STREETS LIFE INSURANCE	17.47	52.40	250.00	197.60	21.0
100-54310-125 STREETS UNIFORMS	150.00	150.00	2,500.00	2,350.00	6.0
100-54310-127 STREETS HSA FUNDING	288.32	581.60	4,200.00	3,618.40	13.9
100-54310-150 STREETS EQUIP REPAIRS/MAINT	2,103.79	9,840.77	25,000.00	15,159.23	39.4
100-54310-160 STREETS SUPPLIES	312.88	1,193.88	6,500.00	5,306.12	18.4
100-54310-170 STREETS GAS	814.10	2,076.37	4,000.00	1,923.63	51.9
100-54310-171 STREETS ELECTRIC	606.61	1,184.76	6,000.00	4,815.24	19.8
100-54310-173 STREETS WATER & SEWER	.00	183.16	900.00	716.84	20.4
100-54310-175 STREETS ROAD MAINTENANCE	.00	1,966.98	25,000.00	23,033.02	7.9
100-54310-180 STREETS FUEL	191.20	243.17	9,500.00	9,256.83	2.6
100-54310-190 STREETS TRAINING	27.83	27.83	1,500.00	1,472.17	1.9
100-54310-200 STREETS TELEPHONE	45.14	135.42	500.00	364.58	27.1
100-54310-280 SNOW/ICE CONTROL MATERIALS	3,933.98	7,802.45	17,500.00	9,697.55	44.6
100-54310-281 TRAFFIC SIGNS/MARKINGS	.00	146.50	2,000.00	1,853.50	7.3
100-54310-282 HIGHWAY STRIPING/MARKING	.00	.00	1,000.00	1,000.00	.0
100-54420-000 STREET LIGHTING	3,316.41	9,670.86	37,000.00	27,329.14	26.1
100-54710-000 REFUSE COLLECTIONS	.00	26,091.38	167,600.00	141,508.62	15.6
TOTAL PUBLIC WORKS	27,116.34	106,822.79	514,250.00	407,427.21	20.8

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

GENERAL FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>LEISURE ACTIVITIES</u>					
100-55210-110 REC DEPT WAGES	7,873.08	23,000.56	122,000.00	98,999.44	18.9
100-55210-121 REC DEPT FICA	578.06	1,686.87	9,300.00	7,613.13	18.1
100-55210-122 REC DEPT RETIREMENT	407.48	1,220.19	5,300.00	4,079.81	23.0
100-55210-123 REC DEPT HEALTH INSURANCE	840.04	2,520.12	10,000.00	7,479.88	25.2
100-55210-124 REC DEPT LIFE INSURANCE	6.05	18.15	80.00	61.85	22.7
100-55210-127 REC DEPT HSA FUNDING	208.33	416.66	2,500.00	2,083.34	16.7
100-55210-130 REC DEPT TECHNOLOGY/IT	60.74	703.36	2,200.00	1,496.64	32.0
100-55210-143 FACILITY MAINTENANC/EQUIPMENT	.00	939.85	6,000.00	5,060.15	15.7
100-55210-160 REC DEPT SUPPLIES/MATERIALS	70.59	517.97	2,500.00	1,982.03	20.7
100-55210-161 REC DEPT MARKETING/PROMOTION	.00	.00	4,000.00	4,000.00	.0
100-55210-190 REC DEPT TRAINING	.00	.00	1,750.00	1,750.00	.0
100-55210-220 REC DEPT SOFTWARE/SUBSCRIPTION	201.86	535.08	1,500.00	964.92	35.7
100-55210-270 REC DEPT ATHLETIC PROGRAM EXP (	3,044.39) (	1,069.39)	20,000.00	21,069.39 (	5.4)
100-55210-275 REC DEPT PROGRAM EXPENSES	330.05	2,207.88	16,000.00	13,792.12	13.8
100-55210-278 EVENT EXPENSES	.00	.00	2,000.00	2,000.00	.0
100-55210-279 REC DEPT DONATION/SPONSORSHIP	.00	999.99	.00 (	999.99)	.0
100-55210-280 KAYAK/PADDLEBOARD EXPENSE	.00	.00	2,000.00	2,000.00	.0
100-55210-281 RECREATION MISC EXPENSE	.00	.00	1,000.00	1,000.00	.0
100-55410-110 PARKS WAGES	2,880.42	8,152.49	64,000.00	55,847.51	12.7
100-55410-112 PARKS OT WAGES	.00	334.43	550.00	215.57	60.8
100-55410-113 PARKS DBL OT WAGES	66.06	66.06	.00 (	66.06)	.0
100-55410-115 PARKS CONTRACT LABOR	.00	.00	10,000.00	10,000.00	.0
100-55410-121 PARKS FICA	218.18	632.61	4,900.00	4,267.39	12.9
100-55410-122 PARKS RETIREMENT	188.25	577.89	2,500.00	1,922.11	23.1
100-55410-123 PARKS HEALTH INSURANCE	1,039.75	3,119.25	12,500.00	9,380.75	25.0
100-55410-124 PARKS LIFE INSURANCE	13.24	39.72	170.00	130.28	23.4
100-55410-125 PARKS UNIFORMS	.00	.00	2,000.00	2,000.00	.0
100-55410-127 PARKS HSA FUNDING	267.22	529.48	2,500.00	1,970.52	21.2
100-55410-148 TENNIS COURT MAINTENANCE	.00	.00	2,000.00	2,000.00	.0
100-55410-150 PARKS REPAIRS/MAINT	795.85	1,387.84	19,000.00	17,612.16	7.3
100-55410-160 PARKS SUPPLIES	.00	398.56	1,500.00	1,101.44	26.6
100-55410-170 PARKS GAS	438.18	1,016.53	4,500.00	3,483.47	22.6
100-55410-171 PARKS ELECTRIC	1,359.29	2,773.67	8,000.00	5,226.33	34.7
100-55410-173 PARKS WATER & SEWER	.00	710.23	3,740.00	3,029.77	19.0
100-55410-180 PARKS FUEL	111.44	246.21	7,500.00	7,253.79	3.3
100-55411-110 LAKEFRONT WAGES	.00	.00	86,000.00	86,000.00	.0
100-55411-121 LAKEFRONT FICA	.00	.00	6,800.00	6,800.00	.0
100-55411-125 LAKEFRONT UNIFORMS	.00	.00	500.00	500.00	.0
100-55411-130 LAKEFRONT IT	.00	11.25	600.00	588.75	1.9
100-55411-150 LAKEFRONT REPAIRS/MAINT	.00	612.39	8,000.00	7,387.61	7.7
100-55411-153 LAKEFRONT PIER REPAIRS	.00	.00	70,000.00	70,000.00	.0
100-55411-154 LAKEFRONT PIER INSTALLATION	.00	.00	125,000.00	125,000.00	.0
100-55411-160 LAKEFRONT SUPPLIES	.00	5,353.50	11,000.00	5,646.50	48.7
100-55411-200 LAKEFRONT TELEPHONE	102.12	306.33	1,000.00	693.67	30.6
100-55412-000 GENEVA LK LEVEL CORP	.00	.00	4,400.00	4,400.00	.0
100-55415-000 ENHANCEMENT COMMITTEE	.00	.00	800.00	800.00	.0
TOTAL LEISURE ACTIVITIES	15,011.89	59,965.73	667,590.00	607,624.27	9.0

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

GENERAL FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>CONSERVATION & DEVELOPMENT</u>					
100-56120-000	HOLIDAY DECORATION SUPPLIES	.00	568.00	1,500.00	932.00	37.9
100-56130-000	TREE ENHANCEMENT	.00	56.94	8,500.00	8,443.06	.7
100-56420-190	HORVATH PROPERTY EXPENSE	.00	.00	1,000.00	1,000.00	.0
	TOTAL CONSERVATION & DEVELOPMENT	.00	624.94	11,000.00	10,375.06	5.7
	<u>COST CATEGORY 57</u>					
100-57921-142	BANK SERVICE CHARGES AND FEES	102.15	102.15	.00	(102.15)	.0
100-57921-150	LATE FEES, PENALTIES, INTEREST	.00	.50	.00	(.50)	.0
	TOTAL COST CATEGORY 57	102.15	102.65	.00	(102.65)	.0
	TOTAL FUND EXPENDITURES	189,886.61	806,472.92	3,698,799.00	2,892,326.08	21.8
	NET REVENUE OVER EXPENDITURES	1,253,657.42	912,559.15	11,625.00	(900,934.15)	7850.0

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

POLCE DEPT DONATIONS

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>INTERGOVERNMENTAL</u>					
110-42004	POLICE DONATIONS	.00	20.00	.00	(20.00)	.0
	TOTAL INTERGOVERNMENTAL	.00	20.00	.00	(20.00)	.0
	TOTAL FUND REVENUE	.00	20.00	.00	(20.00)	.0
	NET REVENUE OVER EXPENDITURES	.00	20.00	.00	(20.00)	.0

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

EMS IGA FUNDS

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>TAXES</u>					
120-41100	EMS TAX LEVY	616,854.04	616,854.04	.00	(616,854.04)	.0
	TOTAL TAXES	616,854.04	616,854.04	.00	(616,854.04)	.0
	<u>INTERGOVERNMENTAL</u>					
120-42004	EMS IGA FUNDS	.00	.00	928,077.00	928,077.00	.0
	TOTAL INTERGOVERNMENTAL	.00	.00	928,077.00	928,077.00	.0
	<u>MISCELLANEOUS</u>					
120-49200	OPERATING TRANSFER IN	.00	.00	121,923.00	121,923.00	.0
	TOTAL MISCELLANEOUS	.00	.00	121,923.00	121,923.00	.0
	TOTAL FUND REVENUE	616,854.04	616,854.04	1,050,000.00	433,145.96	58.8

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

		EMS IGA FUNDS				
		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>EMS IGA EXPENDITURES</u>						
120-52320-165	EMS IGA EXPENDITURES	.00	.00	1,050,000.00	1,050,000.00	.0
	TOTAL EMS IGA EXPENDITURES	.00	.00	1,050,000.00	1,050,000.00	.0
	TOTAL FUND EXPENDITURES	.00	.00	1,050,000.00	1,050,000.00	.0
	NET REVENUE OVER EXPENDITURES	616,854.04	616,854.04	.00	(616,854.04)	.0

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

AD HOC TREE REPLACEMENT PROG

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>INTERGOVERNMENTAL</u>					
125-42004	TREE ENHANCEMENT REVENUE	52.02	172.79	.00	(172.79)	.0
	TOTAL INTERGOVERNMENTAL	52.02	172.79	.00	(172.79)	.0
	TOTAL FUND REVENUE	52.02	172.79	.00	(172.79)	.0
	NET REVENUE OVER EXPENDITURES	52.02	172.79	.00	(172.79)	.0

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

WATER FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>PUBLIC CHARGES FOR SERVICE</u>					
200-44460	WATER RESIDENTIAL SALES	107,140.70	107,410.64	508,000.00	400,589.36	21.1
200-44461	WATER COMMERCIAL SALES	20,089.70	20,089.70	100,000.00	79,910.30	20.1
200-44463	WATER PUBLIC FIRE PROTECTION	66,488.27	66,601.25	265,000.00	198,398.75	25.1
200-44464	WATER PUBLIC AUTH SALES	2,247.05	2,247.05	19,000.00	16,752.95	11.8
200-44465	PRIVATE FIRE REVENUE-WTR	904.20	904.20	3,700.00	2,795.80	24.4
200-44466	MULTIFAMILY RESIDENTIAL	4,256.36	4,256.36	20,000.00	15,743.64	21.3
200-44470	WATER FORFEITED DISCOUNTS	.00	887.77	4,000.00	3,112.23	22.2
200-44474	WATER OTHER REVENUE	235.00	239.00	2,000.00	1,761.00	12.0
200-44475	IMPACT FEES	.00	.00	40,000.00	40,000.00	.0
	TOTAL PUBLIC CHARGES FOR SERVICE	201,361.28	202,635.97	961,700.00	759,064.03	21.1
	<u>COMMERCIAL</u>					
200-48004	INTEREST ON INVESTMENTS	.00	38,949.83	185,500.00	146,550.17	21.0
200-48005	PROPERTY RENTAL/TOWER LEASE	13,395.00	46,012.80	45,000.00	(1,012.80)	102.3
200-48420	IMPACT FEE INTEREST INCOME	.00	1,257.14	3,000.00	1,742.86	41.9
	TOTAL COMMERCIAL	13,395.00	86,219.77	233,500.00	147,280.23	36.9
	TOTAL FUND REVENUE	214,756.28	288,855.74	1,195,200.00	906,344.26	24.2

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

WATER FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>WATER UTILITY</u>					
200-57346-000 NEW WATER METERS	.00	6,509.60	25,000.00	18,490.40	26.0
200-57346-100 METER REPLACEMENT PROGRAM	.00	.00	30,000.00	30,000.00	.0
200-57408-000 WATER TAX EQUIVALENT	.00	.00	100,800.00	100,800.00	.0
200-57599-000 WATER INSURANCE EXPENSE	.00	30,453.00	33,000.00	2,547.00	92.3
200-57601-150 WELL #1 - REPAIRS	.00	.00	800.00	800.00	.0
200-57602-150 WELL #2 - REPAIRS	(79,729.75)	.00	800.00	800.00	.0
200-57610-140 WATER FUND CAPITAL OUTLAY	560,463.00	560,463.00	.00	(560,463.00)	.0
200-57622-170 WATER POWER - NATURAL GAS	725.67	1,891.83	7,000.00	5,108.17	27.0
200-57622-171 WATER POWER - ELECTRIC	6,960.47	13,554.39	65,000.00	51,445.61	20.9
200-57623-160 WATER PUMPING SUPPLIES	.00	.00	200.00	200.00	.0
200-57623-170 WATER TESTING	60.00	220.00	3,000.00	2,780.00	7.3
200-57630-110 WATER TREATMENT - WAGES	4,322.87	12,955.88	55,000.00	42,044.12	23.6
200-57630-112 WATER TREATMENT - OT WAGES	47.57	253.17	1,500.00	1,246.83	16.9
200-57630-113 WATER TREATMENT - DBL OT WAGES	18.13	18.13	1,500.00	1,481.87	1.2
200-57630-121 WATER TREATMENT - FICA	324.19	974.98	4,400.00	3,425.02	22.2
200-57630-122 WATER TREATMENT - RETIREMENT	311.49	935.19	4,000.00	3,064.81	23.4
200-57630-123 WATER TREATMENT - HEALTH INS	556.36	1,669.08	6,800.00	5,130.92	24.6
200-57630-124 WATER TREATMENT - LIFE INS	4.21	12.63	100.00	87.37	12.6
200-57630-127 WATER TREATMENT HSA FUNDING	138.88	277.76	1,700.00	1,422.24	16.3
200-57631-160 WATER TREATMENT CHEMICALS	.00	13,877.46	140,000.00	126,122.54	9.9
200-57632-160 WATER TREATMENT SUPPLIES	.00	150.50	2,200.00	2,049.50	6.8
200-57635-150 WATER TREATMENT REPAIRS/ MAINT	131.32	475.82	5,000.00	4,524.18	9.5
200-57640-110 WATER DISTRIBUTION - WAGES	4,323.87	12,959.30	55,000.00	42,040.70	23.6
200-57640-112 WATER DISTRIBUTION - OT WAGES	47.58	253.23	1,500.00	1,246.77	16.9
200-57640-113 WATER DISTRIBUTION-DBL OT WAGE	18.14	18.14	1,500.00	1,481.86	1.2
200-57640-121 WATER DISTRIBUTION - FICA	324.30	975.21	4,225.00	3,249.79	23.1
200-57640-122 WATER DISTRIBUTION -RETIREMENT	311.59	935.54	4,000.00	3,064.46	23.4
200-57640-123 WATER DISTRIBUTION - HLTH INS	556.34	1,669.02	6,800.00	5,130.98	24.5
200-57640-124 WATER DISTRIBUTION - LIFE INS	4.21	12.63	100.00	87.37	12.6
200-57640-127 WATER DISTRIBUTION HSA FUNDING	138.90	277.80	1,800.00	1,522.20	15.4
200-57641-160 WATER DISTRIBUTION SUPPLIES	23.99	561.64	6,000.00	5,438.36	9.4
200-57650-150 WATER TOWERS REPAIRS/MAINT	.00	.00	8,000.00	8,000.00	.0
200-57651-150 WATER MAINS REPAIRS/MAINT	.00	26,091.63	25,000.00	(1,091.63)	104.4
200-57652-150 WATER SERVICES REPAIRS/ MAINT	.00	.00	10,000.00	10,000.00	.0
200-57653-151 WATER METER TESTING	.00	.00	4,000.00	4,000.00	.0
200-57654-150 HYDRANT REPAIRS/MAINT	.00	.00	12,000.00	12,000.00	.0
200-57656-150 CROSS CONNECTION INSPECTION	.00	1,810.00	22,000.00	20,190.00	8.2
200-57902-110 WATER ACCOUNTING - WAGES	4,347.73	13,055.32	57,000.00	43,944.68	22.9
200-57902-112 WATER ACCOUNTING - OT WAGES	30.87	102.90	.00	(102.90)	.0
200-57902-121 WATER ACCOUNTING - FICA	320.83	964.20	4,400.00	3,435.80	21.9
200-57902-122 WATER ACCOUNTING- RETIREMENT	304.32	914.52	4,100.00	3,185.48	22.3
200-57902-123 WATER ACCOUNTING - HEALTH INS	1,270.41	3,811.23	15,500.00	11,688.77	24.6
200-57902-124 WATER ACCOUNTING - LIFE INS	11.32	33.96	150.00	116.04	22.6
200-57902-127 WATER ACCT HSA FUNDING	260.41	520.83	3,200.00	2,679.17	16.3
200-57903-125 WATER METER READING - UNIFORMS	.00	462.46	5,500.00	5,037.54	8.4
200-57920-110 WATER ADMIN - WAGES	3,149.64	9,439.70	41,000.00	31,560.30	23.0
200-57920-121 WATER ADMIN - FICA	214.30	642.20	3,200.00	2,557.80	20.1
200-57920-122 WATER ADMIN - RETIREMENT	217.92	653.14	2,900.00	2,246.86	22.5
200-57920-123 WATER ADMIN - HEALTH INS	831.81	2,495.43	10,100.00	7,604.57	24.7
200-57920-124 WATER ADMIN - LIFE INS	12.33	36.99	.00	(36.99)	.0
200-57920-127 WATER ADMIN HSA FUNDING	166.67	333.33	2,000.00	1,666.67	16.7
200-57921-130 WATER IT EXPENSE	.00	18.74	1,200.00	1,181.26	1.6

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

WATER FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
200-57921-142 BANK-CR CARD-PMT PROCESS'G FEE	.00	1,293.92	.00	(1,293.92)	.0
200-57921-160 WATER OFFICE SUPPLIES	480.75	617.09	10,000.00	9,382.91	6.2
200-57921-200 WATER TELEPHONE	253.47	942.86	850.00	(92.86)	110.9
200-57923-000 WATER ATTORNEY EXPENSE	.00	.00	1,200.00	1,200.00	.0
200-57924-000 WATER ENGINEERING EXPENSE	.00	.00	10,000.00	10,000.00	.0
200-57925-000 WATER AUDIT EXPENSE	.00	700.00	12,000.00	11,300.00	5.8
200-57925-100 WATER RATE STUDY EXPENSE	.00	4,502.05	.00	(4,502.05)	.0
200-57928-000 WATER REGULATORY EXPENSE	.00	.00	950.00	950.00	.0
200-57930-130 WATER DRUG TESTING EXPENSE	.00	125.00	1,000.00	875.00	12.5
200-57930-140 WATER DIGGERS HOTLINE EXPENSE	.00	1,523.20	3,000.00	1,476.80	50.8
200-57930-190 WATER TRAINING EXPENSE	241.80	826.80	3,500.00	2,673.20	23.6
200-57933-150 WATER TRANSP REPAIRS/MAINT	.00	232.50	1,500.00	1,267.50	15.5
200-57933-180 WATER FUEL EXPENSE	27.35	376.30	1,500.00	1,123.70	25.1
200-57934-000 WATER DEBT SERVICE PRINCIPAL	.00	196,671.00	667,513.00	470,842.00	29.5
200-57934-150 WATER DEBT SERVICE - INTEREST	.00	110,785.00	299,363.00	188,578.00	37.0
200-57935-150 WATER PLANT REPAIRS/MAINT	252.87	4,791.64	15,000.00	10,208.36	31.9
200-57945-000 WATER DEBT SERVICE MISC EXP	.00	483.33	.00	(483.33)	.0
200-57951-000 WATER SHOP TOOLS	.00	.00	500.00	500.00	.0
TOTAL WATER UTILITY	512,478.13	1,047,612.20	1,827,851.00	780,238.80	57.3
WATER UTILITY					
200-58801-000 IT SUPPORT/ANNUAL FEES	.00	6,156.25	.00	(6,156.25)	.0
TOTAL WATER UTILITY	.00	6,156.25	.00	(6,156.25)	.0
TOTAL FUND EXPENDITURES	512,478.13	1,053,768.45	1,827,851.00	774,082.55	57.7
NET REVENUE OVER EXPENDITURES	(297,721.85)	(764,912.71)	(632,651.00)	132,261.71	(120.9)

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

FIRE DEPARTMENT 2% DUES

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>INTERGOVERNMENTAL</u>					
215-42004	2% FIRE DUES	.00	.00	35,000.00	35,000.00	.0
	TOTAL INTERGOVERNMENTAL	.00	.00	35,000.00	35,000.00	.0
	TOTAL FUND REVENUE	.00	.00	35,000.00	35,000.00	.0

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

FIRE DEPARTMENT 2% DUES

		<u>PERIOD ACTUAL</u>	<u>YTD ACTUAL</u>	<u>BUDGET</u>	<u>UNEXPENDED</u>	<u>PCNT</u>
	<u>2% DUES EXPENDITURES</u>					
215-52320-106	FIRE INSPECTOR OFFICER WAGES	533.03	1,583.56	6,000.00	4,416.44	26.4
215-52320-121	FIRE INSPECTOR FICA	45.68	130.26	560.00	429.74	23.3
215-52320-165	FIRE DUES 2% EXPENDITURES	.00	.00	10,000.00	10,000.00	.0
		<u>578.71</u>	<u>1,713.82</u>	<u>16,560.00</u>	<u>14,846.18</u>	<u>10.4</u>
	TOTAL 2% DUES EXPENDITURES					
		<u>578.71</u>	<u>1,713.82</u>	<u>16,560.00</u>	<u>14,846.18</u>	<u>10.4</u>
	TOTAL FUND EXPENDITURES					
		<u>578.71</u>	<u>1,713.82</u>	<u>16,560.00</u>	<u>14,846.18</u>	<u>10.4</u>
	NET REVENUE OVER EXPENDITURES	<u>(578.71)</u>	<u>(1,713.82)</u>	<u>18,440.00</u>	<u>20,153.82</u>	<u>(9.3)</u>

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

ACT 102 RESCUE GRANT

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>ACT 102 GRANT EXPENDITURES</u>					
220-52320-165	ACT 102 EXPENDITURES	.00	.00	8,000.00	8,000.00	.0
	TOTAL ACT 102 GRANT EXPENDITURES	.00	.00	8,000.00	8,000.00	.0
	TOTAL FUND EXPENDITURES	.00	.00	8,000.00	8,000.00	.0
	NET REVENUE OVER EXPENDITURES	.00	.00	(8,000.00)	(8,000.00)	.0

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

RECYCLING FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>TAXES</u>					
225-41100	TAX LEVY-RECYCLING	50,514.03	50,514.03	76,000.00	25,485.97	66.5
	TOTAL TAXES	50,514.03	50,514.03	76,000.00	25,485.97	66.5
	<u>INTERGOVERNMENTAL</u>					
225-42500	STATE RECYCLING GRANTS	.00	.00	8,000.00	8,000.00	.0
	TOTAL INTERGOVERNMENTAL	.00	.00	8,000.00	8,000.00	.0
	<u>MISCELLANEOUS</u>					
225-49200	OPERATING TRANSFER IN	.00	.00	8,000.00	8,000.00	.0
	TOTAL MISCELLANEOUS	.00	.00	8,000.00	8,000.00	.0
	TOTAL FUND REVENUE	50,514.03	50,514.03	92,000.00	41,485.97	54.9

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

RECYCLING FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>RECYCLING</u>					
225-54635-130 RECYCLING OTHER EXPENSE	.00	18,871.87	2,000.00	(16,871.87)	943.6
225-54635-131 RECYCLING FEES	.00	13,761.16	84,000.00	70,238.84	16.4
225-54635-150 RECYCLING REPAIRS/MAINT	.00	.00	3,000.00	3,000.00	.0
225-54635-160 RECYCLING PROGRAM SUPPLIES	.00	.00	500.00	500.00	.0
225-54635-165 RECYCLING TIPPING FEES	.00	.00	2,500.00	2,500.00	.0
TOTAL RECYCLING	.00	32,633.03	92,000.00	59,366.97	35.5
TOTAL FUND EXPENDITURES	.00	32,633.03	92,000.00	59,366.97	35.5
NET REVENUE OVER EXPENDITURES	50,514.03	17,881.00	.00	(17,881.00)	.0

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

KISHWAUKETOE FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>COMMERCIAL</u>					
230-48996 KISHAUKETOE REIMBURSE	.00	34,112.71	.00	(34,112.71)	.0
TOTAL COMMERCIAL	.00	34,112.71	.00	(34,112.71)	.0
TOTAL FUND REVENUE	.00	34,112.71	.00	(34,112.71)	.0

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

KISHWAUKETOE FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>KNC OPERATIONS</u>					
230-58100-110 KNC REGULAR WAGES	6,397.43	20,995.70	.00	(20,995.70)	.0
230-58100-121 KNC FICA	487.37	1,600.05	.00	(1,600.05)	.0
230-58100-122 KNC RETIREMENT	306.14	909.52	.00	(909.52)	.0
230-58100-123 KNC HEALTH INSURANCE	816.00	2,448.00	.00	(2,448.00)	.0
230-58100-124 KNC LIFE INSURANCE	5.00	15.00	.00	(15.00)	.0
230-58100-127 KNC HSA FUNDING	208.33	416.66	.00	(416.66)	.0
230-58100-130 KNC OTHER EXPENSE	808.00	808.00	.00	(808.00)	.0
TOTAL KNC OPERATIONS	9,028.27	27,192.93	.00	(27,192.93)	.0
TOTAL FUND EXPENDITURES	9,028.27	27,192.93	.00	(27,192.93)	.0
NET REVENUE OVER EXPENDITURES	(9,028.27)	6,919.78	.00	(6,919.78)	.0

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

FIRE & DIVE TEAM FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
260-47500 INTEREST INCOME	.00	492.95	.00	(492.95)	.0
TOTAL SOURCE 47	.00	492.95	.00	(492.95)	.0
 <u>COMMERCIAL</u>					
260-48504 DONATIONS - MISCELLANEOUS	.00	2,150.00	.00	(2,150.00)	.0
TOTAL COMMERCIAL	.00	2,150.00	.00	(2,150.00)	.0
TOTAL FUND REVENUE	.00	2,642.95	.00	(2,642.95)	.0

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

FIRE & DIVE TEAM FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
260-52508-000 MISCELLANEOUS	.00	2,375.10	.00	(2,375.10)	.0
TOTAL COST CATEGORY 52	.00	2,375.10	.00	(2,375.10)	.0
TOTAL FUND EXPENDITURES	.00	2,375.10	.00	(2,375.10)	.0
NET REVENUE OVER EXPENDITURES	.00	267.85	.00	(267.85)	.0

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

RESCUE SQUAD FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
270-47500	INTEREST INCOME	.00	51.44	.00	(51.44)	.0
	TOTAL SOURCE 47	.00	51.44	.00	(51.44)	.0
	COMMERCIAL					
270-48500	DONATIONS	.00	25.00	.00	(25.00)	.0
	TOTAL COMMERCIAL	.00	25.00	.00	(25.00)	.0
	TOTAL FUND REVENUE	.00	76.44	.00	(76.44)	.0

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

RESCUE SQUAD FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
270-52500-000 RESCUE SQUAD EXPENSES	.00	431.97	.00	(431.97)	.0
TOTAL COST CATEGORY 52	.00	431.97	.00	(431.97)	.0
TOTAL FUND EXPENDITURES	.00	431.97	.00	(431.97)	.0
NET REVENUE OVER EXPENDITURES	.00	(355.53)	.00	355.53	.0

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

SEWER FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>PUBLIC CHARGES FOR SERVICE</u>					
300-44460	SEWER RESIDENTIAL SALES	260,421.85	260,938.44	1,000,000.00	739,061.56	26.1
300-44461	SEWER COMMERCIAL SALES	42,173.29	42,173.29	168,000.00	125,826.71	25.1
300-44464	SEWER PUBLIC AUTHORITY SALES	3,614.96	3,614.96	14,000.00	10,385.04	25.8
300-44470	SEWER FORFEITED DISCOUNTS	.00	1,191.83	5,200.00	4,008.17	22.9
300-44475	SEWER CONNECTION FEE	.00	.00	80,750.00	80,750.00	.0
	TOTAL PUBLIC CHARGES FOR SERVICE	306,210.10	307,918.52	1,267,950.00	960,031.48	24.3
	<u>COMMERCIAL</u>					
300-48004	INTEREST ON INVESTMENTS	.00	18,004.02	38,500.00	20,495.98	46.8
300-48005	PROPERTY RENTAL INCOME	.00	.00	13,395.00	13,395.00	.0
	TOTAL COMMERCIAL	.00	18,004.02	51,895.00	33,890.98	34.7
	TOTAL FUND REVENUE	306,210.10	325,922.54	1,319,845.00	993,922.46	24.7

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

SEWER FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>SEWER UTILITY</u>					
300-58801-000 IT SUPPORT/ANNUAL FEES	.00	2,456.25	4,000.00	1,543.75	61.4
300-58929-000 SEWER SYSTEM MAPPING	.00	.00	250.00	250.00	.0
300-58960-000 SEWER DEBT SERVICE- PRINC	.00	103,155.00	103,156.00	1.00	100.0
300-58961-000 SEWER DEPT SERVICE - INTEREST	.00	56,596.50	114,224.00	57,627.50	49.6
300-58964-110 SEWER ACCOUNTING - WAGES	4,347.72	13,055.30	57,000.00	43,944.70	22.9
300-58964-112 SEWER ACCOUNTING - OT WAGES	30.87	102.90	.00	(102.90)	.0
300-58964-121 SEWER ACCOUNTING - FICA	320.77	964.05	4,400.00	3,435.95	21.9
300-58964-122 SEWER ACCOUNTING - RETIREMENT	304.30	914.44	4,100.00	3,185.56	22.3
300-58964-123 SEWER ACCOUNTING - HEALTH INS	1,270.40	3,811.20	16,000.00	12,188.80	23.8
300-58964-124 SEWER ACCOUNTING - LIFE INS	11.29	33.87	.00	(33.87)	.0
300-58964-127 SEWER ACCT HSA FUNDING	260.41	520.82	3,200.00	2,679.18	16.3
300-58965-000 SEWER DEBT SERVICE MISC EXP	.00	483.33	750.00	266.67	64.4
300-58965-110 SEWER ADMIN - WAGES	3,149.64	9,439.70	41,000.00	31,560.30	23.0
300-58965-121 SEWER ADMIN - FICA	214.30	642.18	3,200.00	2,557.82	20.1
300-58965-122 SEWER ADMIN - RETIREMENT	217.92	653.13	2,900.00	2,246.87	22.5
300-58965-123 SEWER ADMIN - HEALTH INS	831.79	2,495.37	10,100.00	7,604.63	24.7
300-58965-124 SEWER ADMIN - LIFE INS	12.32	36.97	200.00	163.03	18.5
300-58965-127 SEWER ADMIN HSA FUNDING	166.66	333.30	2,000.00	1,666.70	16.7
300-58965-140 SEWER CAPITAL OUTLAY	648,815.25	648,815.25	.00	(648,815.25)	.0
300-58966-142 BANK-CR CARD-PMT PROCESS'G FEE	.00	1,293.89	.00	(1,293.89)	.0
300-58966-160 SEWER OPERATING SUPPLIES	.00	.00	500.00	500.00	.0
300-58966-170 SEWER UTILITIES - NATURAL GAS	294.65	751.47	3,750.00	2,998.53	20.0
300-58966-171 SEWER UTILITIES - ELECTRIC	1,664.30	3,230.69	14,200.00	10,969.31	22.8
300-58967-000 SEWER FUEL EXPENSE	27.34	376.27	1,780.00	1,403.73	21.1
300-58967-150 SEWER VEHICLE REPAIRS & MAINT	.00	.00	3,000.00	3,000.00	.0
300-58968-150 SEWER COLLECTION SYSTEM MAINT	.00	.00	10,000.00	10,000.00	.0
300-58969-130 SEWER IT EXPENSE	.00	.00	650.00	650.00	.0
300-58969-160 SEWER OFFICE SUPPLIES	480.75	617.09	10,000.00	9,382.91	6.2
300-58969-200 SEWER TELEPHONE	253.46	942.82	1,000.00	57.18	94.3
300-58970-000 SEWER ATTORNEY EXPENSE	.00	.00	925.00	925.00	.0
300-58972-000 SEWER AUDIT EXPENSE	.00	700.00	12,000.00	11,300.00	5.8
300-58973-000 SEWER ENGINEERING EXPENSE	.00	.00	2,000.00	2,000.00	.0
300-58974-000 SEWER INSURANCE EXPENSE	.00	30,453.00	33,000.00	2,547.00	92.3
300-58975-190 SEWER TRAINING EXPENSE	.00	.00	250.00	250.00	.0
300-58976-000 SEWER REPLACEMENT FUND	.00	.00	8,000.00	8,000.00	.0
300-58980-300 WALCOMET SEWERAGE EXPENSES	.00	61,106.32	760,000.00	698,893.68	8.0
300-58980-310 WALCOMET SEWER CONNECTION FEE	.00	4,230.00	71,910.00	67,680.00	5.9
300-58996-000 I & I REDUCTION	.00	.00	20,000.00	20,000.00	.0
TOTAL SEWER UTILITY	662,674.14	948,211.11	1,319,445.00	371,233.89	71.9
TOTAL FUND EXPENDITURES	662,674.14	948,211.11	1,319,445.00	371,233.89	71.9
NET REVENUE OVER EXPENDITURES	(356,464.04)	(622,288.57)	400.00	622,688.57	(15557

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

LIBRARY FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>TAXES</u>					
400-41100	TAX LEVY-LIBRARY FUND	166,373.27	166,373.27	250,314.00	83,940.73	66.5
	TOTAL TAXES	166,373.27	166,373.27	250,314.00	83,940.73	66.5
	<u>COMMERCIAL</u>					
400-48004	INTEREST ON BANK ACCOUNT	592.75	1,627.06	7,602.00	5,974.94	21.4
400-48905	COUNTY REIMBURSEMENT	54,967.09	54,967.09	108,912.00	53,944.91	50.5
400-48955	DONOR DESIGNATED FUNDS	.00	150.00	.00	(150.00)	.0
400-48960	MISC REV FOR BOARD COMMITMENT	.00	719.79	.00	(719.79)	.0
	TOTAL COMMERCIAL	55,559.84	57,463.94	116,514.00	59,050.06	49.3
	TOTAL FUND REVENUE	221,933.11	223,837.21	366,828.00	142,990.79	61.0

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

LIBRARY FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>LIBRARY OPERATIONS</u>					
400-58100-110 LIBRARY WAGES	15,781.44	47,028.88	209,377.00	162,348.12	22.5
400-58100-121 LIBRARY FICA	1,047.02	3,184.12	16,733.00	13,548.88	19.0
400-58100-122 LIBRARY RETIREMENT	821.33	2,506.03	11,515.00	9,008.97	21.8
400-58100-123 LIBRARY HEALTH INSURANCE	3,937.98	11,906.71	41,178.00	29,271.29	28.9
400-58100-124 LIFE INSURANCE	49.39	49.39	.00	(49.39)	.0
400-58100-127 LIBRARY HSA FUNDING	781.24	1,562.48	10,000.00	8,437.52	15.6
400-58100-130 LIBRARY EQUIPMENT	628.29	628.29	1,200.00	571.71	52.4
400-58100-135 LIBRARY ADMINISTRATION COSTS	.00	344.60	9,400.00	9,055.40	3.7
400-58100-150 LIBRARY BLDG REPAIRS & MAINT	.00	.00	2,000.00	2,000.00	.0
400-58100-155 LIBRARY BLDG SUPPLIES	28.40	442.89	1,000.00	557.11	44.3
400-58100-160 LIBRARY SUPPLIES	436.61	1,061.74	3,500.00	2,438.26	30.3
400-58100-161 LIBRARY POSTAGE	.00	.00	125.00	125.00	.0
400-58100-170 LIBRARY BLDG GAS	608.81	1,645.28	2,700.00	1,054.72	60.9
400-58100-171 LIBRARY BLDG ELECTRIC	605.28	1,286.58	6,800.00	5,513.42	18.9
400-58100-173 LIBRARY BLDG WATER & SEWER	.00	168.22	900.00	731.78	18.7
400-58100-175 JANITORIAL SERVICES	.00	1,350.00	6,800.00	5,450.00	19.9
400-58100-200 LIBRARY TELEPHONE	685.13	1,646.88	3,500.00	1,853.12	47.1
400-58200-000 ADULT PRINT	1,089.91	1,633.46	10,000.00	8,366.54	16.3
400-58200-100 ADULT DIGITAL	.00	.00	2,000.00	2,000.00	.0
400-58201-000 CHILDREN PRINT	282.58	282.58	3,500.00	3,217.42	8.1
400-58201-100 CHILDREN DIGITAL	.00	.00	2,000.00	2,000.00	.0
400-58210-000 LIBRARY TRAINING & CONFERENCES	.00	97.00	2,000.00	1,903.00	4.9
400-58220-000 LIBRARY MILEAGE EXPENSE	.00	.00	400.00	400.00	.0
400-58230-000 PROGRAMS FOR CHILDREN & ADULTS	238.97	448.98	3,000.00	2,551.02	15.0
400-58232-000 LIBRARY CRAFTS	76.50	173.68	500.00	326.32	34.7
400-58240-000 LIBRARY MEDIA	133.45	418.29	3,500.00	3,081.71	12.0
400-58280-000 LIBRARY PERIODICALS	1,353.31	1,887.09	2,100.00	212.91	89.9
400-58310-000 IT/LICENSES/CONTRACTED SERVICE	8,432.61	9,493.16	12,600.00	3,106.84	75.3
TOTAL LIBRARY OPERATIONS	37,018.25	89,246.33	368,328.00	279,081.67	24.2
TOTAL FUND EXPENDITURES	37,018.25	89,246.33	368,328.00	279,081.67	24.2
NET REVENUE OVER EXPENDITURES	184,914.86	134,590.88	(1,500.00)	(136,090.88)	8972.7

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

LIBRARY DONATION FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>COMMERCIAL</u>					
410-48925 FRIENDS REVENUE	.00	66.70	4,000.00	3,933.30	1.7
410-48940 LIBRARY CHAPIN REVENUE	.00	.00	4,000.00	4,000.00	.0
410-48955 DONOR DESIGNATED FUNDS	.00	.00	5,000.00	5,000.00	.0
410-48960 MISC REV FOR BOARD COMMITMENT	160.00	160.00	4,000.00	3,840.00	4.0
TOTAL COMMERCIAL	160.00	226.70	17,000.00	16,773.30	1.3
TOTAL FUND REVENUE	160.00	226.70	17,000.00	16,773.30	1.3

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

LIBRARY DONATION FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>LIBRARY OPERATIONS</u>					
410-58200-101	BOOKS, BOARD COMMITTED	.00	.00	200.00	200.00	.0
410-58200-102	BOOKS, DONOR DESIGNATED	35.84	35.84	100.00	64.16	35.8
410-58240-101	MEDIA, DONOR DESIGNATED	.00	.00	200.00	200.00	.0
410-58250-000	LIBRARY CHAPIN EXPENDITURES	2,000.00	2,516.00	4,000.00	1,484.00	62.9
410-58255-000	LIB FRIENDS EXPENDITURE	332.78	1,012.82	4,000.00	2,987.18	25.3
410-58330-000	DONOR DESIGNATED EXPENDITURES	515.70	619.06	2,500.00	1,880.94	24.8
410-58340-000	BOARD COMMITTED EXPENDITURES	256.63	718.91	2,500.00	1,781.09	28.8
	TOTAL LIBRARY OPERATIONS	3,140.95	4,902.63	13,500.00	8,597.37	36.3
	TOTAL FUND EXPENDITURES	3,140.95	4,902.63	13,500.00	8,597.37	36.3
	NET REVENUE OVER EXPENDITURES	(2,980.95)	(4,675.93)	3,500.00	8,175.93	(133.6)

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

FIREWORKS FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>INTERGOVERNMENTAL</u>					
415-42004	FIREWORKS DONATION	50.00	50.00	25,000.00	24,950.00	.2
	TOTAL INTERGOVERNMENTAL	50.00	50.00	25,000.00	24,950.00	.2
	<u>MISCELLANEOUS</u>					
415-49200	OPERATING TRANSFER IN	.00	.00	4,580.00	4,580.00	.0
	TOTAL MISCELLANEOUS	.00	.00	4,580.00	4,580.00	.0
	TOTAL FUND REVENUE	50.00	50.00	29,580.00	29,530.00	.2

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

FIREWORKS FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
<u>FIREWORKS EXPENDITURE</u>					
415-52320-165 FIREWORKS EXPENDITURE	.00	.00	25,000.00	25,000.00	.0
TOTAL FIREWORKS EXPENDITURE	.00	.00	25,000.00	25,000.00	.0
TOTAL FUND EXPENDITURES	.00	.00	25,000.00	25,000.00	.0
NET REVENUE OVER EXPENDITURES	50.00	50.00	4,580.00	4,530.00	1.1

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

CAPITAL PROJECTS FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>COST CATEGORY 50</u>					
500-50000-140	CAPITAL IMPROVEMENTS	.00	292.50	.00	(292.50)	.0
	TOTAL COST CATEGORY 50	.00	292.50	.00	(292.50)	.0
	<u>COST CATEGORY 52</u>					
500-52120-140	POLICE DEPT CAPITAL OUTLAY	.00	4,552.00	.00	(4,552.00)	.0
	TOTAL COST CATEGORY 52	.00	4,552.00	.00	(4,552.00)	.0
	<u>COST CATEGORY 54</u>					
500-54310-131	CAPITAL PROJECT - TR TENNIS CT	.00	53,987.36	.00	(53,987.36)	.0
500-54310-140	STREETS/HIGHWAYS	.00	216.42	.00	(216.42)	.0
	TOTAL COST CATEGORY 54	.00	54,203.78	.00	(54,203.78)	.0
	<u>COST CATEGORY 55</u>					
500-55410-140	PARKS/CAPITAL OUTLAY	38,844.00	38,844.00	.00	(38,844.00)	.0
	TOTAL COST CATEGORY 55	38,844.00	38,844.00	.00	(38,844.00)	.0
	TOTAL FUND EXPENDITURES	38,844.00	97,892.28	.00	(97,892.28)	.0
	NET REVENUE OVER EXPENDITURES	(38,844.00)	(97,892.28)	.00	97,892.28	.0

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

CAPITAL EQUIPMENT FUND

		<u>PERIOD ACTUAL</u>	<u>YTD ACTUAL</u>	<u>BUDGET</u>	<u>UNEXPENDED</u>	<u>PCNT</u>
	<u>COST CATEGORY 52</u>					
550-52120-140	POLICE CAPITAL OUTLAY	<u>.00</u>	<u>6,961.00</u>	<u>.00</u>	<u>(6,961.00)</u>	<u>.0</u>
	TOTAL COST CATEGORY 52	<u>.00</u>	<u>6,961.00</u>	<u>.00</u>	<u>(6,961.00)</u>	<u>.0</u>
	TOTAL FUND EXPENDITURES	<u>.00</u>	<u>6,961.00</u>	<u>.00</u>	<u>(6,961.00)</u>	<u>.0</u>
	NET REVENUE OVER EXPENDITURES	<u>.00</u>	<u>(6,961.00)</u>	<u>.00</u>	<u>6,961.00</u>	<u>.0</u>

VILLAGE OF WILLIAMS BAY
REVENUES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

DEBT SERVICE FUND

		PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
	<u>TAXES</u>					
600-41100	TAX LEVY-DEBT SERVICE	598,163.19	598,163.19	899,956.00	301,792.81	66.5
	TOTAL TAXES	598,163.19	598,163.19	899,956.00	301,792.81	66.5
	TOTAL FUND REVENUE	598,163.19	598,163.19	899,956.00	301,792.81	66.5

VILLAGE OF WILLIAMS BAY
EXPENDITURES WITH COMPARISON TO BUDGET
FOR THE 3 MONTHS ENDING MARCH 31, 2025

DEBT SERVICE FUND

	PERIOD ACTUAL	YTD ACTUAL	BUDGET	UNEXPENDED	PCNT
600-59160-000 PRINCIPAL-DEBT SERVICE LTD	.00	660,657.34	660,174.00	(483.34)	100.1
600-59260-000 INTEREST-DEBT SERVICE LTD	.00	124,900.50	239,582.00	114,681.50	52.1
600-59360-000 DEBT SERVICE MISC EXPENSE	.00	.00	200.00	200.00	.0
TOTAL COST CATEGORY 59	.00	785,557.84	899,956.00	114,398.16	87.3
TOTAL FUND EXPENDITURES	.00	785,557.84	899,956.00	114,398.16	87.3
NET REVENUE OVER EXPENDITURES	598,163.19	(187,394.65)	.00	187,394.65	.0

May 13, 2025 Plan Commission Motions

APPLICANT: Dancing Dudes, LLC (Owner)

TAX KEYS: WOP 00088

STREET ADDRESS: 77 N. Walworth AVE, Williams Bay, WI 53191

The applicant requests a recommendation to the Village Board for approval for architectural design alteration to remove the window shutters and to have black metal awnings on the upper level and black cloth awnings on the lower level.

The motion to recommend Village Board approval of the addition of a door on the north side of the building as it has already been constructed was initiated by Trustee Wright and seconded by Commissioner Gage. Commissioner Klemke No. Motion Carries.

The motion to recommend Village Board approval of a 90-day architectural review on whether or not the shutters will or will not be included was initiated by Commissioner Robbins and seconded by Commissioner Gage. Commissioner Fieweger No. Motion Carries.

The motion to recommend Village Board approval of the drawings as presented on May 13, 2025 showing black metal awnings on the second-floor north & west faces and black fabric awnings on the lower level facing north & west faces of the building and window boxes as shown in the drawing was initiated by Trustee Wright and seconded by Commissioner Robbins. Unanimously Carried.

The motion to recommend Village Board approval of a written opinion as to the feasibility as to the shielding of the vent from the North side of the property was initiated by Trustee Wright and seconded by Commissioner Gage.

The motion to recommend Village Board approval of a written opinion as to the feasibility as to the shielding of the vent from the North side of the property was amended to include a 90-day timeframe was initiated by President Jaramillo and seconded by Commissioner Gage. Unanimously Carried.



VILLAGE OF WILLIAMS BAY

Office of Zoning Administrator

Evaluation Report Plan Commission Meeting May 13, 2025

May 6, 2025

APPLICANT: Dancing Dudes, LLC (Owner)

TAX KEY: WOP 00088

STREET ADDRESS: 77 N. Walworth Ave, Williams Bay, WI 53191

The applicant requests a recommendation to the Village Board for approval for architectural design alteration to remove the window shutters and to have black metal awnings on the upper level and black cloth awnings on the lower level.

The original approved building elevations are included in the agenda packet. Almost all the upper floor windows had copper window heads (awnings) and white shutters.

Section 390-0705.C(2)(d) Village Center Overlay Sign Design Standards:

Design of signage.

[1] Signs shall be constructed of materials consistent with the architecture of the building.

[2] Internally lit signage is prohibited.

[3] Awning signs shall be discouraged.

[4] On buildings with more than one story, wall signage shall not be placed above the ground floor.

[5] Window signage shall not cover more than 30% of any ground floor window or more than 10% of any upper-story window.

[6] Freestanding pole signs shall be prohibited. All freestanding signs shall be monument-style signs.

[7] Roof signs are prohibited.

Respectfully submitted,

Bonnie M. Schaeffer
Zoning Administrator







Black
Canvas
Awning #1
#2
#3
Green Grocer & Deli
Clear Waters Salon Med Spa
Chocolaterie Colibri

Second Floor Awnings
* Black Metal

May 5, 2025

Village Board Meeting Public Comments – Follow-up Answers

05/15/2025

The following answers have been provided directly to the individuals that asked the questions during the Public Comments portion of the Village Board Meeting.

1Q. Pamela Russell, 24 Lower Loch Vista, Spoke regarding a dangerous bridge on the shore path near the Lake Geneva Youth Camp

1A. This item was relayed to the Village Code Enforcement Officer. The building (Ross) and property will be inspected with further report to follow.

2Q. Connie Gluth, 95 Lackey, spoke regarding traffic problems near the boat launch. Gluth gave several suggestions, such as a staging area where they could telephone in to check congestion or a buzzer system letting them know it was their turn.

2A. The Streets and Highways Committee and Parks and Recreation Committee have previously identified this as an issue and will be reviewing further in the coming months.

3Q. Tim Halas, 134 Elmhurst Ct, spoke regarding the need to keep tight control on Topography, and requested that we request a calendar of events and major milestones from them to help with this.

3A. Topography has been asked to provide a schedule for The Preserve Project. Topography has been working on their Final Development Plan (FDP) since approval of the Preliminary Development Plan and anticipates submittal of their FDP in the coming month or two. Topography closed on the purchase of the property in April and as owners of the property they began preparing for the development in anticipation of approval of the FDP. Once the FDP is approved, Village staff will host a pre-construction meeting and Topography will provide a construction schedule that will be made public.

The following question was raised by Resident Tim Halas during the April 21, 2025 Village Board Meeting and the following interim response was provided during the May 5, 2025 Village Board Meeting.

4Q. Village interpretation of “Single Family” zoning in regards to whether a business can legally purchase a property and use exclusively for rental?

Response provided as part of May 5, 2025 Board Packet.

4AI. Village Attorney Mark Schroeder and Zoning Administrator Bonnie Schaefer have a call scheduled for discussing this question next week. Further update to be provided.

4AII. Village Attorney Mark Schroeder provided the enclosed legal opinion memo “Single Family Zoning Question” dated 05/12/2025. The memo concludes “that the village does not have a legal or practical way to prohibit a commercial entity, such as a corporation or LLC, from owning a single-family residential structure for use as a short-term rental. Building, Zoning and Ordinance Committee will be reviewing the existing ordinances and enforcement in the coming months.

Memo

To: Dave Lothspeich
Cc: Bonnie Schaeffer
From: Attorney Mark Schroeder
Date: May 14, 2025
Re: Single Family Zoning Question

Dave,

I understand that a question has been asked concerning the situation where a zoning district allows single-family as a permitted use. Specifically, I understand the question is whether, in that scenario, a business can purchase a property on which there is a single-family structure and use it exclusively for short-term rental purposes. In my review of the zoning ordinance, I find that the focus is on the use of the property. If single-family is a permitted use, it does not distinguish between owner occupied single-family use and renter single-family use. Neither does the zoning ordinance distinguish between forms of ownership of the single-family structure. Whether the single-family structure is owned by a business or natural person, the focus is again on the use of the structure.

This is consistent with Wis. Stat. Sec. 62.23 (7) (am-c), which is the statutory authority for municipalities to enact zoning ordinances. Pursuant to that statute, it is the ability to regulate the use of property which is authorized. Specifically, in reference to short-term rentals, Wis. Stat. Sec. 66.1014 prohibits certain limits on the rental of residential dwellings. In subsection (2) (a), it provides that a municipality may not enact or enforce an ordinance that prohibits the rental of a residential dwelling for 7 consecutive days or longer. It does not provide that a municipality may determine the form of ownership of a residential dwelling which is being offered for rent. Short-term rentals are defined as rentals of residential dwellings for 7 to 29 consecutive days. As a result, if the village were to enact an ordinance not allowing a commercial entity, such as a corporation or LLC, to own a short-term rental, that ordinance would be prohibiting the rental of a residential dwelling for 7 consecutive days or longer in violation of Wis. Stat. Sec. 66.1014.

Based upon the above, it is my opinion that the village does not have a legal or practical way to prohibit a commercial entity, such as a corporation or LLC, from owning a single-family residential structure for use as a short-term rental.

ORDINANCE #2025-O-05
AN ORDINANCE REPEALING SECTION 281-12
OF THE CODE OF ORDINANCES OF THE VILLAGE OF WILLIAMS BAY
REGARDING LAUNCH AREA USE

WHEREAS, the State of Wisconsin NR 1.91, Wis. Adm. Code allows Municipalities, lake management districts, and other access providers to operate public boat launch facilities; and

WHEREAS, the Village of Williams Bay operates a public boat launch facility; and

WHEREAS, the State of Wisconsin Department of Natural Resources prohibits municipalities from establishing separate Commercial Use Launch Fees; and

WHEREAS, the Building, Zoning and Ordinance Committee having recommended to the Village Board the adoption of an ordinance repealing Section 281-12 concerning Commercial Use of the Williams Bay Boat Launch ; and

WHEREAS, the Village Board having considered the recommendation of the Building, Zoning and Ordinance Committee and having determined that it is appropriate to repeal Section 281-12 of the Code of Ordinances.

NOW, THEREFORE, the Village Board of the Village of Williams Bay do hereby ordain as follows:

Section I. Section 281-12 of the Code of Ordinances of the Village of Williams Bay is hereby repealed as follows:

- A. ~~Definition. As used in this section, the following terms shall have the meanings indicated:~~

~~COMMERCIAL USE~~

~~A person using the Village municipal launch parking area and ramp as a bulk handling area for the loading or unloading of construction materials from or onto watercraft from or to locations on Geneva Lake. Such construction materials include building materials, landscaping materials, earth, stone, timbers and pier lumber.~~

- B. ~~Prohibition. The commercial use of the Williams Bay municipal launch parking area and ramp is prohibited without a permit. Every person shall obtain a permit prior to the commercial use of the Williams Bay municipal launch parking area and ramp. The permit shall be issued by the Village Clerk upon the filing of an application and the payment of the required fee. The application shall contain the name, address, telephone number and gross vehicle weight of the loads proposed to be handled by the commercial user at the municipal launch parking area and ramp. The exact nature, including the weight thereof, of the construction materials to be handled and the equipment proposed for handling, must be stated in the application. The applicant is responsible for all damage to the Village owned launch facility, including the pier, parking lot and ramp area, and will promptly reimburse the Village for any such damages. Any violation of this provision will result in the immediate revocation of the permit, the commencement of an action for the imposition of the standard penalties of this Code and/or an action for money damages.~~

Section II. This ordinance shall take effect upon passage and publication as provided by law.

Approved by the Village Board of the Village of Williams Bay this 19th day of May, 2025.

VILLAGE OF WILLIAMS BAY

By: _____
Adam Jaramillo, President

Attest: _____
Tina Kolls, Clerk

First Reading: 05/05/2025
Second Reading: 05/19/2025
Date Adopted: 05/19/2025
Date Published: _____

*Village of Williams Bay, WI
Monday, April 28, 2025*

Chapter 281. Public Waters and Beaches

Article I. General Regulations

§ 281-12. Commercial use of launch area.

- A. Definition. As used in this section, the following terms shall have the meanings indicated:

COMMERCIAL USE

A person using the Village municipal launch parking area and ramp as a bulk handling area for the loading or unloading of construction materials from or onto watercraft from or to locations on Geneva Lake. Such construction materials include building materials, landscaping materials, earth, stone, timbers and pier lumber.

- B. Prohibition. The commercial use of the Williams Bay municipal launch parking area and ramp is prohibited without a permit. Every person shall obtain a permit prior to the commercial use of the Williams Bay municipal launch parking area and ramp. The permit shall be issued by the Village Clerk upon the filing of an application and the payment of the required fee. The application shall contain the name, address, telephone number and gross vehicle weight of the loads proposed to be handled by the commercial user at the municipal launch parking area and ramp. The exact nature, including the weight thereof, of the construction materials to be handled and the equipment proposed for handling, must be stated in the application. The applicant is responsible for all damage to the Village-owned launch facility, including the pier, parking lot and ramp area, and will promptly reimburse the Village for any such damages. Any violation of this provision will result in the immediate revocation of the permit, the commencement of an action for the imposition of the standard penalties of this Code and/or an action for money damages.

COMMERCIAL BOAT LAUNCH FEE ISSUE LEGAL OPINION SUMMARY

If the Department has not provided any grants for acquisition, development, maintenance or improvement to this public boat launch site, the entity owning this launch site does not need to comply with s. NR 1.91, Wis. Admin Code, unless the site is subject to another WDNR decision of the types cited in s. NR 1.91(1), Wis. Admin. Code. Department funding, permitting, and/or any other type of Department decision can be the event that makes s. NR 1.91, Wis. Admin. Code requirements apply to a public boating access site, and if s. NR 1.91, Wis. Admin. Code applies, the site owner may only impose launch fees as allowed under s. NR 1.91(11), Wis. Admin. Code for any parking and access area subject to s. NR 1.91, Wis. Admin. Code. If residents launch for free, everyone must be able to launch for free; if residents pay a launch fee, the launch fee for non-residents may not be more than 150% of the resident's fee.

The Department has determined that municipalities owning public boat launches cannot charge higher fees based on whether the person using the public boat ramp is engaging in recreational or commercial uses. Commercial as well as noncommercial use of navigable waters is protected by the Public Trust doctrine. S. 30.77, Stats., and s. NR 1.91(11), Wis Admin. Code do not distinguish between commercial and non-commercial users or allow surcharges based on commercial use of the launch. s. NR 1.91(11)(e) limits launch fees for all users to the base fee and any Department approved surcharges and differential residency fee specified in s. NR 1.91(11). Whether the person using the launch is engaged in commercial or recreational use is not included in the rule as a consideration for higher fees or a surcharge.

A municipality owning a public boat launch cannot flatly prohibit a member of the public from using the public boat launch for commercial purposes; however, the municipality may develop policies that help them manage the facility more effectively; for example, limiting the time a boat is moored at the launch, use of open rental slips, or charging for damage or excessive wear and tear. S. 30.77, Stats., and ss. NR 1.90 and 1.91, Wis. Admin. Code regulate public access to navigable waters, and apply only to the public access facilities (the boat launch, launch piers, and associated parking and restrooms, etc.). Department authority to regulate public access does not extend to use and operation of the marina (boat slips, piers) and other facilities (marina parking area, etc.). A municipality may also set policy as to whether a person who owns several boats could launch all his or her boats with one season pass or if a season pass would be needed for each boat. S. NR 1.91 does not specifically address this issue, but the Department believes that a "one pass/one boat" (like state park season windshield stickers) requirement is probably simplest to administer.

If the commercial use essentially monopolizes the site for a period of time, then the launch owner may want to consider drafting an ordinance that provides for time limits or time-of day restrictions or that charges the user a reasonable fee for rental of the site. We believe that a launch owner may also be able to prohibit use of the boat ramp by boats above a specified weight and/or size if it is clear that those boats would cause damage or excessive wear and tear on the boat ramp. We believe that this is an allowable restriction to prevent damage to the property. Since such restrictions could be challenged, any ordinance imposing limits or rental fees should be supported by facts showing that the regulations are reasonably necessary to address public health or safety concerns or to protect against harm to public property. Launch owners considering such restrictions should consult with their counsel and should consider making the restrictions as least restrictive as possible to alleviate or minimize the impact on the access to navigable waters. The restrictions should be consistent with statute and code, consistently applied to all users, and not a significant deterrent to those who are entitled to access the public waters of Wisconsin.

MAXIMUM DAILY BOAT LAUNCHING FEES INLAND WATER FACILITIES

- A. Base Fee = \$13.00**
B. Surcharge for facility with restroom or attendant = \$13.00 x 0.20 = \$2.60
C. Surcharge for facility with restroom and attendant = \$13.00 x 0.40 = \$5.20
D. Surcharge for boat length of at least 20 ft but less than 26 ft = .30; \$13.00 x 0.30 = \$3.90
E. Surcharge for boat length of 26 ft or greater = .60; \$13.00 x .60 = \$7.80

Non-motorized or non-trailerred watercraft (Resident and Non-resident boaters):

Base Fee	\$13.00
Base Fee plus tax*	\$13.75 (A + \$0.72 (tax on \$13.00) = \$13.72, rounded to nearest \$0.25)

Motorized Boats under 20 feet in length (the following fees cover both Resident and Non-resident boaters. Fees for non-residents may not exceed 150% of the fee charged a resident, and non-resident fees may not exceed the maximum allowable amounts for boats under 20 feet):

Base Fee	\$13.00
Base Fee plus tax*	\$13.75 (A + \$0.72 (tax on \$13.00) = \$13.72, rounded to nearest \$0.25)
Facility w/restroom or attendant	\$15.50 (A + B = \$15.60, rounded to nearest \$0.25)
Facility w/restroom or attendant plus tax*	\$16.25 (\$15.50 + \$0.85 (tax on \$15.50) = \$16.35, rounded to nearest \$0.25)
Facility w/restroom and attendant	\$18.25 (A + C = \$18.20, rounded to nearest \$0.25)
Facility w/restrooms and attendant, plus tax*	\$19.25 (\$18.20 + \$1.00 (tax on \$18.25, rounded to nearest \$0.25)

Motorized boats 20 feet in length or more but less than 26 feet (the following fees cover Resident boaters only; non-residents can be charged a differential fee of 150% for this category of boats):

Base Fee plus length surcharge	\$17.00 (A + D = \$16.90, rounded to nearest \$0.25); Non-resident = \$25.50
Base Fee plus length surcharge plus tax*	\$18.00 (\$17.00 + \$0.94 (tax on \$17.00) = \$17.94, rounded to nearest \$0.25); Non-resident = \$27.00
Facility w/restroom or attendant	\$19.50 (A + D + B = \$19.50); Non-resident = \$29.25
Facility w/restroom or attendant plus tax*	\$20.50 (\$19.50 + \$1.07 (tax on \$19.50) = \$20.57, rounded to nearest \$0.25); Non-resident = \$30.75
Facility w/restroom and attendant	\$22.00 (A + D + C = \$22.10, rounded to nearest \$0.25); Non-resident = \$33.00
Facility w/restrooms and attendant, plus tax*	\$23.25 (\$22.00 + \$1.21 (tax on \$22.00) = \$23.21, rounded to nearest \$0.25); Non-resident = \$35.00

Motorized boats 26 feet in length or greater (the following fees cover Resident boaters only; non-residents can be charged a differential fee of 150% for this category of boats)

Base Fee plus length surcharge	\$20.75 (A + E = \$20.80); Non-resident = \$31.25
Base Fee plus length surcharge plus tax*	\$22.00 (\$20.75 + \$1.14 (tax on \$20.75) = \$21.89, rounded to nearest \$0.25); Non-resident = \$33.00
Facility w/restroom or attendant	\$23.50 (A + E + B = \$23.40, rounded to nearest \$0.25); Non-resident = \$35.25
Facility w/restroom or attendant plus tax*	\$24.75 (\$23.50 + \$1.29 (tax on \$23.50) = \$24.79, rounded to nearest \$0.25); Non-resident = \$37.25
Facility w/restroom and attendant	\$26.00 (A + E + C = \$26.00); Non-resident = \$39.00
Facility w/restrooms and attendant, plus tax*	\$27.50 (\$26.00 + \$1.43 (tax on \$26.00) = \$27.43, rounded to nearest \$0.25); Non-resident = \$41.25

Notes about launch fees:

- The governance of launch fees is covered under section NR 1.91(11), Wisconsin Administrative Code.
- All fees listed above use the \$13.00 maximum Base Fee, which is based on the \$13.00 Daily State Park Entrance Fee for residents. You may set a lower Base Fee based upon local needs. Section NR 1.91(11)(e), Wis. Adm. Code, requires that before a local unit of government adopts a launch fee exceeding \$13.00, it must submit the fee schedule to the Department for approval and include information demonstrating: (a) that it maintains facilities or services that justify charges exceeding \$13.00; and (b) that season passes are available.
- Fees collected by the operating authority shall be used only for the operation and maintenance of boat launching facilities.
- Parking is included with launch fees. Segregated fees for parking are not allowed.
- If a launch fee is charged, a season pass at a fee not to exceed 10 times the daily fee shall be provided for both residents and non-residents.
- In no case shall the fee charged a non-resident exceed 150% of the fee charged a resident.

*Tax rate used = 5.5%. An operating authority may incorporate the state sales tax into the Base Fee for ease of collection, but must pay the sales taxes collected to the Wisconsin Department of Revenue.



MEMORANDUM

DATE: MAY 19, 2025
TO: VILLAGE TRUSTEES
FROM: TINA KOLLS, VILLAGE CLERK
RE: OPERATOR LICENSE APPLICATIONS

The following License applications have been filed with the Village Clerk.

All applications have completed a records check, and PASS/FAIL suggestions have been made by the Williams Bay Police Department based on their findings.

Original/ Renewal	Name	Address	Establishment	Record Check
R	Graciela V Jaramillo	Harvard, IL	Harpoon Willies	P
R	Daisy Jane Vogel	Elkhorn, WI	Lucke's Cantina	P
R	Harley Knight	Williams Bay, WI	Lucke'S Cantina	P
O	Michaela Lynn Fletcher	Elkhorn, WI	Gage Marine	P
O	Julie Logue	Lake Geneva, WI	Gage Marine	P
O	Donald M Carlson	Williams Bay, WI	Gage Marine/Lake Geneva Cruise Line	P
O	Amy Grau	Lake Geneva, WI	Lake Geneva Cruise Line	P
O	Mia Katherine Fiscelli	Western Springs, IL	Lake Geneva Cruise Line	P
R	Taylor Julia Blair	Burlington, WI	Gage Marine	P
R	Angie Peters Jahns	Lake Geneva, WI	Gage Marine	P
R	Juliann Hecksel	Lake Geneva, WI	Gage Marine/Lake Geneva Cruise Line	P
R	Meaghan Biba	Genoa City, WI	Lake Geneva Cruise Line	P
R	Shannan Michelle Biba	Genoa City, WI	Lake Geneva Cruise Line	P
R	Nicole Cetnarowski	Mukwonago, WI	Lake Geneva Cruise Line	P
R	Barbara J Handel	Elkhorn, WI	Lake Geneva Cruise Line	P
R	Jason Aaron Hecksel	Lake Geneva, WI	Lake Geneva Cruise Line	P
R	Joel K Jacobsen	Burlington, WI	Lake Geneva Cruise Line	P
R	Roberta Lynn Jean North	Lake Geneva, WI	Lake Geneva Cruise Line	P
R	Sidney Pearl	Park Ridge, IL	Lake Geneva Cruise Line	P
R	Gregory R Pennington	Burlington, WI	Lake Geneva Cruise Line	P
R	Charles Renfro	Williams Bay, WI	Lake Geneva Cruise Line	P
R	Anastasia Sanchez	Williams Bay, WI	Lake Geneva Cruise Line	P
R	Susan Christine Schindler	Lake Geneva, WI	Lake Geneva Cruise Line	P
R	Keith Vogt	Fitchburg, WI	Lake Geneva Cruise Line	P



Proclamation

Emergency Medical Services Week

May 18 - 24, 2025

WHEREAS, emergency medical services is a vital public service; and

WHEREAS, the members of emergency medical services teams are ready to provide lifesaving care to those in need 24 hours a day, seven days a week; and

WHEREAS, access to quality emergency care dramatically improves the survival and recovery rate of those who experience sudden illness or injury; and

WHEREAS, emergency medical services fills healthcare gaps by providing important, out-of-hospital care, including preventative medicine, follow-up care, and access to telemedicine; and

WHEREAS, the emergency medical services system consists of first responders, emergency medical technicians, paramedics, emergency medical dispatchers, firefighters, police officers, educators, administrators, pre-hospital nurses, emergency nurses, emergency physicians, trained members of the public, and other out of hospital medical care providers; and

WHEREAS, the members of emergency medical services teams, whether career or volunteer, engage in thousands of hours of specialized training and continuing education to enhance their lifesaving skills; and

WHEREAS, it is appropriate to recognize the value and the accomplishments of emergency medical services providers by designating the Emergency Medical Services Week; now

THEREFORE, I Adam Jaramillo, Village President of the Village of Williams Bay, in recognition of this event do hereby proclaim the week of May 18 - 24, 2025, as

EMERGENCY MEDICAL SERVICES WEEK

*The 51st anniversary of EMS Week theme is **EMS WEEK: We Care. For Everyone.** I encourage the community to observe this week with appropriate programs, ceremonies, and activities in honor of the EMS profession and the essential service it provides.*

Given under my hand and seal of the Village of Williams Bay this 19th Day of May, 2025.

Signed: _____ Attest: _____

NOTICE OF PUBLIC HEARING VILLAGE OF WILLIAMS BAY

Re: Vacation, abandonment and discontinuance of a portion of a public street and an alley in the Village of Williams Bay, Walworth County, Wisconsin, described as follows:

THAT PART OF HANSON'S ADDITION TO WILLIAMS BAY, ACCORDING TO THE RECORDED PLAT, BEING THE EAST HALF OF HANSON STREET, AND ALL OF THE 16.5 FOOT ALLEY, AS SHOWN ON HANSON'S ADDITION TO WILLIAMS BAY, DESCRIBED AS FOLLOWS:

BEGINNING AT THE SOUTHWEST CORNER OF LOT 8 OF HANSON'S ADDITION TO WILLIAMS BAY, A SUBDIVISION; THENCE SOUTH 88°57'36" WEST (RECORDED AS WEST) ALONG THE SOUTH LINE OF HANSON STREET, 33.00 FEET TO THE CENTERLINE OF HANSON STREET; THENCE NORTH 00°54'29" WEST (RECORDED AS NORTH), ALONG SAID CENTERLINE, 286.33 FEET, TO A POINT ON THE SOUTH LINE OF BAYVIEW AVE; THENCE NORTH 88°58'13" EAST 33.00 FEET, TO THE NORTHWEST CORNER OF LOT 7; THENCE SOUTH 00°54'29" EAST, ALONG THE WEST LINE OF LOT 7 134.87 FEET (RECORDED AS 135 FEET), TO THE SOUTH WEST CORNER OF LOT 7; THENCE NORTH 88°57'59" EAST 448.01 FEET (RECORDED AS 448 FEET), TO THE SOUTH EAST CORNER OF LOT 1 AND THE EAST END OF ALLY; THENCE SOUTH 00°54'49" EAST (RECORDED AS SOUTH), 16.50 FEET, TO THE NORTHEAST CORNER OF LOT 14; THENCE SOUTH 88°57'59" WEST, 448.01 FEET (RECORDED AS 448 FEET), TO THE NORTHWEST CORNER OF LOT 8; THENCE SOUTH 00°54'29" EAST 134.89 FEET (RECORDED AS 135 FEET) TO THE SOUTHWEST CORNER OF LOT 8 AND THE POINT OF BEGINNING, ALL IN THE NORTHEAST 1/4 OF THE NORTHEAST 1/4 OF SECTION 6 TOWNSHIP 1 NORTH, RANGE 17 EAST OF THE FOURTH PRINCIPAL MERIDIAN, VILLAGE OF WILLIAMS BAY, WALWORTH COUNTY, WISCONSIN.

TAX ID: WHA00001-00006

NOTICE IS HEREBY given that a petition has been filed to vacate the east half of Hanson Street and all of the 16.5 foot alley, as shown on Hanson's Addition to Williams Bay. The petition is subject to passage of a resolution by the Village Board after a public hearing scheduled for such consideration to be held on May 19, 2025, at 6:30 PM at the Village of Williams Bay Village Hall, located at 250 Williams St., Williams Bay, Wisconsin. All persons, their agents or attorneys will be given an opportunity to be heard relative thereto.

Dated this 21st day of April, 2025.

VILLAGE OF WILLIAMS BAY

By: _____
Tina Kolls, Clerk

**PETITION TO VACATE THE EAST HALF OF HANSON STREET AND ALL OF THE
16.5-FOOT ALLEY, AS SHOWN ON HANSON'S ADDITION TO WILLIAMS BAY**

TO: Village of Williams Bay
ATTN: Village Board
250 Williams Street
Williams Bay, WI 53191

The undersigned, acting as agent for Breezy Bay LLC, the owner of all the frontage of the land abutting upon the following described right of ways, requests that the Village of Williams Bay vacate those right of ways described below and that the right of ways to be discontinued be equally divided between the abutting property owners. This request is made pursuant to Wis. Stat. § 66.1003.

The right of ways sought to be discontinued are the east half of Hanson Street and the 16.5-foot wide unimproved alley, located in the Village of Williams Bay, Walworth County, Wisconsin, described more fully as follows:

THAT PART OF HANSON'S ADDITION TO WILLIAMS BAY, ACCORDING TO THE RECORDED PLAT, BEING THE EAST HALF OF HANSON STREET, AND ALL OF THE 16.5 FOOT ALLEY, AS SHOWN ON HANSON'S ADDITION TO WILLIAMS BAY, DESCRIBED AS FOLLOWS:

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TAX ID: WHA00001-00006

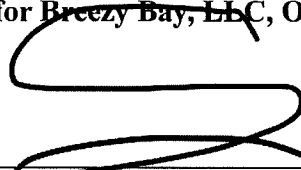
A copy of the map identifying the right of ways that are to be vacated is attached and incorporated herein as Exhibit A.

The road and alley subject to this petition, while originally platted in 1894, have never been developed or utilized as a public way. As part of the development of Hanson's Addition, the developer is providing the Village with a 50' by 35' temporary drainage and grading easement to facilitate the Geneva Street rehabilitation project and a 15' wide drainage easement on the entire length of the east side of Hanson's Addition to facilitate drainage from a wet basin to be developed by the Village on Bayview Avenue. Therefore, it is in the best interest of the Village to facilitate the development of Hanson's Addition by vacating these right of ways.

Pursuant to Wis. Stat. § 66.1003(8), notice stating when and where this petition will be acted upon and what public way and unpaved alley are proposed to be discontinued shall be published as a class three (3) notice. A copy of this petition must also be sent to the following: (1) the secretary of transportation if any of the public ways or unpaved alley that are the subject of this petition are located within one-quarter mile of a state trunk highway or connecting highway; and (2) the commissioner of railroads if there is a railroad highway crossing within the portion of the public ways that are the subject of this petition. The notices required under Wis. Stat. § 66.1003(8) shall be given prior to any action being taken by the Village Board of the Village of Williams Bay.

Dated this 15 day of April, 2025.

SG2 INVESTMENTS, LLC
as agent for ~~Breezy Bay, LLC~~, Owner

By: 
Stephen Greenberg, Its Managing Member

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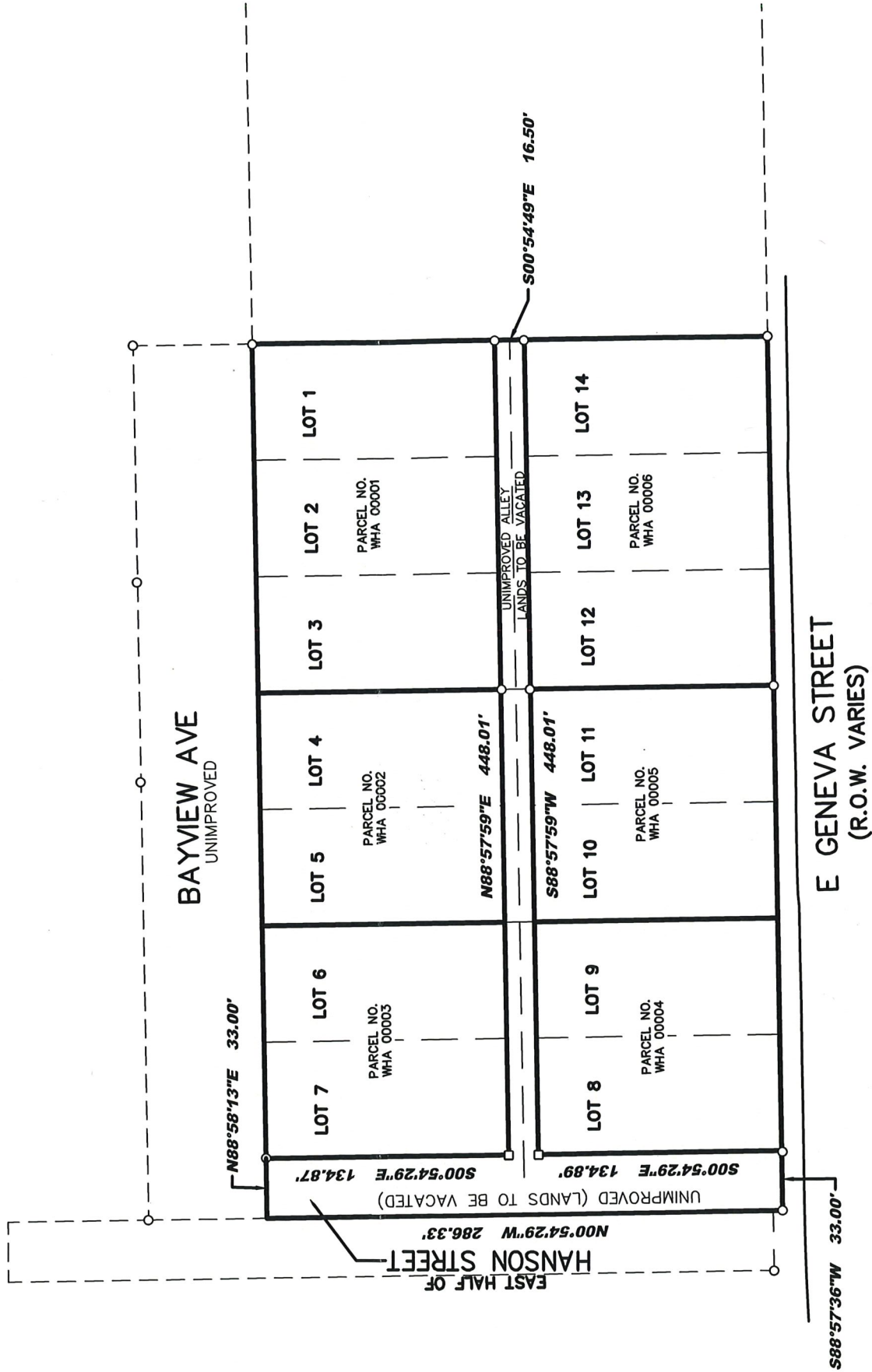
EXHIBIT
FOR
LANDS TO BE VACATED

LOCATION: WILLIAMS BAY, WISCONSIN
PREPARED FOR: CANYON DEVELOPMENT
TAX ID: WHA00001-00006



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LYNCH & ASSOCIATES
ENGINEERING CONSULTANTS, LLC
5482 S. WESTRIDGE DRIVE
NEW BERLIN, WI 53151
440 MILWAUKEE AVENUE
BURLINGTON, WI 53105
(262) 402-5040

**PETITION TO VACATE THE EAST HALF OF HANSON STREET AND ALL OF THE
16.5-FOOT ALLEY, AS SHOWN ON HANSON'S ADDITION TO WILLIAMS BAY**

TO: Village of Williams Bay
ATTN: Village Board
250 Williams Street
Williams Bay, WI 53191

The undersigned, acting as agent for Breezy Bay LLC, the owner of all the frontage of the land abutting upon the following described right of ways, requests that the Village of Williams Bay vacate those right of ways described below and that the right of ways to be discontinued be equally divided between the abutting property owners. This request is made pursuant to Wis. Stat. § 66.1003.

The right of ways sought to be discontinued are the east half of Hanson Street and the 16.5-foot wide unimproved alley, located in the Village of Williams Bay, Walworth County, Wisconsin, described more fully as follows:

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TAX ID: WHA00001-00006

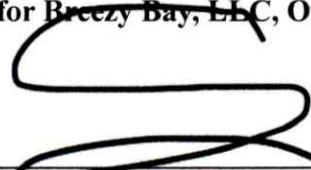
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Dated this 15 day of April, 2025.

SG2 INVESTMENTS, LLC
as agent for ~~Breezy Bay, LLC~~, Owner

By: 
Stephen Greenberg, Its Managing Member

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EXHIBIT FOR LANDS TO BE VACATED

LOCATION: WILLIAMS BAY, WISCONSIN

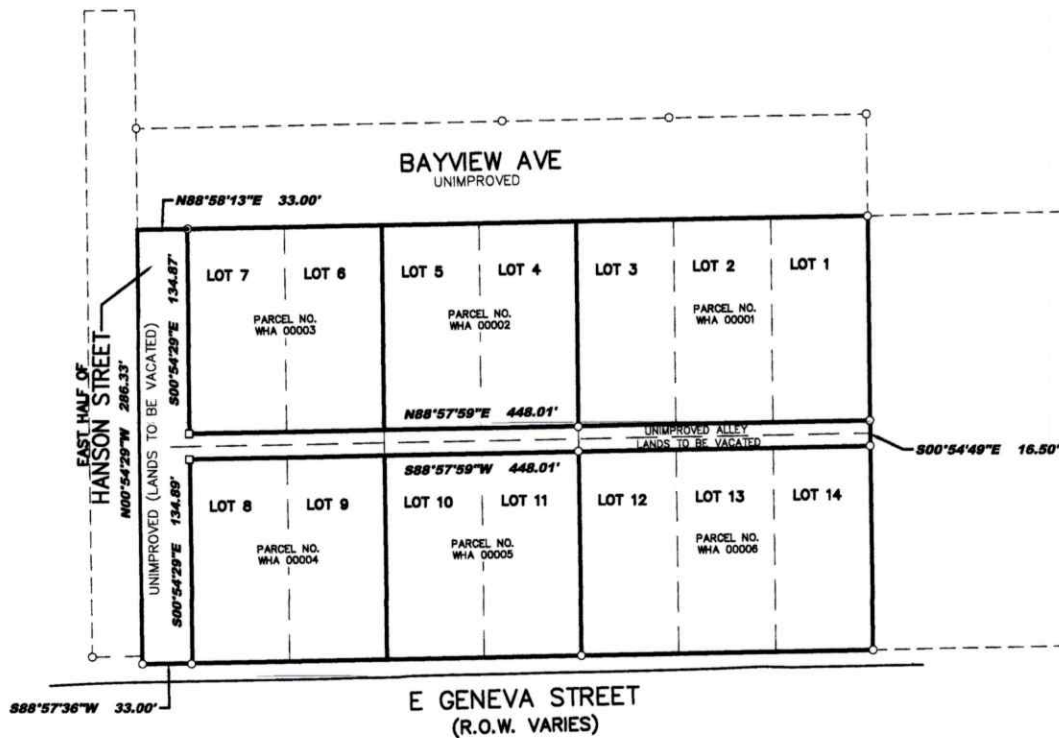
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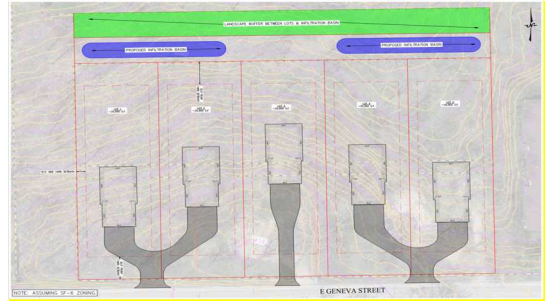


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STORM WATER MANAGEMENT PLAN



Geneva Street Development Village of Williams Bay, Wisconsin

Project Number: 23.0023
May 2025



5482 S. WESTRIDGE DRIVE
NEW BERLIN, WI 53151

TABLE OF CONTENTS

INTRODUCTION	3
DESIGN CRITERIA	3
ANALYSIS METHODS	4
PRE-DEVELOPMENT CONDITIONS	4
POST-DEVELOPMENT CONDITIONS	5
INFILTRATION	7
STORMWATER QUALITY CONTROL	7
MAINTENANCE PROGRAM	7
EROSION CONTROL	8
SOIL LOSS	8
WETLANDS/WATERWAYS	8
CONCLUSION	8

APPENDICES

APPENDIX 1 – MAPS

- Project Location Map.
- USDA SCS Soils Map.

APPENDIX 2 - PRE-DEVELOPMENT CONDITIONS

- Hydrology Exhibit – Pre-Development Conditions.
- Pre-Development HydroCAD Modeling.

APPENDIX 3 – POST-DEVELOPMENT CONDITIONS

- Hydrology Exhibit – Post-Development Conditions.
- Post-Development HydroCAD Modeling.

APPENDIX 4 - POST-DEVELOPMENT CONDITIONS (WATER QUALITY)

- WinSLAMM Modeling Input Data & Output Computations.

APPENDIX 5 - GEOTECHNICAL REPORT

- Geotechnical Report.

APPENDIX 6 – MAINTENANCE AGREEMENT

APPENDIX 7 – CONSTRUCTION DETAILS

Questions and comments can be directed to:

Ryan W. Kloth, P.E., CFM
Project Manager II.
Phone: 262.402.5038
rkloth@lynch-engineering.com

5482 S. Westridge Drive
New Berlin, WI 53151
www.lynch-engineering.com

INTRODUCTION

The proposed Geneva Street Development is located on the north side of East Geneva Street, specifically, it is across the road from the intersection with Johnson Terrace. Please see **Appendix 1** for a Location Map for the project. The proposed development encompasses a total of approximately 4 acres which includes:

- 5 lots, each, 0.64 acres in size.
- 3,000 square foot residential units on each lot.
- A Stormwater and Maintenance Easement.
- A proposed Utility Easement.
- A proposed Wet-Detention Basin along the north end of the property.

The following report summarizes the Stormwater Quantity calculations for Pre-Development and Post-Development Conditions, and Stormwater Quality.

DESIGN CRITERIA

Village of Williams Bay

For Post-Construction Stormwater Management, the Village of Williams Bay follows the Model Ordinance from the Wisconsin Department of Natural Resources (WDNR) Bureau of

Watershed Management Program Guidance# 3800-2015-05 with an effective date of April 2015. Specifically for Stormwater Quantity and Quality:

Stormwater Quantity: BMPs shall be employed to maintain or reduce the Post-Development 2, 10, and 100-year peak runoff discharge rates to the 2, 10, and 100-year Pre-Development peak runoff discharge rates, respectively, or to the maximum extent practicable.

Stormwater Quality: Best management practices shall be designed, installed, and maintained to control total suspended solids carried in runoff from the post-construction site as follows: For new development, by design, to reduce the maximum extent practicable, the total suspended solids load by 80%, based on an average annual rainfall, as compared to no runoff management controls.

Stormwater Infiltration: The Village does not list infiltration requirements within Section 15.09 of their ordinances, the Village uses applicable regulations and standards from the WDNR or Walworth County. One of the following Walworth County infiltration requirements must be met for this development:

- Infiltrate 60% of the Pre-Development infiltration volume; based on the annual rainfall.
- Infiltrate 25% of the Post-Development runoff volume calculated for the 2-year 24-hour design storm.
- Establish an effective infiltration area covering 2% or more of the project site.

ANALYSIS METHODS

Hydrology calculations have been performed using HydroCAD, Version 10.00-15 which uses the accepted TR-20 methodology for computing hydrographs and determining peak flow rates. Hydrologic Curve Numbers for the Pre-Development and Post-Development land use were selected using the standard values specified in TR-55 for a HSG “B” soil type. Peak flow rates have been calculated for the 2-, 10-, and 100-year storm events with a MSE3 rainfall distribution, with rainfall depths as shown in Table 1 below:

Table 1: Design Rainfall Values

Storm Recurrence Interval	24-Hour Rainfall Volume (Inches)
2-Year	2.80
10-year	3.97
100-year	6.22

WinSLAMM (Version 10.5) Source Loading and Management Model (SLAMM) as developed by Robert Pitt and John Voorhees was used to determine the removal rate of

Total Suspended Solids on the site. The WinSLAMM model input data and output can be viewed in **Appendix 4**.

PRE-DEVELOPMENT CONDITIONS

Under Pre-Development Conditions, this area contains approximately 4 acres draining to the site outfall at the north edge of the property. The area consists of woodlands. The site currently drains in a northern direction from the south. There is one drainage basin in Pre-Development Conditions. Existing soils throughout the site consist of primarily Miami Silt Loams, therefore, the Hydrologic Soil Group classification for the site has been selected as HSG B. A map of the existing soils along with their classifications, can be found in **Appendix 1**. The following is a tabular summary including peak flows for Pre-Development Conditions:

Table 2: Hydrologic Analysis of Pre-Development Conditions

Drainage Area	Area (Acres)	Runoff Curve Number	Time of Conc. (Min.)	Peak Flows (cfs)		
				2-Year	10-Year	100-year
Pre-1	3.94	55	26.9	0.20	1.32	5.59
Total	3.94			0.20	1.32	5.59

HydroCAD model input and output data for Pre-Development Conditions is in **Appendix 2**.

POST-DEVELOPMENT CONDITIONS

HydroCAD input and output data for Post-Development conditions along with an exhibit depicting Post-Development Conditions is located in **Appendix 3**. We were instructed to analyze Post-Development Conditions under two scenarios: One scenario was to model just the proposed development parcels; the second was to analyze the proposed development parcels along with the existing land to the south; this land does not currently drain to our project location, however, a culvert is proposed to be constructed underneath E. Geneva Street that would convey runoff from the south, into a proposed drainage swale that would convey the runoff along the east side of the proposed development to the north to a proposed Wet-Detention Basin which has been designed to satisfy all applicable requirements, including the offsite flow. There are approximately 2 acres of impervious area that is tributary to the Wet-Detention Basin, approximately half of this is from the offsite area to the south.

Table 3: Hydrologic Analysis of Post-Development Conditions-Project Parcels

Drainage Basin	Area (Acres)	Runoff Curve Number	Time of Conc. (Min.)	Peak Flows (cfs)		
				2-Year	10-Year	100-year
2S Onsite	3.94	72	11.8	0.13	0.23	0.37
Total	3.94			0.13	0.23	0.37

Table 4: Wet-Detention Basin Data

Wet-Detention Basin Data							
Basin	Peak Outflow 2-Year (cfs)	Peak Outflow 100-Year (cfs)	Bottom Elevation (ft.)	Peak W.S. Elev. 2-Year	Peak W.S. Elev. 100-Year	Spillway Elev.	Top of Berm Elev.
(1P)	0.13	0.37	905.00	905.30	906.45	907.50	908.00

Table 5

Comparison Between Pre-Development and Post-Development Flows					
Pre-Dev. 2-Year (cfs)	Post-Dev. 2-Year (cfs)	Pre-Dev. 10-Year (cfs)	Post-Dev. 10-Year (cfs)	Pre-Dev. 100-Year (cfs)	Post-Dev. 100-Year (cfs)
0.20	0.13	1.32	0.23	5.59	0.37

Table 6: Hydrologic Analysis of Post-Development Conditions-Project Parcels with Offsite

Drainage Basin	Area (Acres)	Runoff Curve Number	Time of Conc. (Min.)	Peak Flows (cfs)		
				2-Year	10-Year	100-year
2S Onsite	3.94	72	11.8	3.58	7.97	17.91
1S Offsite	3.00	72	12.9	2.59	5.84	13.17
Total	6.94			6.17	13.78	25.13

Table 7: Wet-Detention Basin Data

Wet-Detention Basin Data Including Offsite							
Basin	Peak Outflow 2-Year (cfs)	Peak Outflow 100-Year (cfs)	Bottom Elevation (ft.)	Peak W.S. Elev. 2-Year	Peak W.S. Elev. 100-Year	Spillway Elev.	Top of Berm Elev.
(1P)	0.20	2.80	905.00	905.52	907.08	907.50	908.00

Table 8

Comparison Between Pre-Development and Post-Development Flows					
Pre-Dev. 2-Year (cfs)	Post-Dev. 2-Year (cfs)	Pre-Dev. 10-Year (cfs)	Post-Dev. 10-Year (cfs)	Pre-Dev. 100-Year (cfs)	Post-Dev. 100-Year (cfs)
0.20	0.20	1.32	0.32	5.59	2.80

INFILTRATION

ECS Midwest, LLC, performed soil test pits for the project, and provided their report dated October 28, 2024. A summary of their work, and their report can be found in **Appendix 5**. The native soil infiltration rates ranged from 0.03 to 0.11 inches per hour. These rates were based on the textural classification of the soils encountered and the guidelines provided in the WDNR Conservation Practice Standard 1002. Based on these rates, we are claiming an exemption of the infiltration requirements.

STORMWATER QUALITY CONTROL

The proposed project will utilize Wet-Detention (Basin 1P) to achieve post-construction storm water quality control in accordance with Village of Williams Bay and WDNR requirements for suspended solids removal. The requirement for this site is 80% removal of the Total Suspended Solids (TSS) based on no controls. WinSLAMM was used to calculate the water quality impacts resulting from this project.

Table 9: Water Quality Summary-Project Parcels

Source	Area (ac)	Pounds of TSS Generated	Pounds of TSS Remaining	Percent Removal
2S Onsite	3.94	271	10	96.15%

Table 10: Water Quality Summary-Project Parcels with Offsite

Source	Area (ac)	Pounds of TSS Generated	Pounds of TSS Remaining	Percent Removal
2S Onsite, 1S Offsite	6.94	492	25	94.88%

MAINTENANCE PROGRAM

Inspection and maintenance frequencies for the proposed wet-detention basin vary depending on the task. The following is a recommended frequency for inspection items and associated maintenance:

Once, after the first year of construction:

- Check for invasive plants, if present, remove the invasive plants.

Monthly to Quarterly, or after major storms:

- Inspect orifices and pipes for clogging. Check the shoreline, inflows, and discharges, for signs of erosion, repair eroded areas, and replant bare soils. Check for and remove any floating debris.

Semiannual to Annual:

- Monitor plant composition and health. Ensure mechanical components are functional. Clean up trash and debris, remove any invasive plants, repair broken mechanical components.

Every 1 to 3 years:

- Inspect all mechanical components including pipes and risers. Monitor sediment disposition. Repair broken mechanical components and remove sediment from the basin if needed.

EROSION CONTROL

Erosion Control plans will be prepared in accordance with Village of Williams Bay and WDNR requirements. Construction erosion control practices such as a tracking pad, silt fence, ditch checks and inlet protection will be incorporated into the plans. Permanent site stabilization includes the use of erosion mats on steep slopes and rip rap at all pipe outlets.

Erosion Control measures will be installed prior to construction. Measures will remain in place until site restoration is completed, and ninety percent growth is achieved.

SOIL LOSS

Soil erosion is calculated using the USLE (Universal Soil Loss Equation). The project is required to reduce the soil losses to a maximum of 5.0 Tons/Acre and provide erosion measures to reduce any soil loss.

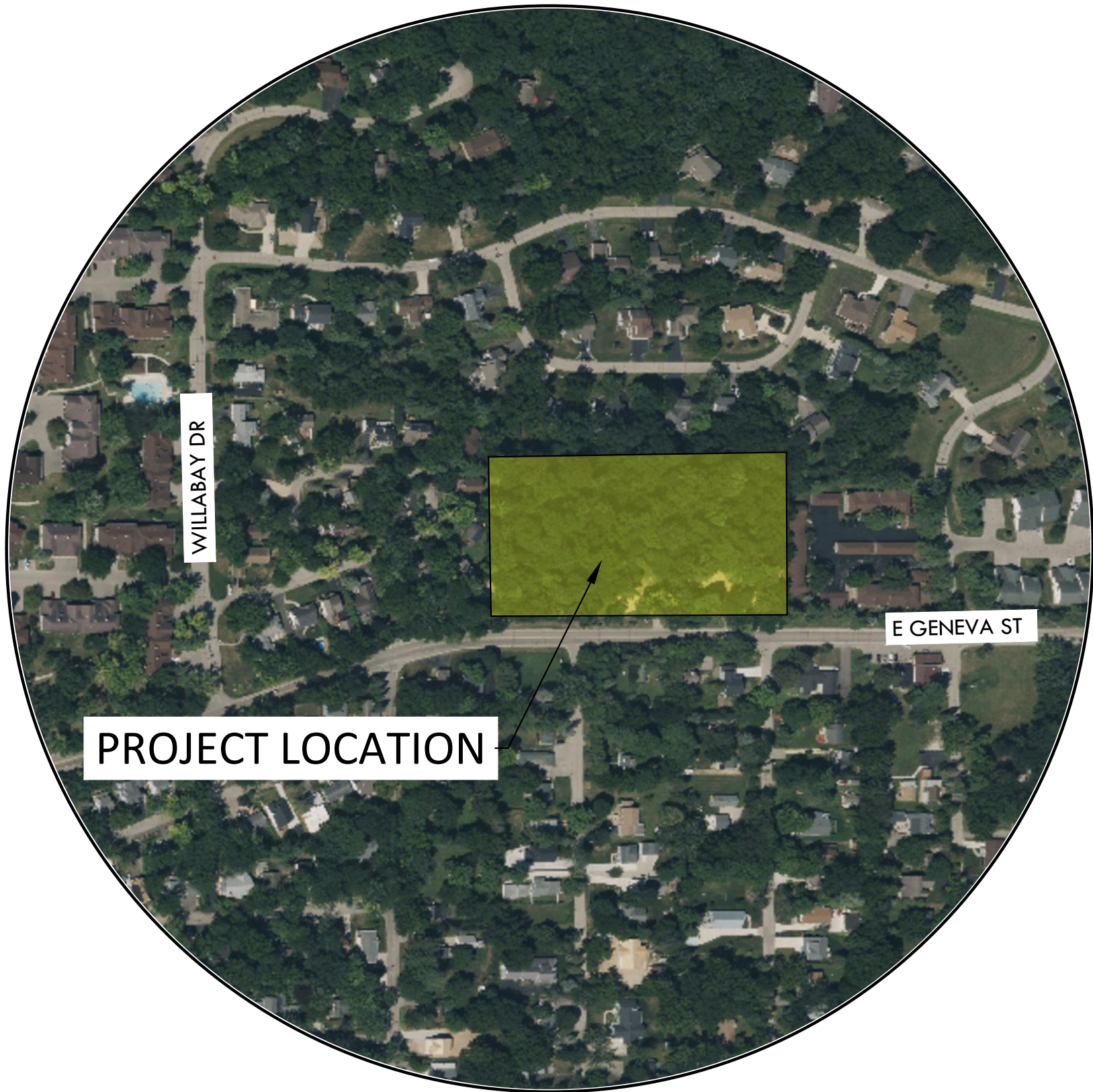
WETLANDS/WATERWAYS

No indicator soils or other typical wetlands indicators were identified on the site upon review of the WDNR Wetland Indicator Map, therefore, a wetland delineation was not completed.

CONCLUSION

The site has been evaluated for the effects of the proposed development. In conclusion, the design requirements for both stormwater quality and stormwater quantity control for this site have been met.

Appendix 1 MAPS



VICINITY MAP

SITE IMPROVEMENT PLANS
FOR
GENEVA STREET DEVELOPMENT
WILLIAMS BAY, WI

SHEET INDEX

C001	TITLE SHEET & VICINITY MAP
C002	GENERAL NOTES
C102	GRADING & EROSION CONTROL PLAN
C201	SANITARY SEWER PLAN AND PROFILE
C202	WATER MAIN PLAN AND PROFILE
C501	CONSTRUCTION DETAILS
C502	CONSTRUCTION DETAILS

BENCHMARKS

FOUND IRON PIPE	EL. 812.93	N: 214504.42	E: 2366013.53
FOUND IRON PIPE	EL. 799.21	N: 214790.56	E: 2366008.72
FOUND IRON PIPE	EL. 796.44	N: 214782.51	E: 2365561.11
FOUND IRON PIPE	EL. 817.00	N: 214495.91	E: 2365564.93

CONTACT INFORMATION

COMPANY NAME	ADDRESS	CONTACT	PHONE	EMAIL
LYNCH & ASSOCIATES	440 MILWAUKEE AVE. BURLINGTON, WI 53105	TIM LYNCH, P.E.	262.402.5034	tlynch@lynch-engineering.com
LYNCH & ASSOCIATES	440 MILWAUKEE AVE. BURLINGTON, WI 53105	DAVE VIVIAN, P.E.	262.402.5042	dvivian@lynch-engineering.com
LYNCH & ASSOCIATES	5482 S. WESTRIDGE DR. NEW BERLIN, WI 53151	RYAN KLOTH, P.E. CFM	262.402.5038	rkloth@lynch-engineering.com

REVISIONS

NO.	REVISION	DATE
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SITE IMPROVEMENT PLANS FOR
GENEVA STREET DEVELOPMENT
WILLIAMS BAY, WI

LYNCH & ASSOCIATES
ENGINEERING CONSULTANTS, LLC

ISSUED FOR
REVIEW

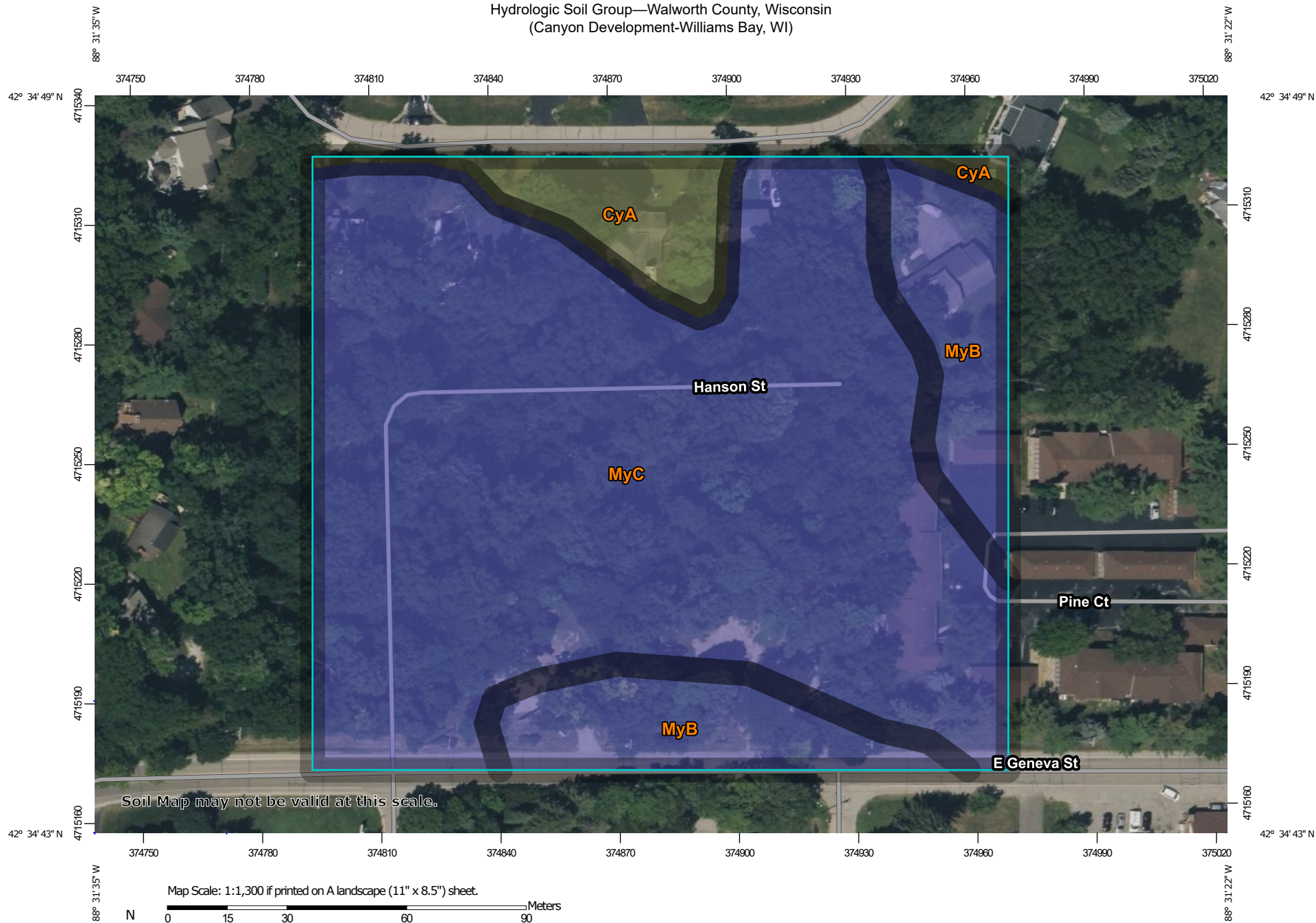
PLAN DATE 05.05.25

PROJECT NO.
23.0023

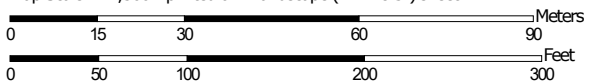
SHEET NO.

C001

Hydrologic Soil Group—Walworth County, Wisconsin (Canyon Development-Williams Bay, WI)



Map Scale: 1:1,300 if printed on A landscape (11" x 8.5") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 16N WGS84



**Natural Resources
Conservation Service**

Web Soil Survey
National Cooperative Soil Survey

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

Soil Rating Polygons

 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Lines

 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Points

 A
 A/D
 B
 B/D

 C
 C/D
 D
 Not rated or not available

Water Features

 Streams and Canals

Transportation

 Rails
 Interstate Highways
 US Routes
 Major Roads
 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15,800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Walworth County, Wisconsin
Survey Area Data: Version 21, Sep 3, 2024

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 30, 2022—Aug 18, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
CyA	Conover silt loam, 1 to 3 percent slopes	C/D	0.5	6.9%
MyB	Miami silt loam, 2 to 6 percent slopes	B	1.1	16.8%
MyC	Miami silt loam, 6 to 12 percent slopes	B	5.1	76.4%
Totals for Area of Interest			6.7	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

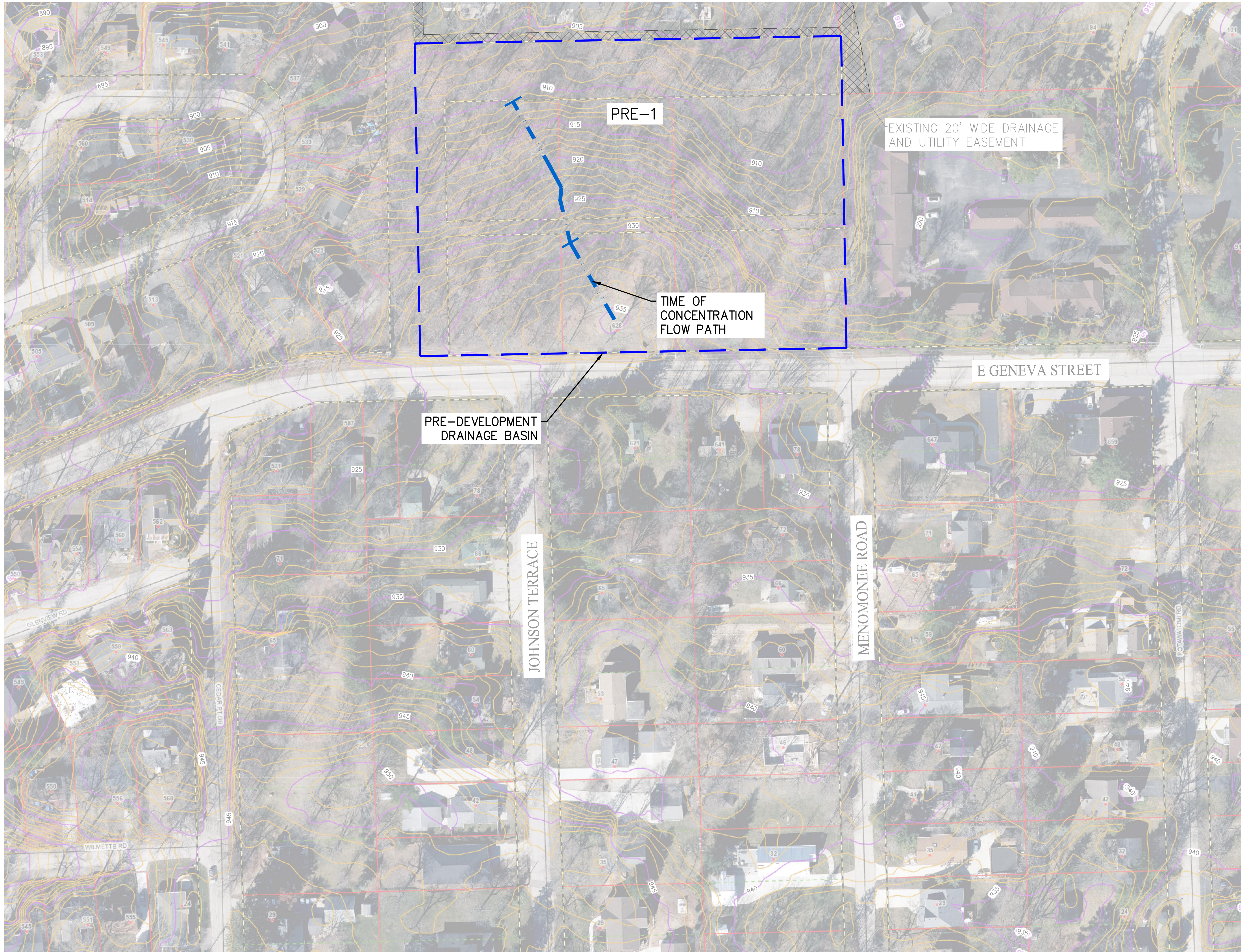
Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

Appendix 2 PRE-DEVELOPMENT CONDITIONS



STORM WATER MANAGEMENT PLAN
CANYON DEVELOPMENT

WILLIAMS BAY, WI

LYNCH & ASSOCIATES
ENGINEERING CONSULTANTS, LLC

PRE-DEVELOPMENT CONDITIONS

NO. REVISIONS BY DATE

ISSUED FOR REVIEW

PLAN DATE 05.05.2025

DESIGNED BY RWK

0 60'
SCALE

PROJECT NO.
23.0023

SHEET NO.

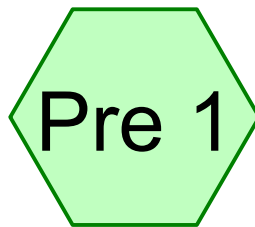
APPENDIX 2



Pre-Dev.

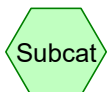


Pre-Dev



Pre 1

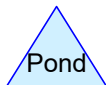
Pre 1



Subcat



Reach



Pond



Link

Routing Diagram for 23.0023 Pre-Development 5-5-25

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23.0023 Pre-Development 5-5-25

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Page 2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
3.940	55	Woods, Good, HSG B (Pre 1)
3.940	55	TOTAL AREA

23.0023 Pre-Development 5-5-25

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Page 3

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
3.940	HSG B	Pre 1
0.000	HSG C	
0.000	HSG D	
0.000	Other	
3.940		TOTAL AREA

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Page 4

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	3.940	0.000	0.000	0.000	3.940	Woods, Good	Pre 1
0.000	3.940	0.000	0.000	0.000	3.940	TOTAL AREA	

23.0023 Pre-Development 5-5-25

MSE 24-hr 3 2 yr Rainfall=2.80"

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Page 5

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Pre 1: Pre 1

Runoff Area=3.940 ac 0.00% Impervious Runoff Depth>0.13"
Flow Length=276' Tc=26.9 min CN=55 Runoff=0.20 cfs 0.041 af

Link Pre-Dev.: Pre-Dev

Inflow=0.20 cfs 0.041 af
Primary=0.20 cfs 0.041 af

Total Runoff Area = 3.940 ac Runoff Volume = 0.041 af Average Runoff Depth = 0.13"
100.00% Pervious = 3.940 ac 0.00% Impervious = 0.000 ac

23.0023 Pre-Development 5-5-25

MSE 24-hr 3 2 yr Rainfall=2.80"

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Summary for Subcatchment Pre 1: Pre 1

Runoff = 0.20 cfs @ 12.70 hrs, Volume= 0.041 af, Depth> 0.13"

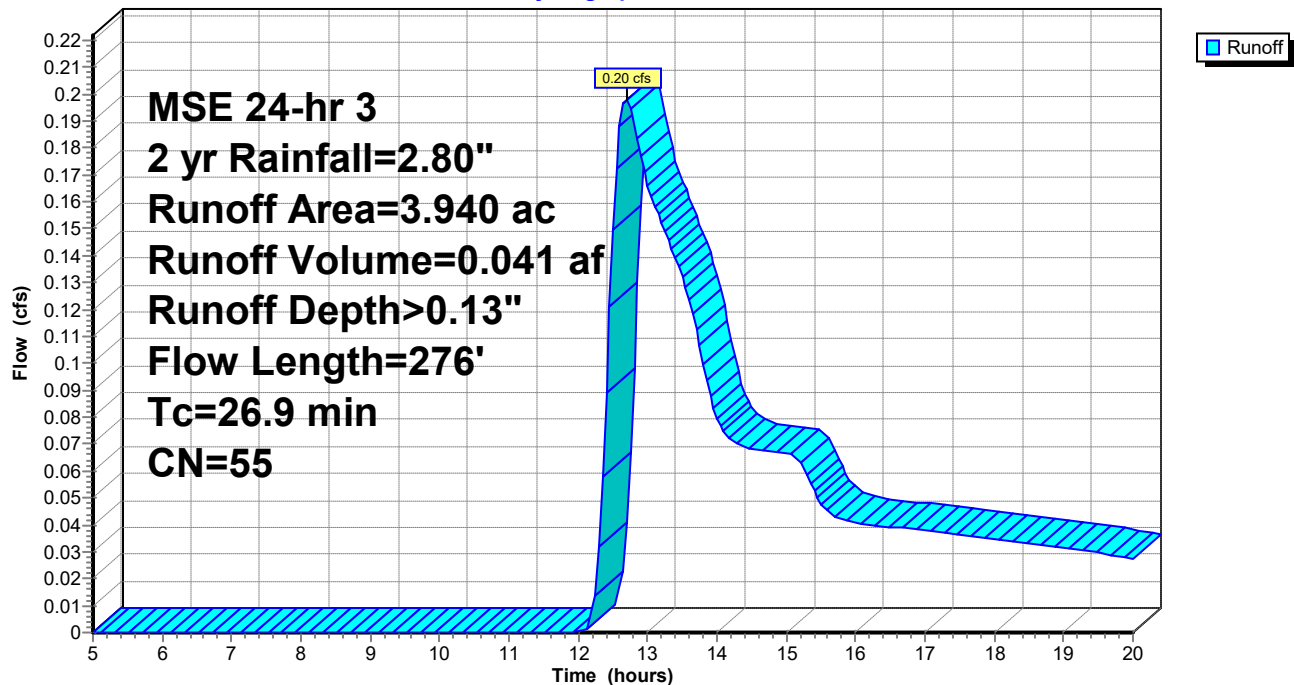
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
MSE 24-hr 3 2 yr Rainfall=2.80"

Area (ac)	CN	Description
3.940	55	Woods, Good, HSG B
3.940		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.1	100	0.0640	0.07		Sheet Flow, Sheet Flow Woods
					Woods: Dense underbrush n= 0.800 P2= 2.80"
1.8	176	0.1028	1.60		Shallow Concentrated Flow, Shallow Conc. Flow-Woods
					Woodland Kv= 5.0 fps
26.9	276	Total			

Subcatchment Pre 1: Pre 1

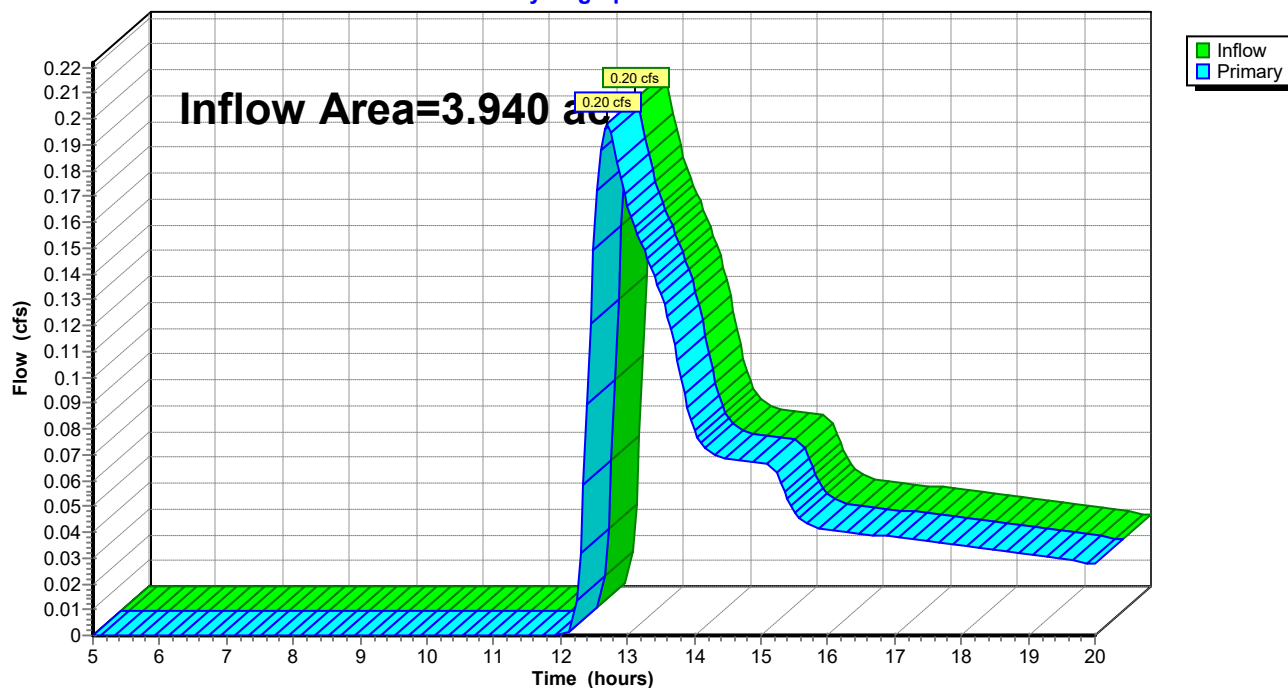
Hydrograph



Summary for Link Pre-Dev.: Pre-Dev

Inflow Area = 3.940 ac, 0.00% Impervious, Inflow Depth > 0.13" for 2 yr event
Inflow = 0.20 cfs @ 12.70 hrs, Volume= 0.041 af
Primary = 0.20 cfs @ 12.70 hrs, Volume= 0.041 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Pre-Dev.: Pre-Dev**Hydrograph**

23.0023 Pre-Development 5-5-25*MSE 24-hr 3 10 yr Rainfall=3.97"*

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Pre 1: Pre 1Runoff Area=3.940 ac 0.00% Impervious Runoff Depth>0.47"
Flow Length=276' Tc=26.9 min CN=55 Runoff=1.32 cfs 0.155 af**Link Pre-Dev.: Pre-Dev**Inflow=1.32 cfs 0.155 af
Primary=1.32 cfs 0.155 af**Total Runoff Area = 3.940 ac Runoff Volume = 0.155 af Average Runoff Depth = 0.47"**
100.00% Pervious = 3.940 ac 0.00% Impervious = 0.000 ac

23.0023 Pre-Development 5-5-25

MSE 24-hr 3 10 yr Rainfall=3.97"

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Page 9

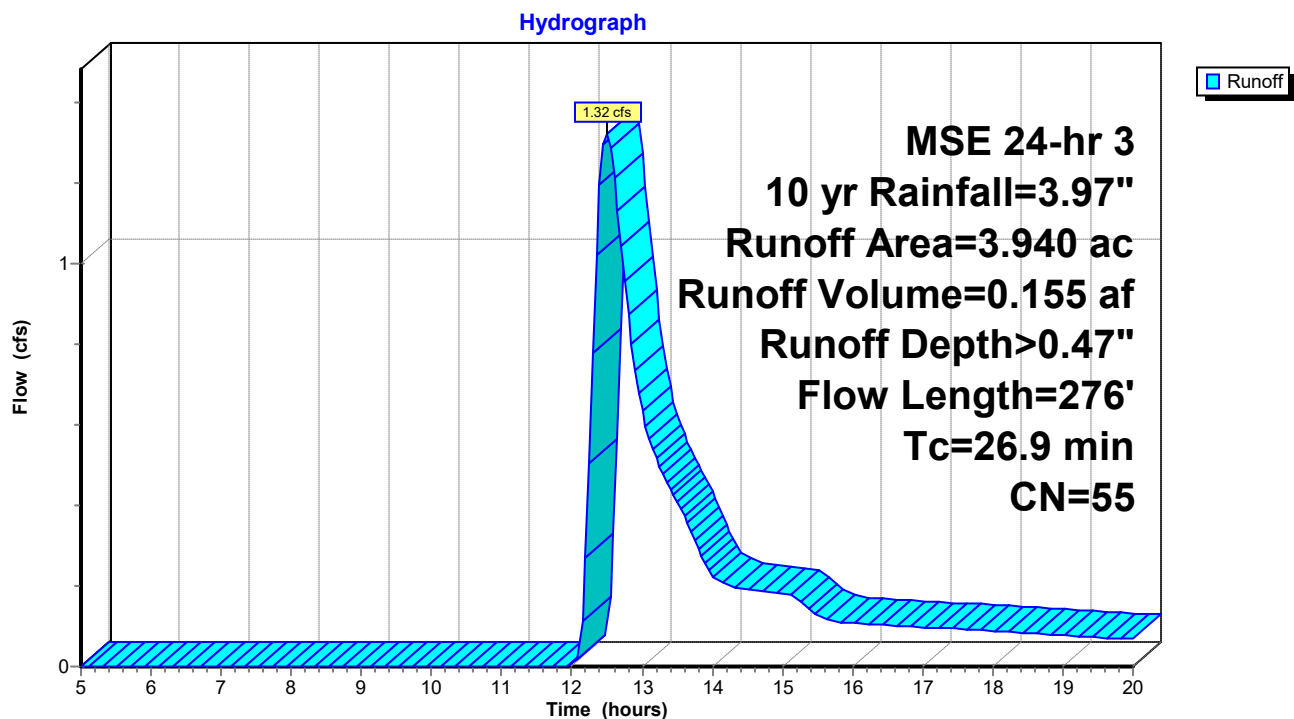
Summary for Subcatchment Pre 1: Pre 1

Runoff = 1.32 cfs @ 12.50 hrs, Volume= 0.155 af, Depth> 0.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
MSE 24-hr 3 10 yr Rainfall=3.97"

Area (ac)	CN	Description
3.940	55	Woods, Good, HSG B
3.940		100.00% Pervious Area

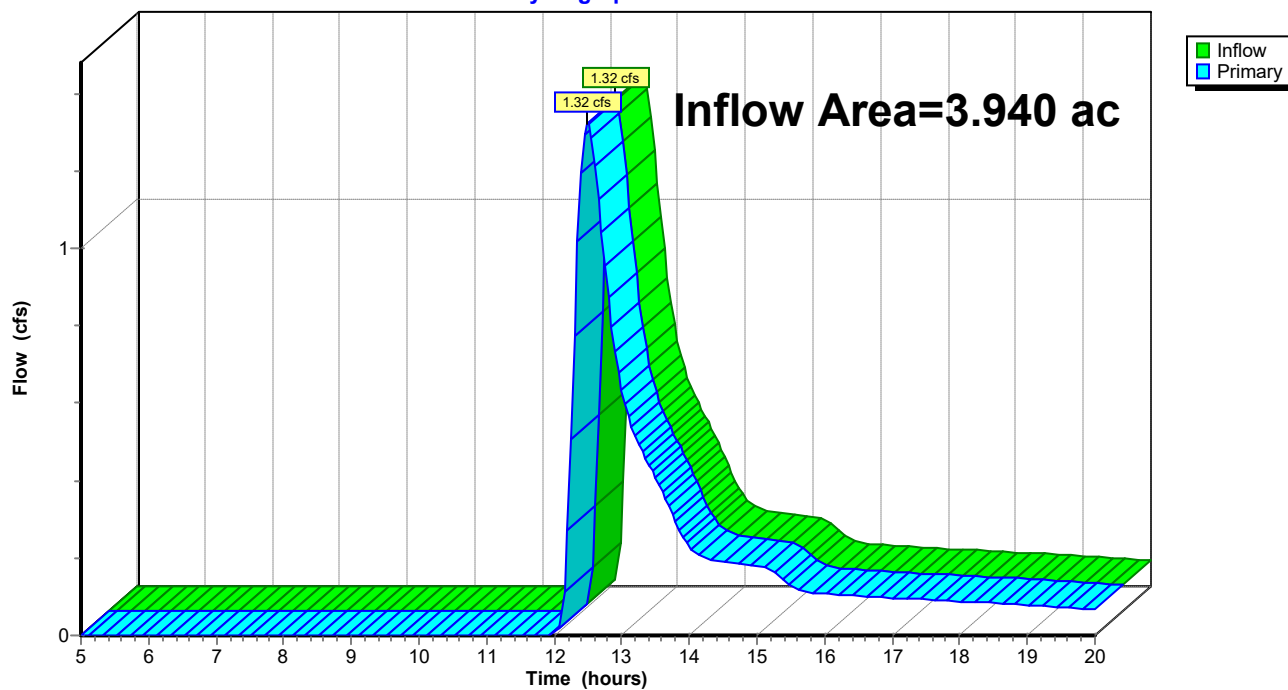
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.1	100	0.0640	0.07		Sheet Flow, Sheet Flow Woods
					Woods: Dense underbrush n= 0.800 P2= 2.80"
1.8	176	0.1028	1.60		Shallow Concentrated Flow, Shallow Conc. Flow-Woods
					Woodland Kv= 5.0 fps
26.9	276	Total			

Subcatchment Pre 1: Pre 1

Summary for Link Pre-Dev.: Pre-Dev

Inflow Area = 3.940 ac, 0.00% Impervious, Inflow Depth > 0.47" for 10 yr event
Inflow = 1.32 cfs @ 12.50 hrs, Volume= 0.155 af
Primary = 1.32 cfs @ 12.50 hrs, Volume= 0.155 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Pre-Dev.: Pre-Dev**Hydrograph**

23.0023 Pre-Development 5-5-25*MSE 24-hr 3 100 yr Rainfall=6.22"*

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Page 11

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment Pre 1: Pre 1Runoff Area=3.940 ac 0.00% Impervious Runoff Depth>1.54"
Flow Length=276' Tc=26.9 min CN=55 Runoff=5.59 cfs 0.504 af**Link Pre-Dev.: Pre-Dev**Inflow=5.59 cfs 0.504 af
Primary=5.59 cfs 0.504 af**Total Runoff Area = 3.940 ac Runoff Volume = 0.504 af Average Runoff Depth = 1.54"**
100.00% Pervious = 3.940 ac 0.00% Impervious = 0.000 ac

23.0023 Pre-Development 5-5-25

MSE 24-hr 3 100 yr Rainfall=6.22"

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Page 12

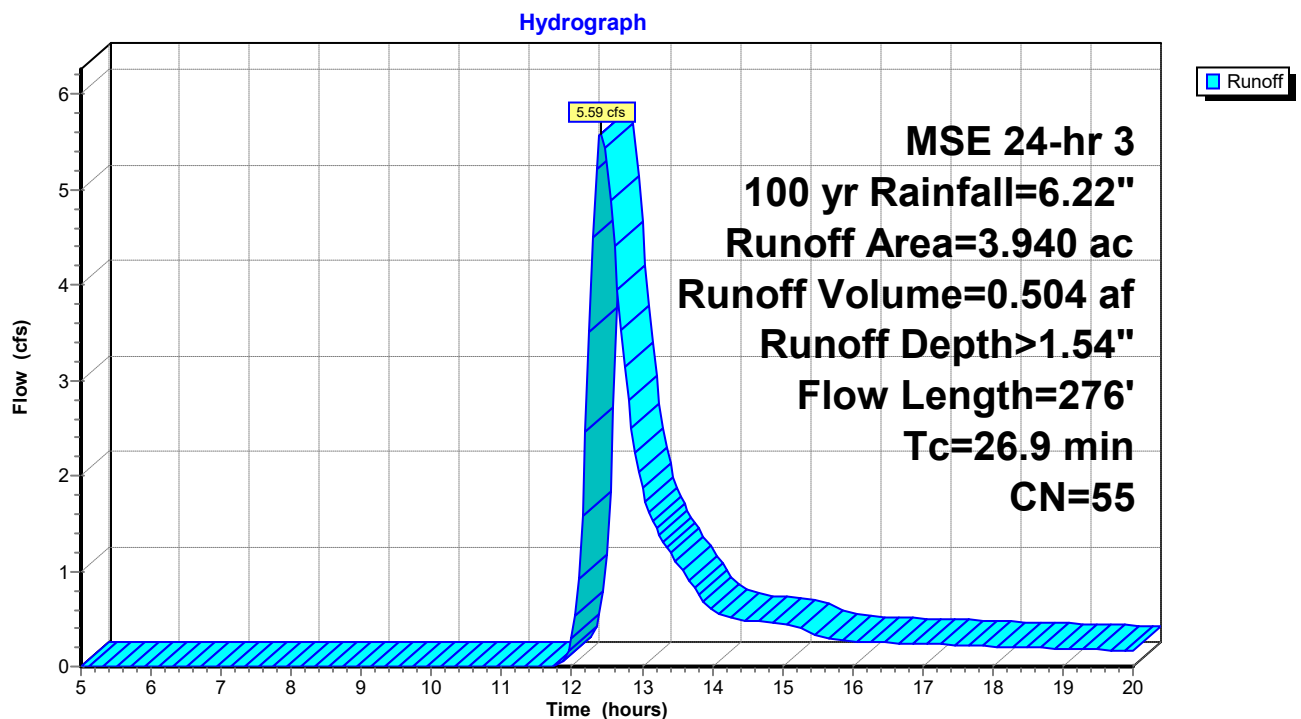
Summary for Subcatchment Pre 1: Pre 1

Runoff = 5.59 cfs @ 12.43 hrs, Volume= 0.504 af, Depth> 1.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
MSE 24-hr 3 100 yr Rainfall=6.22"

Area (ac)	CN	Description
3.940	55	Woods, Good, HSG B
3.940		100.00% Pervious Area

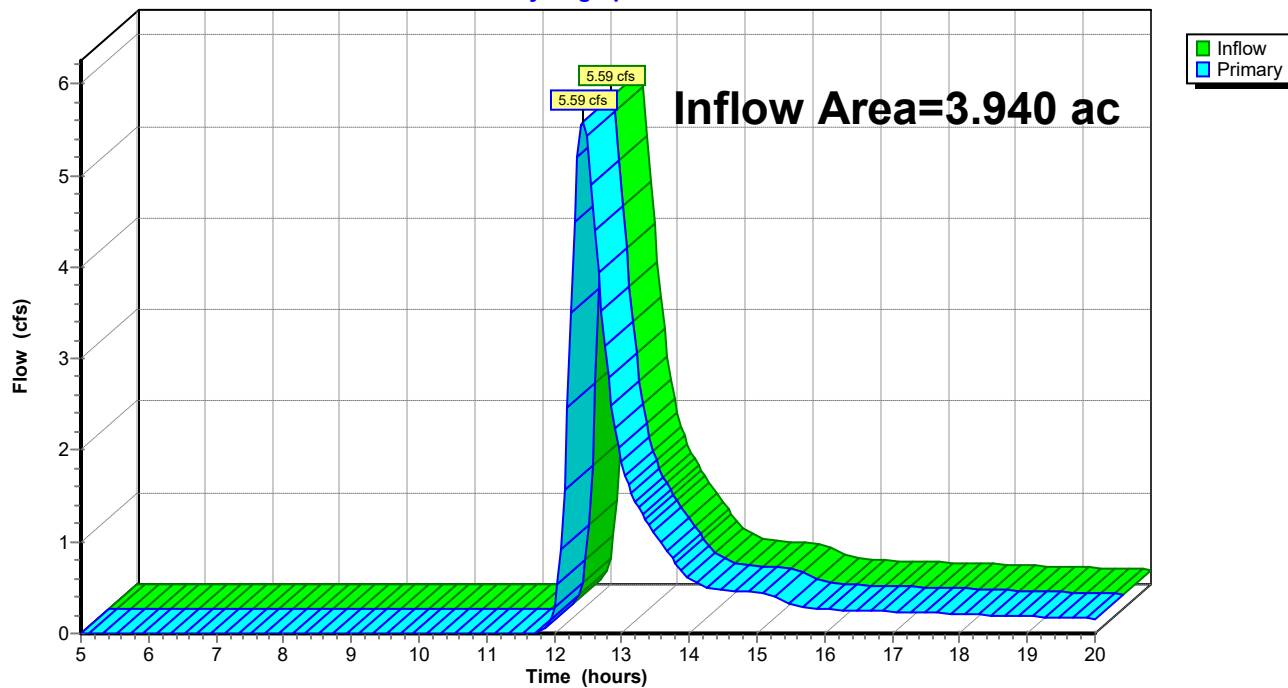
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.1	100	0.0640	0.07		Sheet Flow, Sheet Flow Woods
					Woods: Dense underbrush n= 0.800 P2= 2.80"
1.8	176	0.1028	1.60		Shallow Concentrated Flow, Shallow Conc. Flow-Woods
					Woodland Kv= 5.0 fps
26.9	276	Total			

Subcatchment Pre 1: Pre 1

Summary for Link Pre-Dev.: Pre-Dev

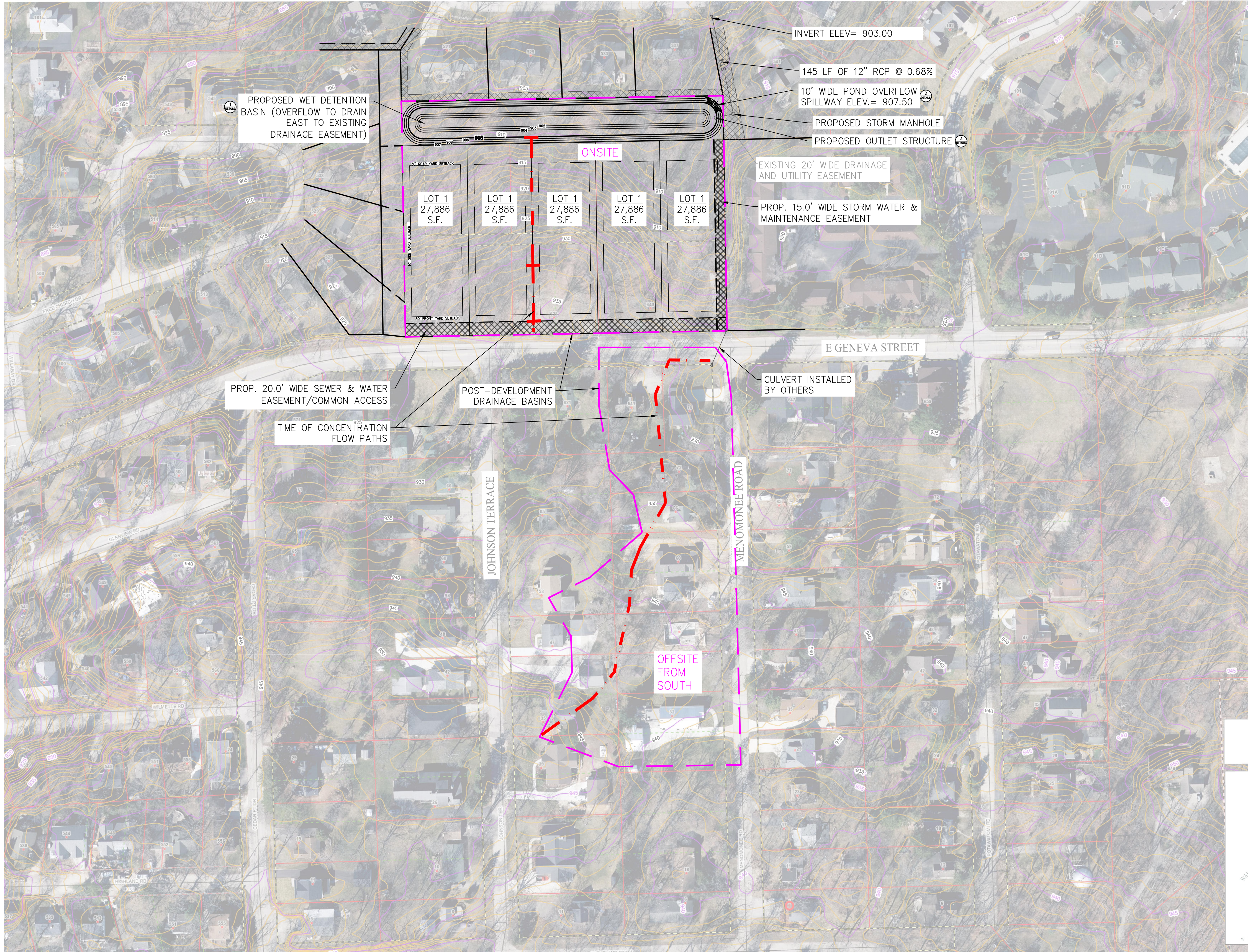
Inflow Area = 3.940 ac, 0.00% Impervious, Inflow Depth > 1.54" for 100 yr event
Inflow = 5.59 cfs @ 12.43 hrs, Volume= 0.504 af
Primary = 5.59 cfs @ 12.43 hrs, Volume= 0.504 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Pre-Dev.: Pre-Dev**Hydrograph**

Appendix 3

POST DEVELOPMENT CONDITIONS



NOTE: SF-3 ZONING.

STORM WATER MANAGEMENT PLAN
CANYON DEVELOPMENT

WILLIAMS BAY, WI

LYNCH & ASSOCIATES
ENGINEERING CONSULTANTS, LLC

POST-DEVELOPMENT CONDITIONS

NO. BY DATE

REVISIONS

ISSUED FOR REVIEW

PLAN DATE 05.05.2025

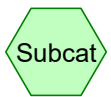
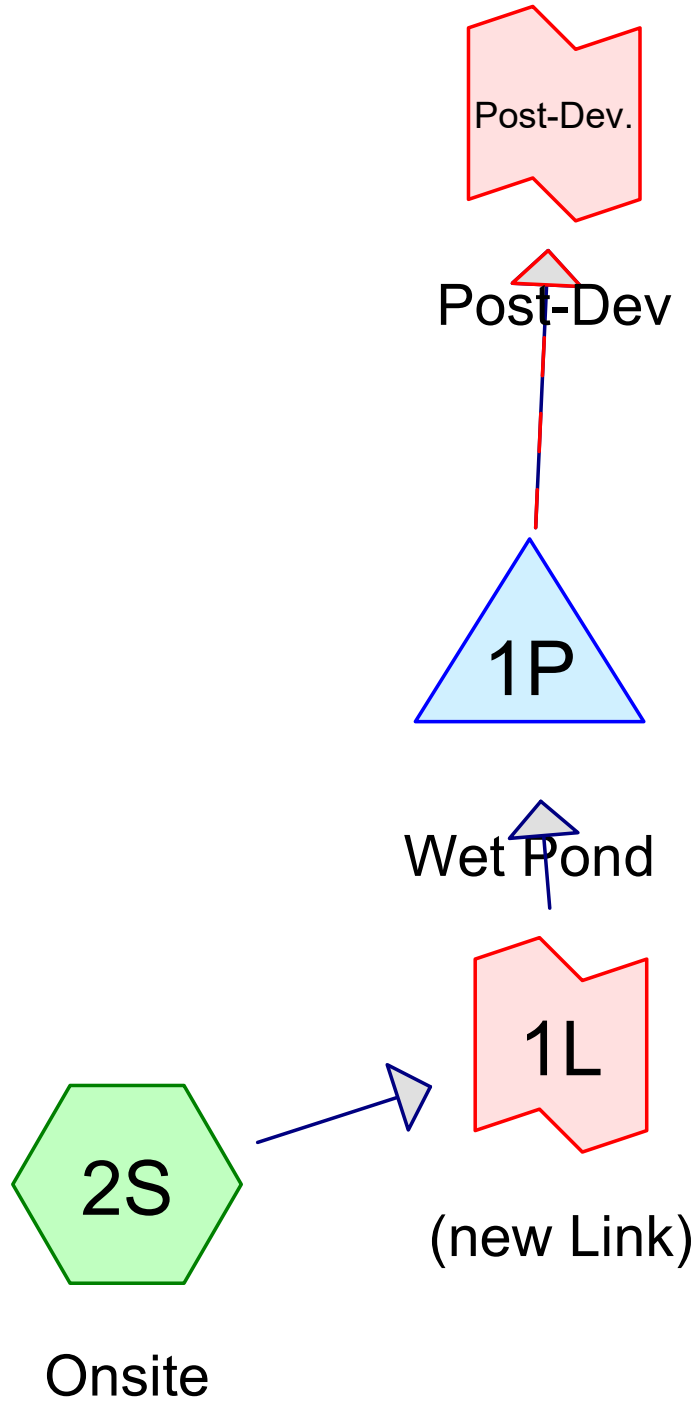
DESIGNED BY RWK

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SCALE

PROJECT NO.
23.0023

SHEET NO.

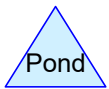
APPENDIX 3



Subcat



Reach



Pond



Link

Routing Diagram for 23.0023 Post-Development Proposed Parcels 5-5-25
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23.0023 Post-Development Proposed Parcels 5-5-25

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Page 2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
2.770	61	>75% Grass cover, Good, HSG B (2S)
0.330	98	Driveways, HSG B (2S)
0.350	98	Roofs, HSG B (2S)
0.490	98	Water Surface, HSG B (2S)
3.940	72	TOTAL AREA

23.0023 Post-Development Proposed Parcels 5-5-25

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Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
3.940	HSG B	2S
0.000	HSG C	
0.000	HSG D	
0.000	Other	
3.940		TOTAL AREA

23.0023 Post-Development Proposed Parcels 5-5-25

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Page 4

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	2.770	0.000	0.000	0.000	2.770	>75% Grass cover, Good	2S
0.000	0.330	0.000	0.000	0.000	0.330	Driveways	2S
0.000	0.350	0.000	0.000	0.000	0.350	Roofs	2S
0.000	0.490	0.000	0.000	0.000	0.490	Water Surface	2S
0.000	3.940	0.000	0.000	0.000	3.940	TOTAL AREA	

23.0023 Post-Development Proposed Parcels 5-5-25

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Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	1P	905.00	904.00	22.0	0.0455	0.011	12.0	0.0	0.0

23.0023 Post-Development Proposed Parcels 5-5-25*MSE 24-hr 3 2 yr Rainfall=2.80"*

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Page 6

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 2S: Onsite

Runoff Area=3.940 ac 29.70% Impervious Runoff Depth>0.65"
Flow Length=293' Tc=11.8 min CN=72 Runoff=3.58 cfs 0.213 af

Pond 1P: Wet Pond

Peak Elev=905.30' Storage=0.149 af Inflow=3.58 cfs 0.213 af
Primary=0.13 cfs 0.075 af Secondary=0.00 cfs 0.000 af Outflow=0.13 cfs 0.075 af

Link 1L: (new Link)

Inflow=3.58 cfs 0.213 af
Primary=3.58 cfs 0.213 af

Link Post-Dev.: Post-Dev

Inflow=0.13 cfs 0.075 af
Primary=0.13 cfs 0.075 af

Total Runoff Area = 3.940 ac Runoff Volume = 0.213 af Average Runoff Depth = 0.65"
70.30% Pervious = 2.770 ac 29.70% Impervious = 1.170 ac

23.0023 Post-Development Proposed Parcels 5-5-25

MSE 24-hr 3 2 yr Rainfall=2.80"

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Page 7

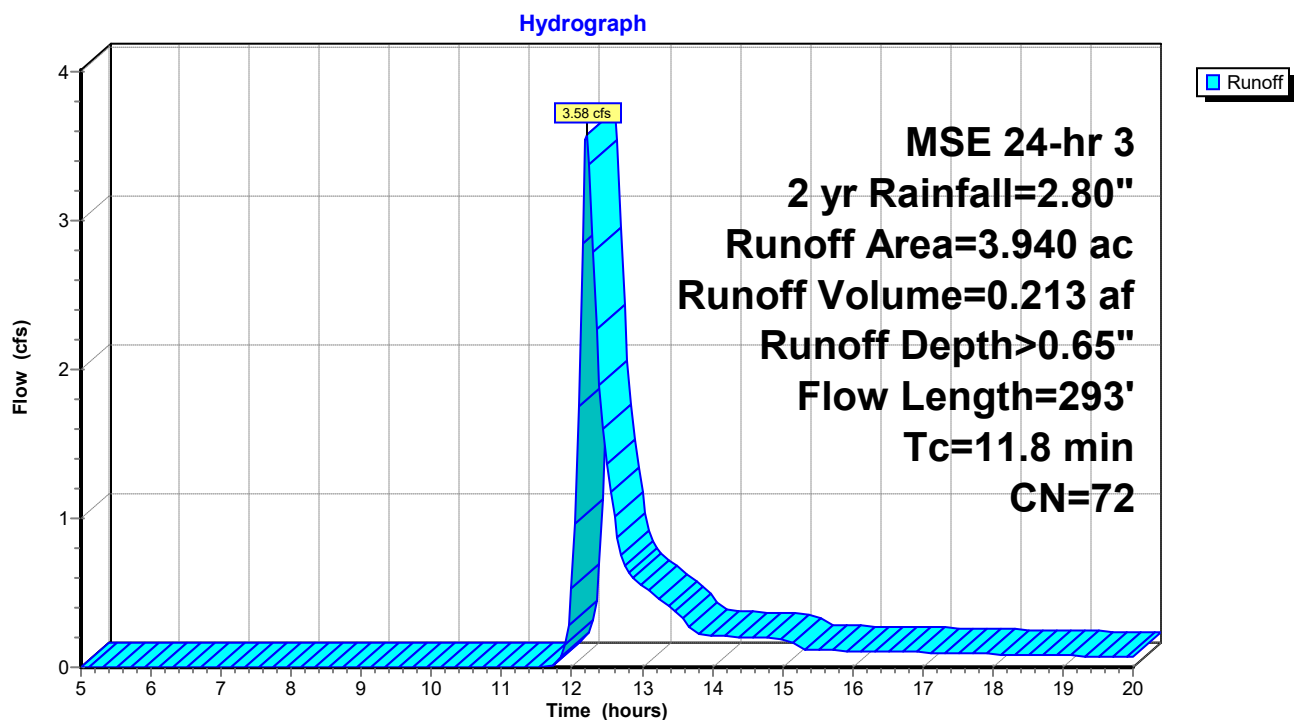
Summary for Subcatchment 2S: Onsite

Runoff = 3.58 cfs @ 12.22 hrs, Volume= 0.213 af, Depth> 0.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
MSE 24-hr 3 2 yr Rainfall=2.80"

Area (ac)	CN	Description
0.350	98	Roofs, HSG B
* 0.330	98	Driveways, HSG B
0.490	98	Water Surface, HSG B
2.770	61	>75% Grass cover, Good, HSG B
3.940	72	Weighted Average
2.770		70.30% Pervious Area
1.170		29.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.3	16	0.0300	1.05		Sheet Flow, Sheet Flow Pavement Smooth surfaces n= 0.011 P2= 2.80"
11.3	84	0.0300	0.12		Sheet Flow, Sheet Flow-Grass Grass: Dense n= 0.240 P2= 2.80"
0.2	193	0.0850	14.44	288.83	Channel Flow, Channel-Grass Swale Area= 20.0 sf Perim= 20.0' r= 1.00' n= 0.030 Short grass
11.8	293	Total			

Subcatchment 2S: Onsite

23.0023 Post-Development Proposed Parcels 5-5-25

MSE 24-hr 3 2 yr Rainfall=2.80"

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Page 8

Summary for Pond 1P: Wet Pond

Inflow Area = 3.940 ac, 29.70% Impervious, Inflow Depth > 0.65" for 2 yr event
 Inflow = 3.58 cfs @ 12.22 hrs, Volume= 0.213 af
 Outflow = 0.13 cfs @ 15.28 hrs, Volume= 0.075 af, Atten= 96%, Lag= 184.1 min
 Primary = 0.13 cfs @ 15.28 hrs, Volume= 0.075 af
 Secondary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 905.30' @ 15.28 hrs Surf.Area= 0.511 ac Storage= 0.149 af

Plug-Flow detention time= 244.3 min calculated for 0.075 af (35% of inflow)
 Center-of-Mass det. time= 161.5 min (977.0 - 815.5)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	1.785 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.490	0.000	0.000
906.00	0.560	0.525	0.525
907.00	0.630	0.595	1.120
908.00	0.700	0.665	1.785

Device	Routing	Invert	Outlet Devices
#1	Primary	905.00'	12.0" Round RCP_Round 12" L= 22.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 905.00' / 904.00' S= 0.0455 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf
#2	Device 1	905.00'	3.5" Vert. Orifice/Grate C= 0.600
#3	Device 1	906.90'	36.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Secondary	907.50'	10.0' long x 7.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.40 2.52 2.70 2.68 2.68 2.67 2.66 2.65 2.65 2.65 2.66 2.65 2.66 2.68 2.70 2.73 2.78

Primary OutFlow Max=0.13 cfs @ 15.28 hrs HW=905.30' (Free Discharge)

↑ **1=RCP_Round 12"** (Passes 0.13 cfs of 0.32 cfs potential flow)

↑ **2=Orifice/Grate** (Orifice Controls 0.13 cfs @ 1.88 fps)

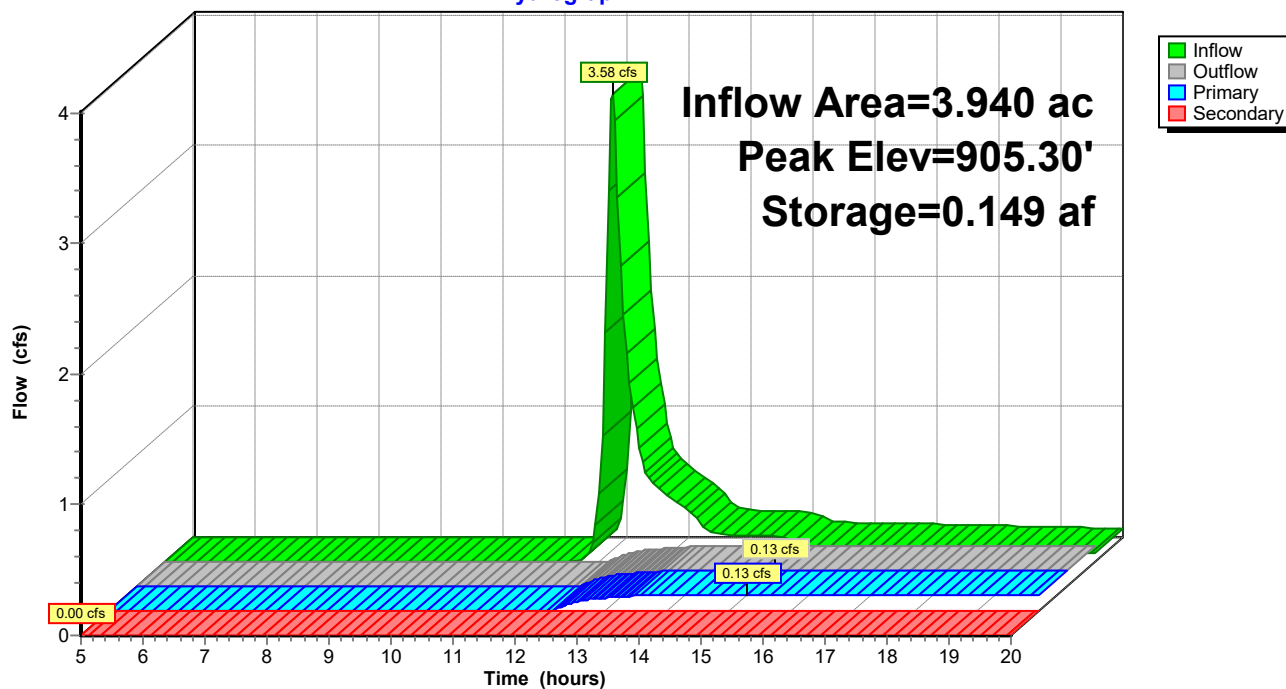
↑ **3=Orifice/Grate** (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=905.00' (Free Discharge)

↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond 1P: Wet Pond

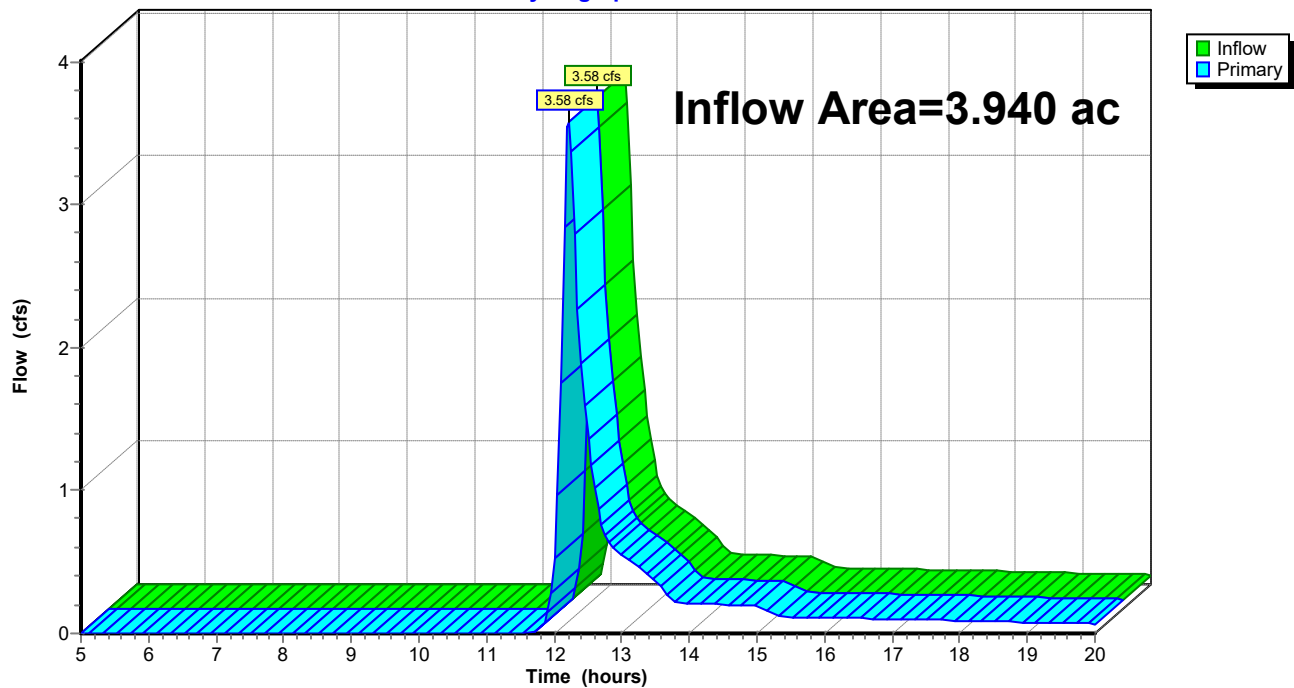
Hydrograph



Summary for Link 1L: (new Link)

Inflow Area = 3.940 ac, 29.70% Impervious, Inflow Depth > 0.65" for 2 yr event
Inflow = 3.58 cfs @ 12.22 hrs, Volume= 0.213 af
Primary = 3.58 cfs @ 12.22 hrs, Volume= 0.213 af, Atten= 0%, Lag= 0.0 min

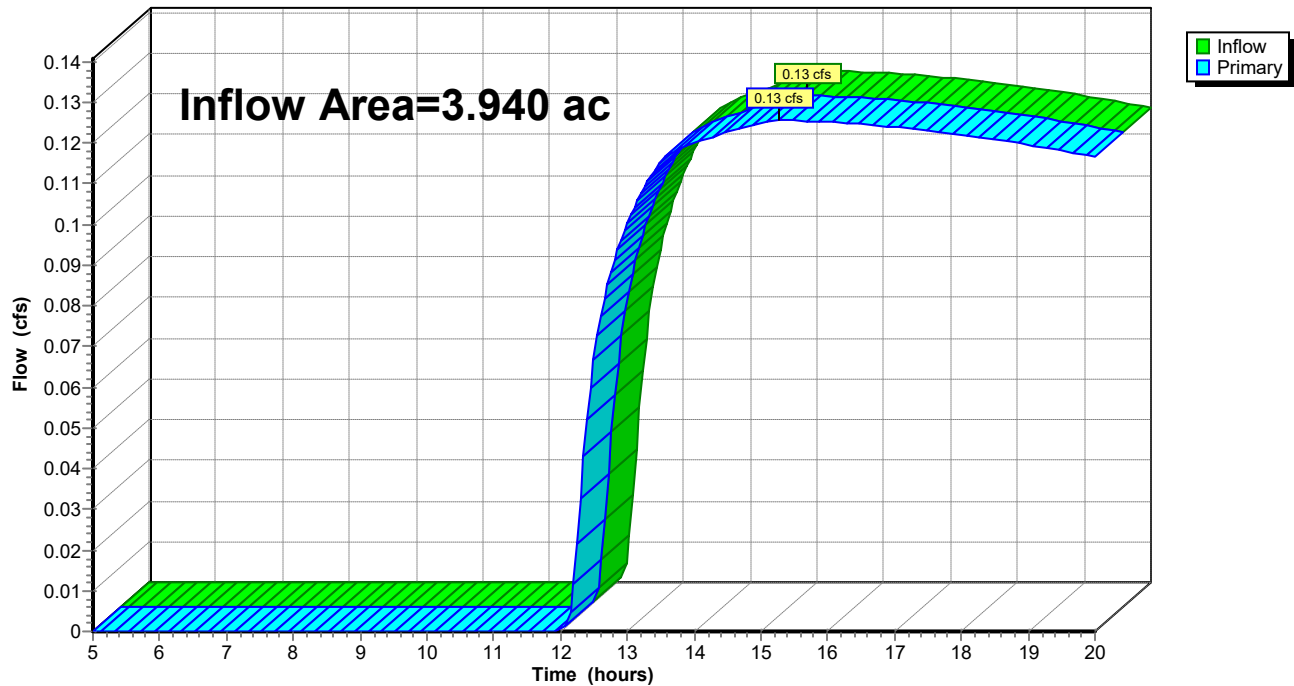
Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link 1L: (new Link)**Hydrograph**

Summary for Link Post-Dev.: Post-Dev

Inflow Area = 3.940 ac, 29.70% Impervious, Inflow Depth > 0.23" for 2 yr event
Inflow = 0.13 cfs @ 15.28 hrs, Volume= 0.075 af
Primary = 0.13 cfs @ 15.28 hrs, Volume= 0.075 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Post-Dev.: Post-Dev**Hydrograph**

23.0023 Post-Development Proposed Parcels 5-5-25*MSE 24-hr 3 10 yr Rainfall=3.97"*

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Page 12

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 2S: Onsite

Runoff Area=3.940 ac 29.70% Impervious Runoff Depth>1.36"
Flow Length=293' Tc=11.8 min CN=72 Runoff=7.97 cfs 0.447 af

Pond 1P: Wet Pond

Peak Elev=905.64' Storage=0.329 af Inflow=7.97 cfs 0.447 af
Primary=0.23 cfs 0.141 af Secondary=0.00 cfs 0.000 af Outflow=0.23 cfs 0.141 af

Link 1L: (new Link)

Inflow=7.97 cfs 0.447 af
Primary=7.97 cfs 0.447 af

Link Post-Dev.: Post-Dev

Inflow=0.23 cfs 0.141 af
Primary=0.23 cfs 0.141 af

Total Runoff Area = 3.940 ac Runoff Volume = 0.447 af Average Runoff Depth = 1.36"
70.30% Pervious = 2.770 ac 29.70% Impervious = 1.170 ac

23.0023 Post-Development Proposed Parcels 5-5-25

MSE 24-hr 3 10 yr Rainfall=3.97"

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Page 13

Summary for Subcatchment 2S: Onsite

Runoff = 7.97 cfs @ 12.21 hrs, Volume= 0.447 af, Depth> 1.36"

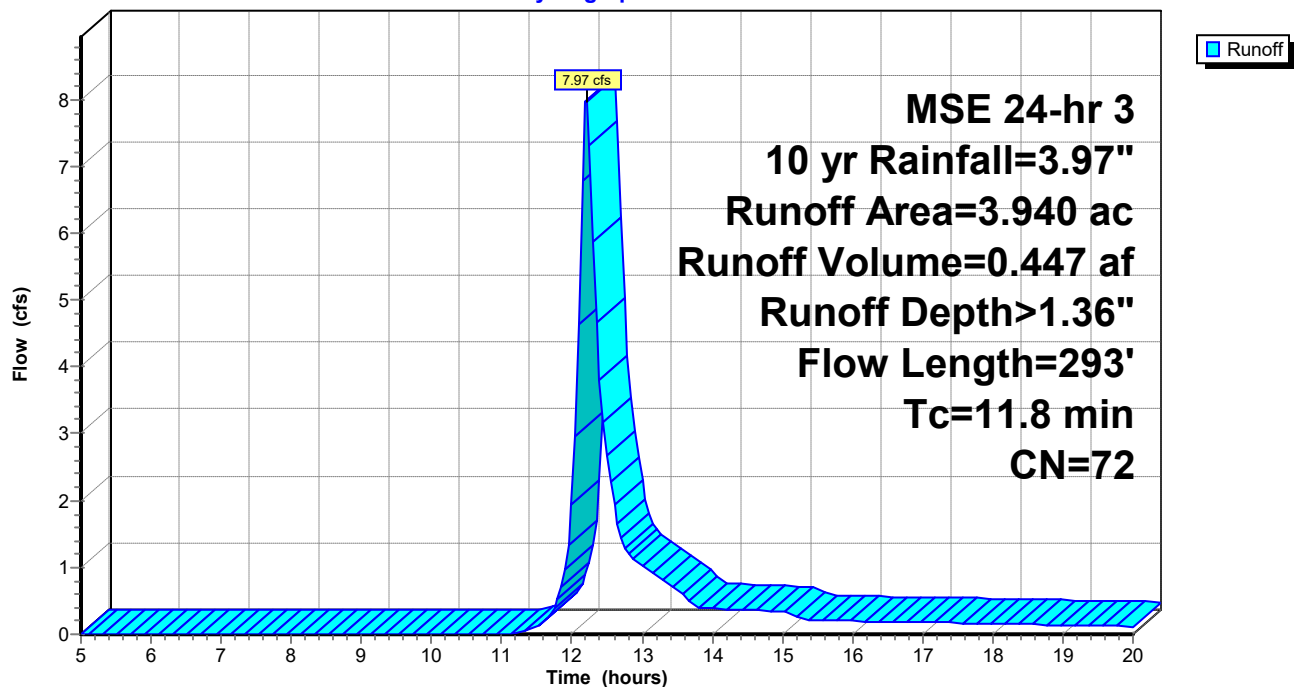
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
MSE 24-hr 3 10 yr Rainfall=3.97"

Area (ac)	CN	Description
0.350	98	Roofs, HSG B
* 0.330	98	Driveways, HSG B
0.490	98	Water Surface, HSG B
2.770	61	>75% Grass cover, Good, HSG B
3.940	72	Weighted Average
2.770		70.30% Pervious Area
1.170		29.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.3	16	0.0300	1.05		Sheet Flow, Sheet Flow Pavement Smooth surfaces n= 0.011 P2= 2.80"
11.3	84	0.0300	0.12		Sheet Flow, Sheet Flow-Grass Grass: Dense n= 0.240 P2= 2.80"
0.2	193	0.0850	14.44	288.83	Channel Flow, Channel-Grass Swale Area= 20.0 sf Perim= 20.0' r= 1.00' n= 0.030 Short grass
11.8	293	Total			

Subcatchment 2S: Onsite

Hydrograph



23.0023 Post-Development Proposed Parcels 5-5-25

MSE 24-hr 3 10 yr Rainfall=3.97"

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Page 14

Summary for Pond 1P: Wet Pond

Inflow Area = 3.940 ac, 29.70% Impervious, Inflow Depth > 1.36" for 10 yr event
 Inflow = 7.97 cfs @ 12.21 hrs, Volume= 0.447 af
 Outflow = 0.23 cfs @ 15.28 hrs, Volume= 0.141 af, Atten= 97%, Lag= 184.2 min
 Primary = 0.23 cfs @ 15.28 hrs, Volume= 0.141 af
 Secondary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 905.64' @ 15.28 hrs Surf.Area= 0.535 ac Storage= 0.329 af

Plug-Flow detention time= 244.8 min calculated for 0.141 af (32% of inflow)
 Center-of-Mass det. time= 168.6 min (970.8 - 802.1)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	1.785 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.490	0.000	0.000
906.00	0.560	0.525	0.525
907.00	0.630	0.595	1.120
908.00	0.700	0.665	1.785

Device	Routing	Invert	Outlet Devices
#1	Primary	905.00'	12.0" Round RCP_Round 12" L= 22.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 905.00' / 904.00' S= 0.0455 ' / Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf
#2	Device 1	905.00'	3.5" Vert. Orifice/Grate C= 0.600
#3	Device 1	906.90'	36.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Secondary	907.50'	10.0' long x 7.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.40 2.52 2.70 2.68 2.68 2.67 2.66 2.65 2.65 2.65 2.66 2.65 2.66 2.68 2.70 2.73 2.78

Primary OutFlow Max=0.23 cfs @ 15.28 hrs HW=905.64' (Free Discharge)

↑ **1=RCP_Round 12"** (Passes 0.23 cfs of 1.28 cfs potential flow)

↑ **2=Orifice/Grate** (Orifice Controls 0.23 cfs @ 3.39 fps)

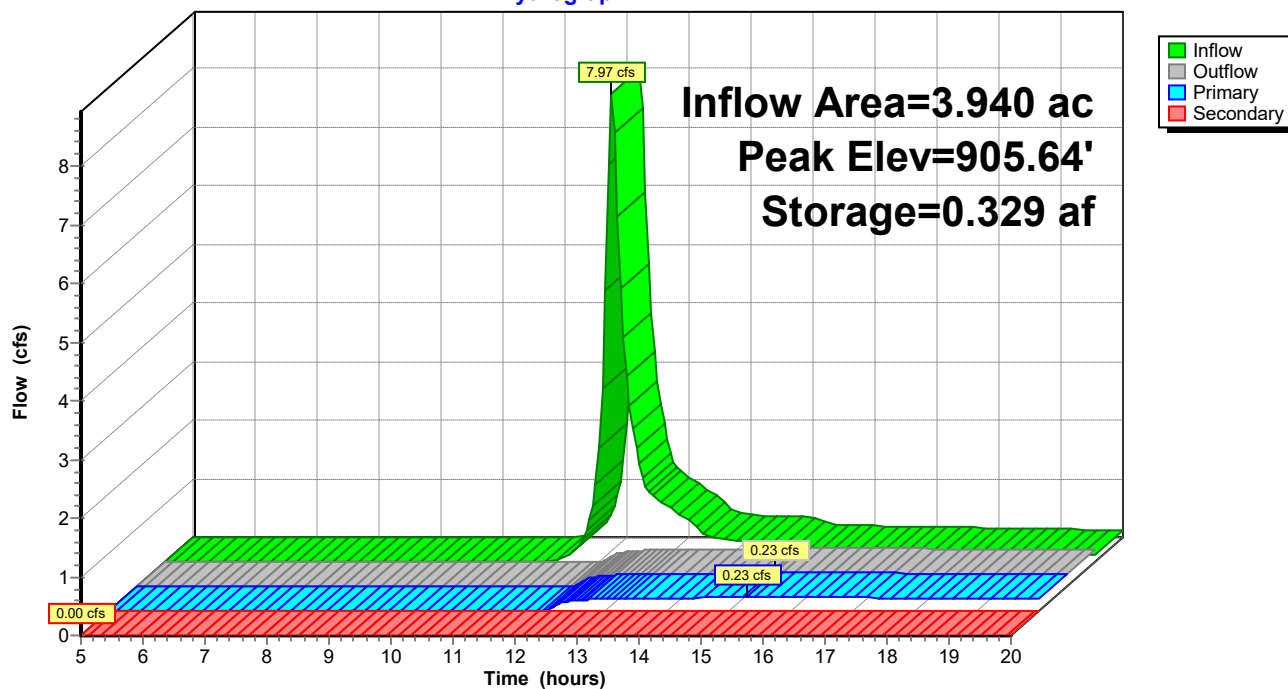
↑ **3=Orifice/Grate** (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=905.00' (Free Discharge)

↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond 1P: Wet Pond

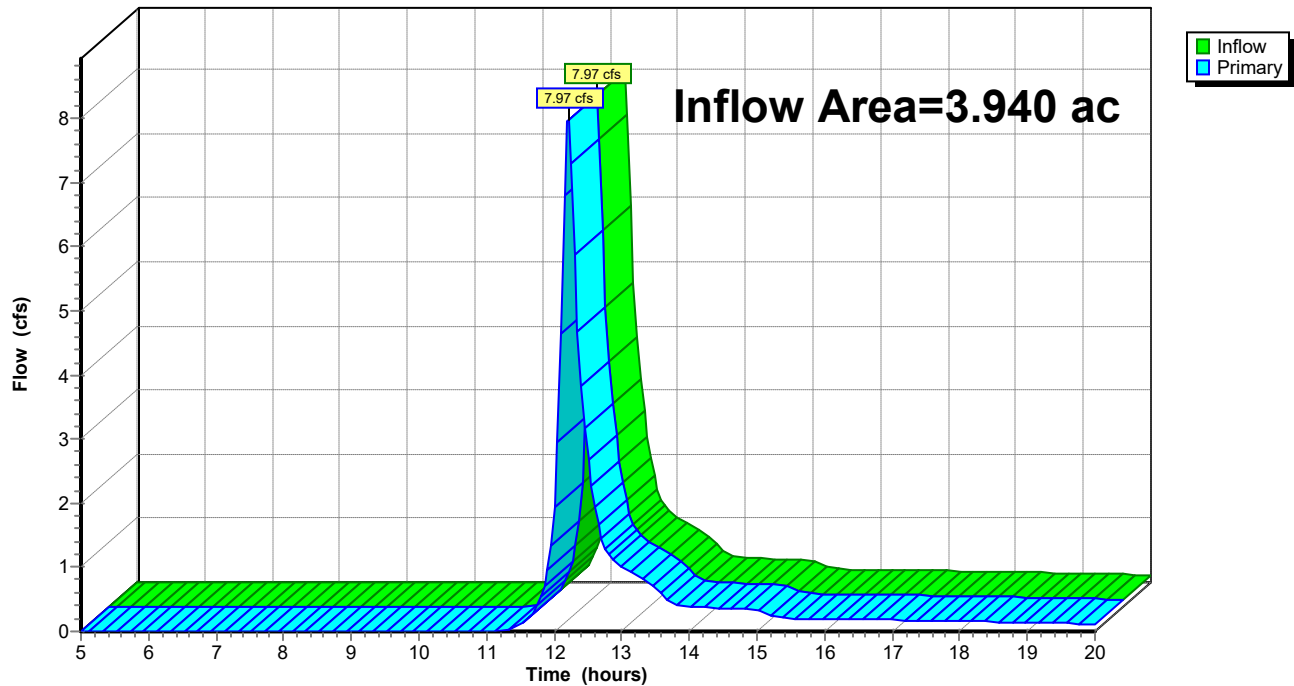
Hydrograph



Summary for Link 1L: (new Link)

Inflow Area = 3.940 ac, 29.70% Impervious, Inflow Depth > 1.36" for 10 yr event
Inflow = 7.97 cfs @ 12.21 hrs, Volume= 0.447 af
Primary = 7.97 cfs @ 12.21 hrs, Volume= 0.447 af, Atten= 0%, Lag= 0.0 min

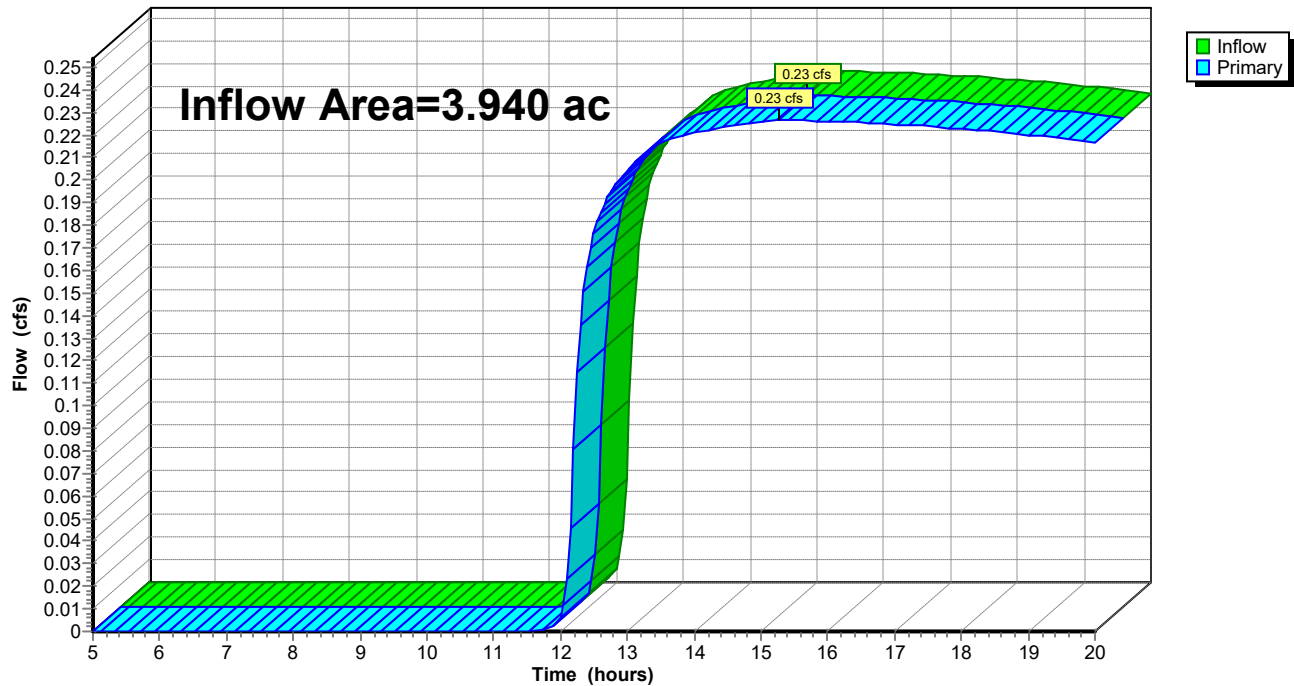
Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link 1L: (new Link)**Hydrograph**

Summary for Link Post-Dev.: Post-Dev

Inflow Area = 3.940 ac, 29.70% Impervious, Inflow Depth > 0.43" for 10 yr event
Inflow = 0.23 cfs @ 15.28 hrs, Volume= 0.141 af
Primary = 0.23 cfs @ 15.28 hrs, Volume= 0.141 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Post-Dev.: Post-Dev**Hydrograph**

23.0023 Post-Development Proposed Parcels 5-5-25*MSE 24-hr 3 100 yr Rainfall=6.22"*

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Page 18

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 2S: Onsite

Runoff Area=3.940 ac 29.70% Impervious Runoff Depth>3.03"
Flow Length=293' Tc=11.8 min CN=72 Runoff=17.91 cfs 0.995 af

Pond 1P: Wet Pond

Peak Elev=906.45' Storage=0.783 af Inflow=17.91 cfs 0.995 af
Primary=0.37 cfs 0.235 af Secondary=0.00 cfs 0.000 af Outflow=0.37 cfs 0.235 af

Link 1L: (new Link)

Inflow=17.91 cfs 0.995 af
Primary=17.91 cfs 0.995 af

Link Post-Dev.: Post-Dev

Inflow=0.37 cfs 0.235 af
Primary=0.37 cfs 0.235 af

Total Runoff Area = 3.940 ac Runoff Volume = 0.995 af Average Runoff Depth = 3.03"
70.30% Pervious = 2.770 ac 29.70% Impervious = 1.170 ac

23.0023 Post-Development Proposed Parcels 5-5-25

MSE 24-hr 3 100 yr Rainfall=6.22"

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Page 19

Summary for Subcatchment 2S: Onsite

Runoff = 17.91 cfs @ 12.20 hrs, Volume= 0.995 af, Depth> 3.03"

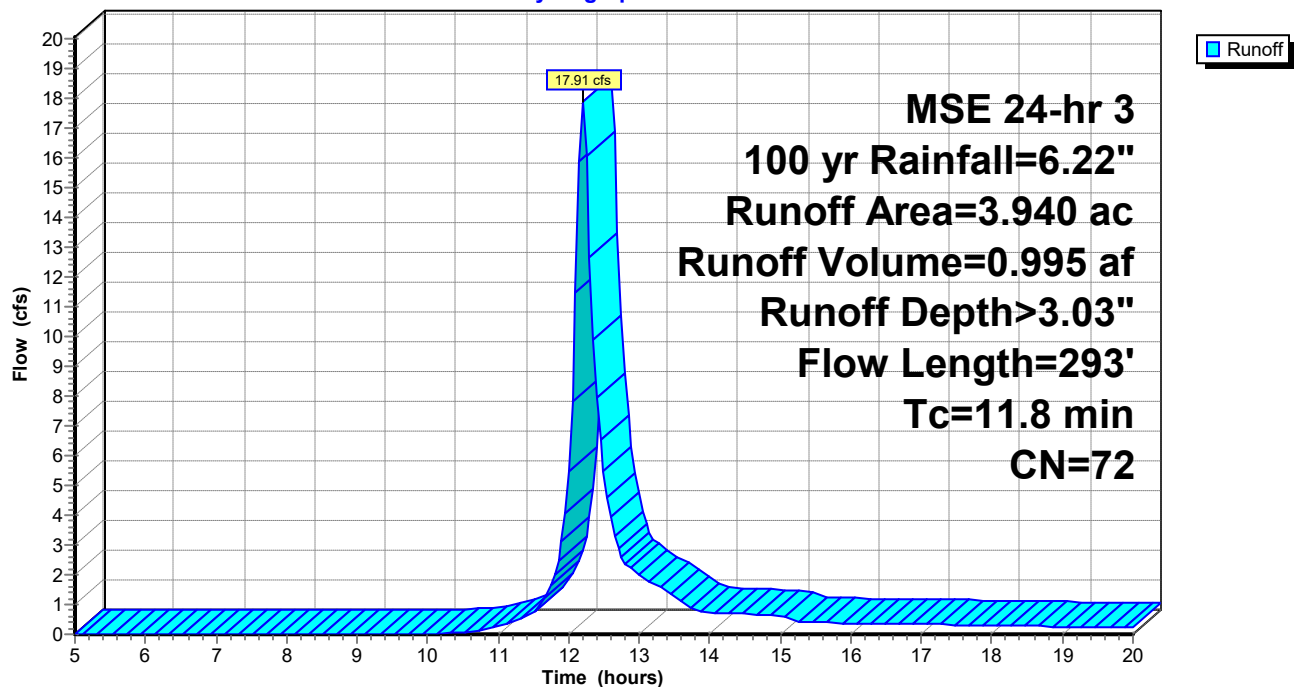
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
MSE 24-hr 3 100 yr Rainfall=6.22"

Area (ac)	CN	Description
0.350	98	Roofs, HSG B
* 0.330	98	Driveways, HSG B
0.490	98	Water Surface, HSG B
2.770	61	>75% Grass cover, Good, HSG B
3.940	72	Weighted Average
2.770		70.30% Pervious Area
1.170		29.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.3	16	0.0300	1.05		Sheet Flow, Sheet Flow Pavement Smooth surfaces n= 0.011 P2= 2.80"
11.3	84	0.0300	0.12		Sheet Flow, Sheet Flow-Grass Grass: Dense n= 0.240 P2= 2.80"
0.2	193	0.0850	14.44	288.83	Channel Flow, Channel-Grass Swale Area= 20.0 sf Perim= 20.0' r= 1.00' n= 0.030 Short grass
11.8	293	Total			

Subcatchment 2S: Onsite

Hydrograph



Summary for Pond 1P: Wet Pond

Inflow Area = 3.940 ac, 29.70% Impervious, Inflow Depth > 3.03" for 100 yr event
 Inflow = 17.91 cfs @ 12.20 hrs, Volume= 0.995 af
 Outflow = 0.37 cfs @ 16.13 hrs, Volume= 0.235 af, Atten= 98%, Lag= 235.6 min
 Primary = 0.37 cfs @ 16.13 hrs, Volume= 0.235 af
 Secondary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 906.45' @ 16.13 hrs Surf.Area= 0.591 ac Storage= 0.783 af

Plug-Flow detention time= 253.4 min calculated for 0.234 af (24% of inflow)
 Center-of-Mass det. time= 177.8 min (966.8 - 789.0)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	1.785 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.490	0.000	0.000
906.00	0.560	0.525	0.525
907.00	0.630	0.595	1.120
908.00	0.700	0.665	1.785

Device	Routing	Invert	Outlet Devices
#1	Primary	905.00'	12.0" Round RCP_Round 12" L= 22.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 905.00' / 904.00' S= 0.0455 ' / Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf
#2	Device 1	905.00'	3.5" Vert. Orifice/Grate C= 0.600
#3	Device 1	906.90'	36.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Secondary	907.50'	10.0' long x 7.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.40 2.52 2.70 2.68 2.68 2.67 2.66 2.65 2.65 2.65 2.66 2.65 2.66 2.68 2.70 2.73 2.78

Primary OutFlow Max=0.37 cfs @ 16.13 hrs HW=906.45' (Free Discharge)

↑ **1=RCP_Round 12"** (Passes 0.37 cfs of 3.25 cfs potential flow)

↑ **2=Orifice/Grate** (Orifice Controls 0.37 cfs @ 5.50 fps)

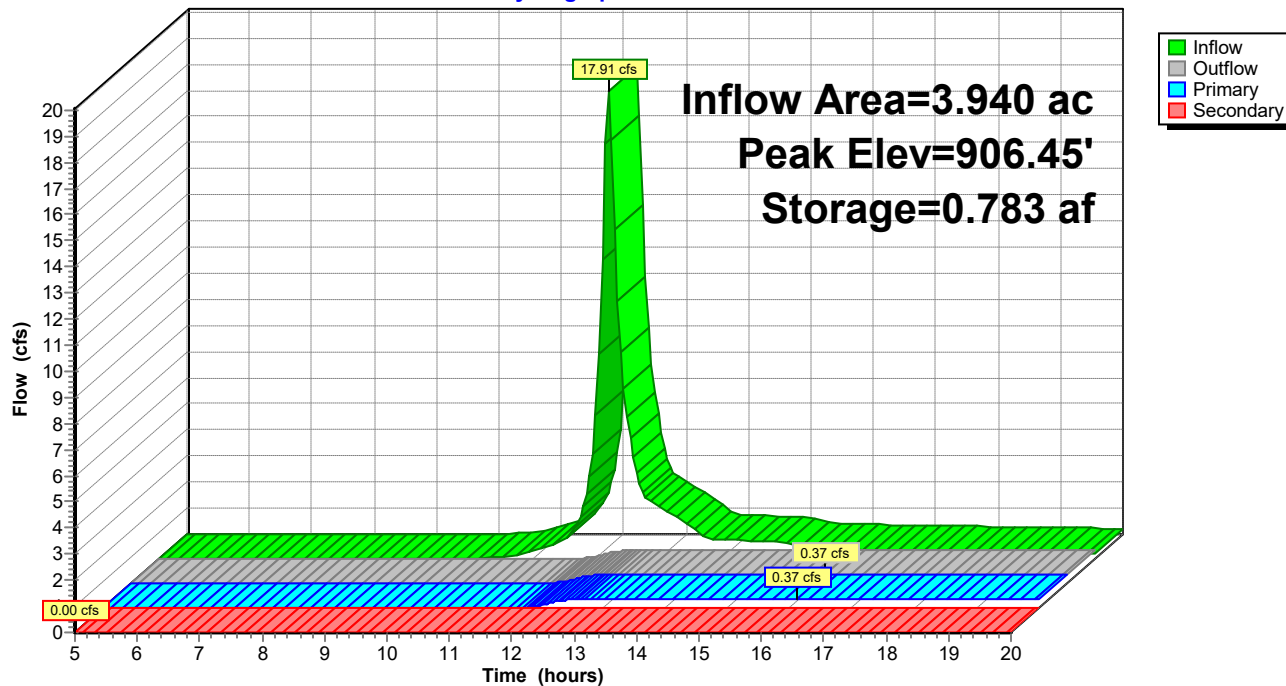
↑ **3=Orifice/Grate** (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=905.00' (Free Discharge)

↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond 1P: Wet Pond

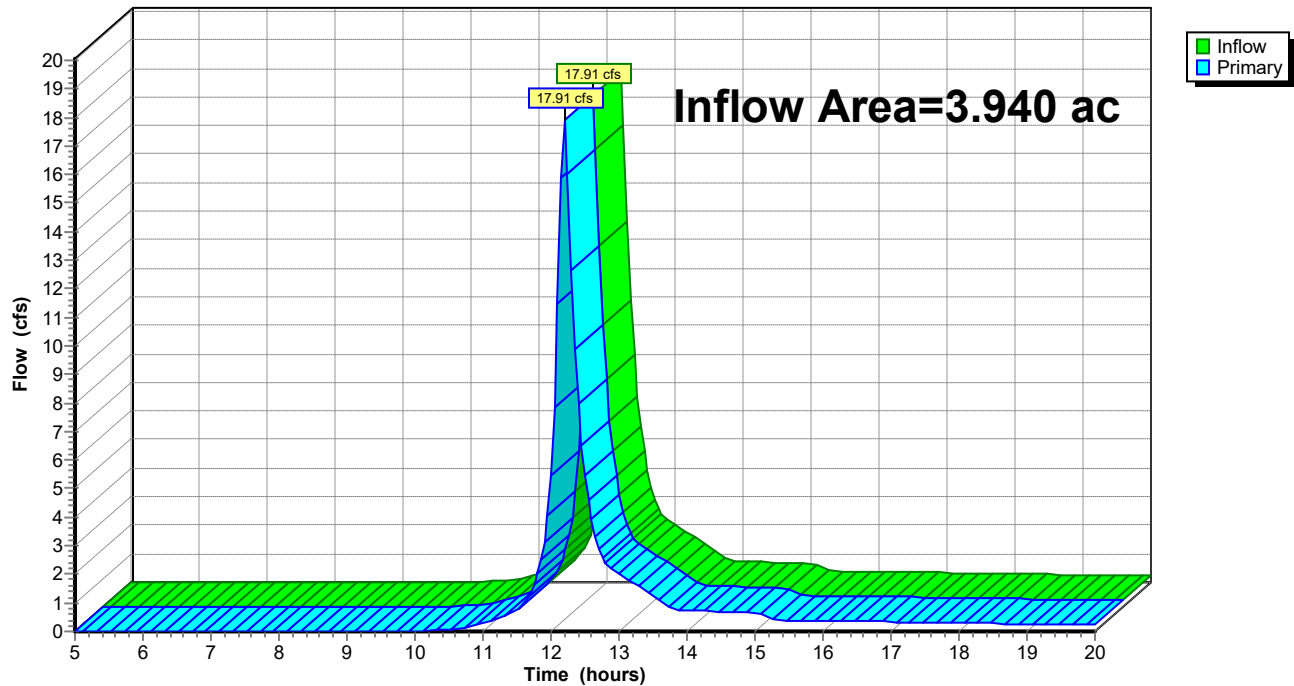
Hydrograph



Summary for Link 1L: (new Link)

Inflow Area = 3.940 ac, 29.70% Impervious, Inflow Depth > 3.03" for 100 yr event
Inflow = 17.91 cfs @ 12.20 hrs, Volume= 0.995 af
Primary = 17.91 cfs @ 12.20 hrs, Volume= 0.995 af, Atten= 0%, Lag= 0.0 min

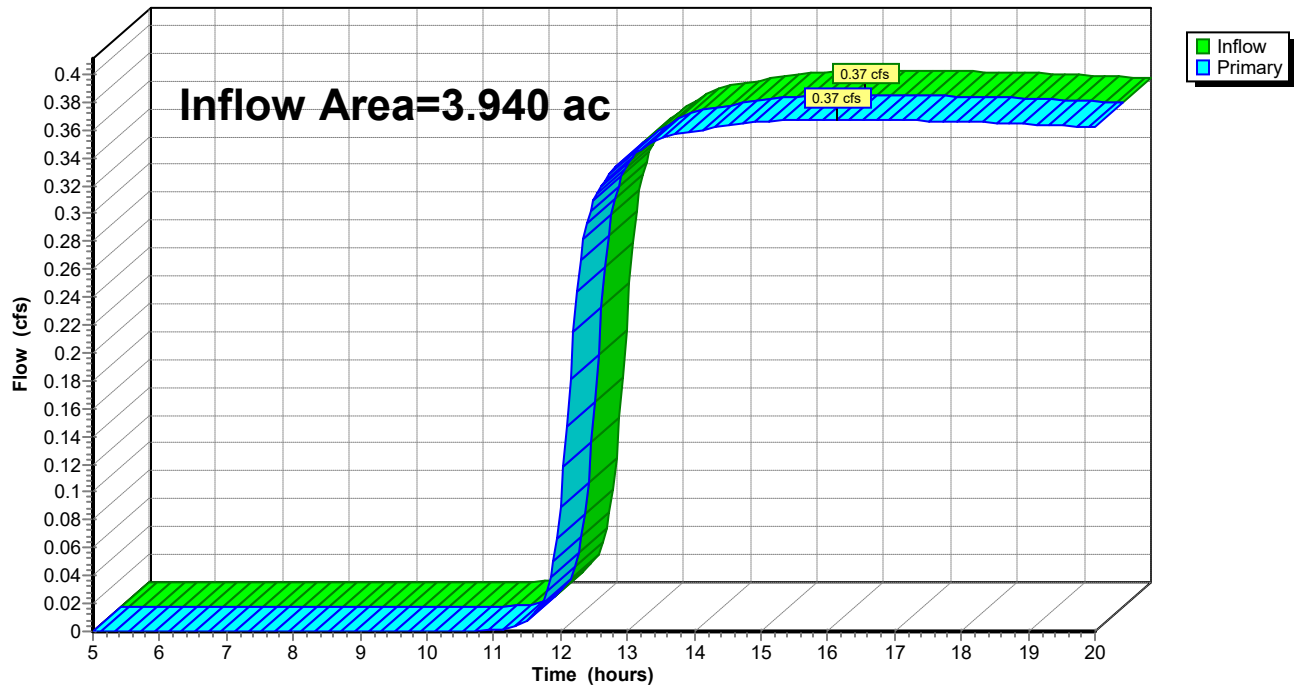
Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

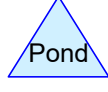
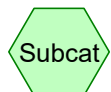
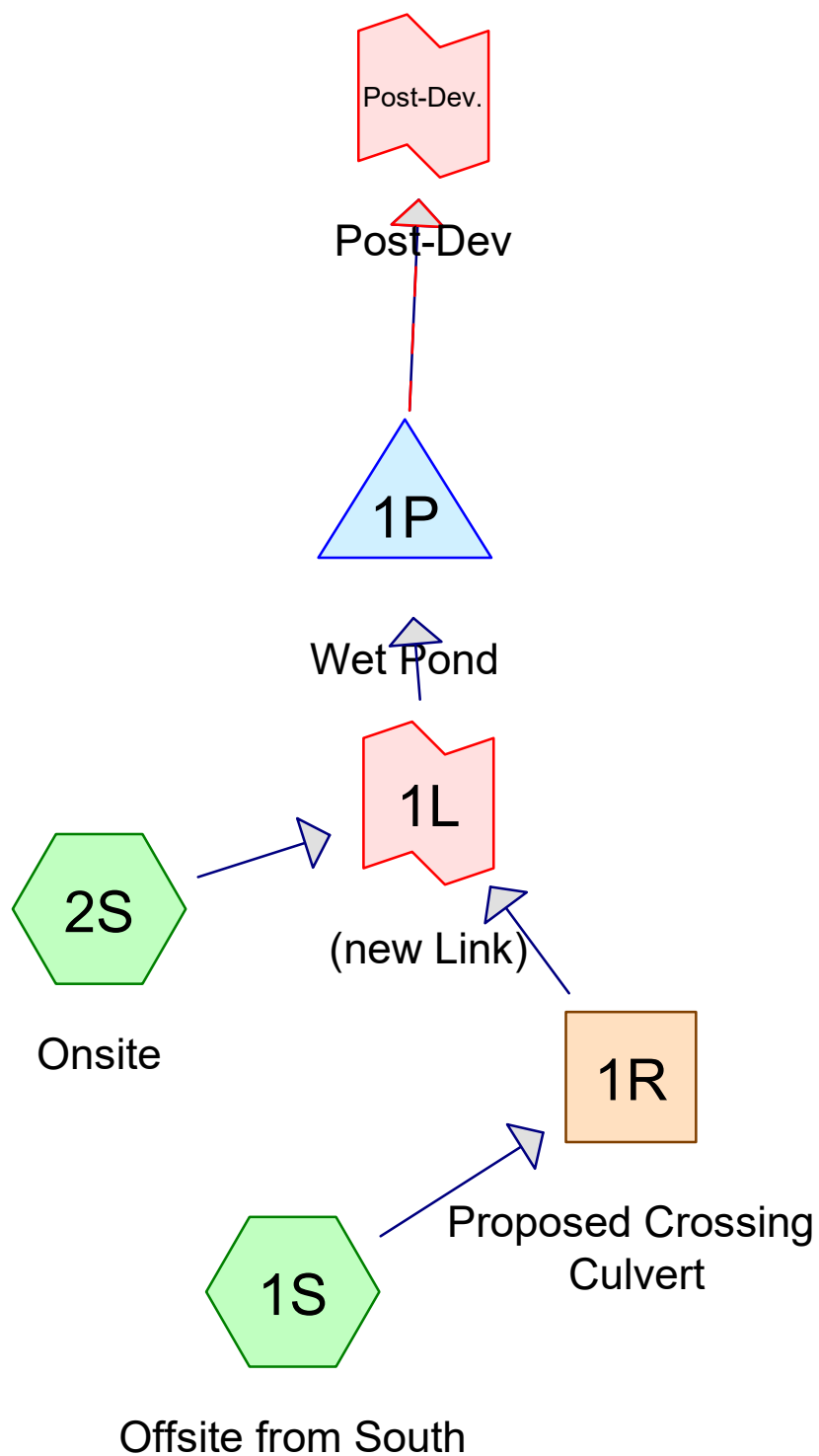
Link 1L: (new Link)**Hydrograph**

Summary for Link Post-Dev.: Post-Dev

Inflow Area = 3.940 ac, 29.70% Impervious, Inflow Depth > 0.72" for 100 yr event
Inflow = 0.37 cfs @ 16.13 hrs, Volume= 0.235 af
Primary = 0.37 cfs @ 16.13 hrs, Volume= 0.235 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Post-Dev.: Post-Dev**Hydrograph**



Routing Diagram for 23.0023 Post-Development Proposed Parcels Including Offsite 5-5-25
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23.0023 Post-Development Proposed Parcels Including Offsite 5-5-25

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Page 2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
3.000	72	1/3 acre lots, 30% imp, HSG B (1S)
2.770	61	>75% Grass cover, Good, HSG B (2S)
0.330	98	Driveways, HSG B (2S)
0.350	98	Roofs, HSG B (2S)
0.490	98	Water Surface, HSG B (2S)
6.940	72	TOTAL AREA

23.0023 Post-Development Proposed Parcels Including Offsite 5-5-25

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Page 3

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
6.940	HSG B	1S, 2S
0.000	HSG C	
0.000	HSG D	
0.000	Other	
6.940		TOTAL AREA

23.0023 Post-Development Proposed Parcels Including Offsite 5-5-25

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Page 4

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	3.000	0.000	0.000	0.000	3.000	1/3 acre lots, 30% imp	1S
0.000	2.770	0.000	0.000	0.000	2.770	>75% Grass cover, Good	2S
0.000	0.330	0.000	0.000	0.000	0.330	Driveways	2S
0.000	0.350	0.000	0.000	0.000	0.350	Roofs	2S
0.000	0.490	0.000	0.000	0.000	0.490	Water Surface	2S
0.000	6.940	0.000	0.000	0.000	6.940	TOTAL AREA	

23.0023 Post-Development Proposed Parcels Including Offsite 5-5-25

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Page 5

Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	1R	925.00	924.00	34.0	0.0294	0.011	12.0	0.0	0.0
2	1P	905.00	904.00	22.0	0.0455	0.011	12.0	0.0	0.0

23.0023 Post-Development Proposed Parcels Including Off MSE 24-hr 3 2 yr Rainfall=2.80"

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Page 6

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Offsite from South

Runoff Area=3.000 ac 30.00% Impervious Runoff Depth>0.65"
Flow Length=595' Tc=12.9 min CN=72 Runoff=2.59 cfs 0.162 af

Subcatchment 2S: Onsite

Runoff Area=3.940 ac 29.70% Impervious Runoff Depth>0.65"
Flow Length=293' Tc=11.8 min CN=72 Runoff=3.58 cfs 0.213 af

Reach 1R: Proposed Crossing Culvert

Avg. Flow Depth=0.41' Max Vel=8.41 fps Inflow=2.59 cfs 0.162 af
12.0" Round Pipe n=0.011 L=34.0' S=0.0294 '/' Capacity=7.22 cfs Outflow=2.58 cfs 0.162 af

Pond 1P: Wet Pond

Peak Elev=905.52' Storage=0.266 af Inflow=6.17 cfs 0.375 af
Primary=0.20 cfs 0.121 af Secondary=0.00 cfs 0.000 af Outflow=0.20 cfs 0.121 af

Link 1L: (new Link)

Inflow=6.17 cfs 0.375 af
Primary=6.17 cfs 0.375 af

Link Post-Dev.: Post-Dev

Inflow=0.20 cfs 0.121 af
Primary=0.20 cfs 0.121 af

Total Runoff Area = 6.940 ac Runoff Volume = 0.375 af Average Runoff Depth = 0.65"
70.17% Pervious = 4.870 ac 29.83% Impervious = 2.070 ac

Summary for Subcatchment 1S: Offsite from South

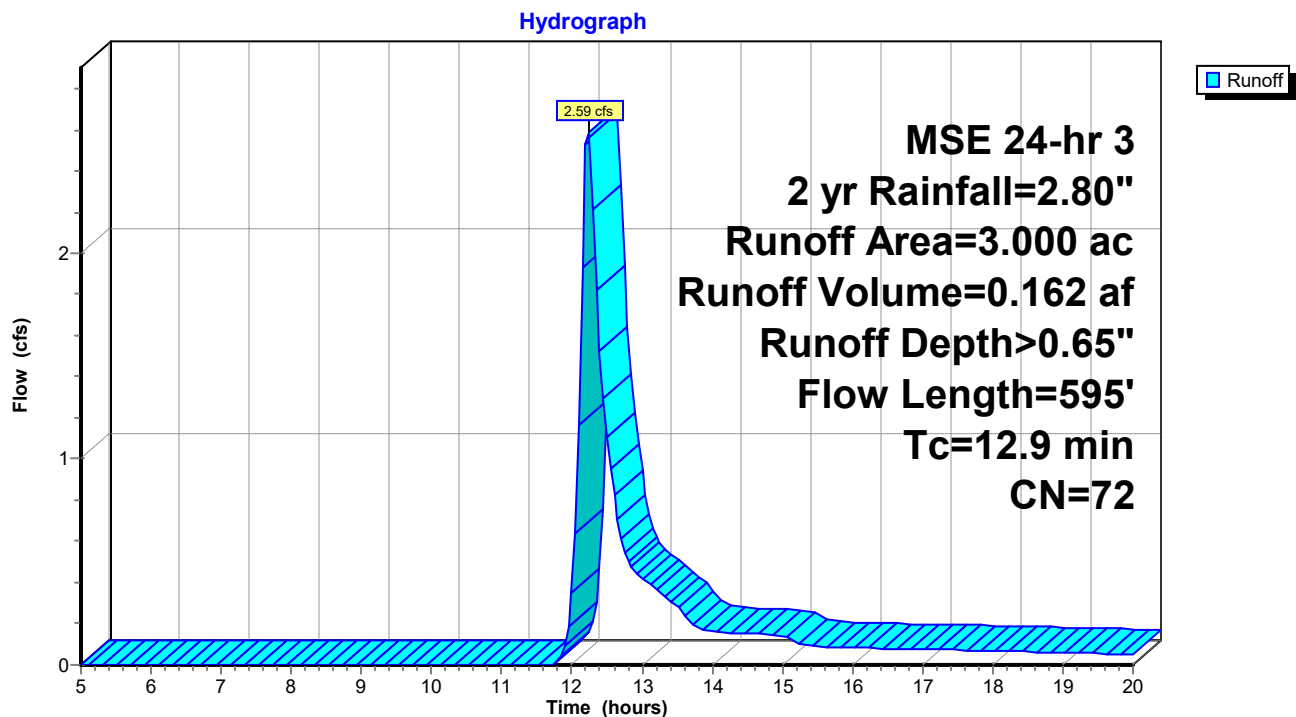
Runoff = 2.59 cfs @ 12.23 hrs, Volume= 0.162 af, Depth> 0.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 MSE 24-hr 3 2 yr Rainfall=2.80"

Area (ac)	CN	Description
3.000	72	1/3 acre lots, 30% imp, HSG B
2.100		70.00% Pervious Area
0.900		30.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.2	100	0.0700	0.18		Sheet Flow, Sheet Flow-Grass Grass: Dense n= 0.240 P2= 2.80"
1.8	185	0.0108	1.67		Shallow Concentrated Flow, Sh. Conc. Flow-Grass Unpaved Kv= 16.1 fps
1.4	250	0.0360	3.05		Shallow Concentrated Flow, Sh. Conc. Flow-Trees Unpaved Kv= 16.1 fps
0.5	60	0.0167	2.08		Shallow Concentrated Flow, Sh. Conc. Flow-Grass Unpaved Kv= 16.1 fps
12.9	595	Total			

Subcatchment 1S: Offsite from South



Summary for Subcatchment 2S: Onsite

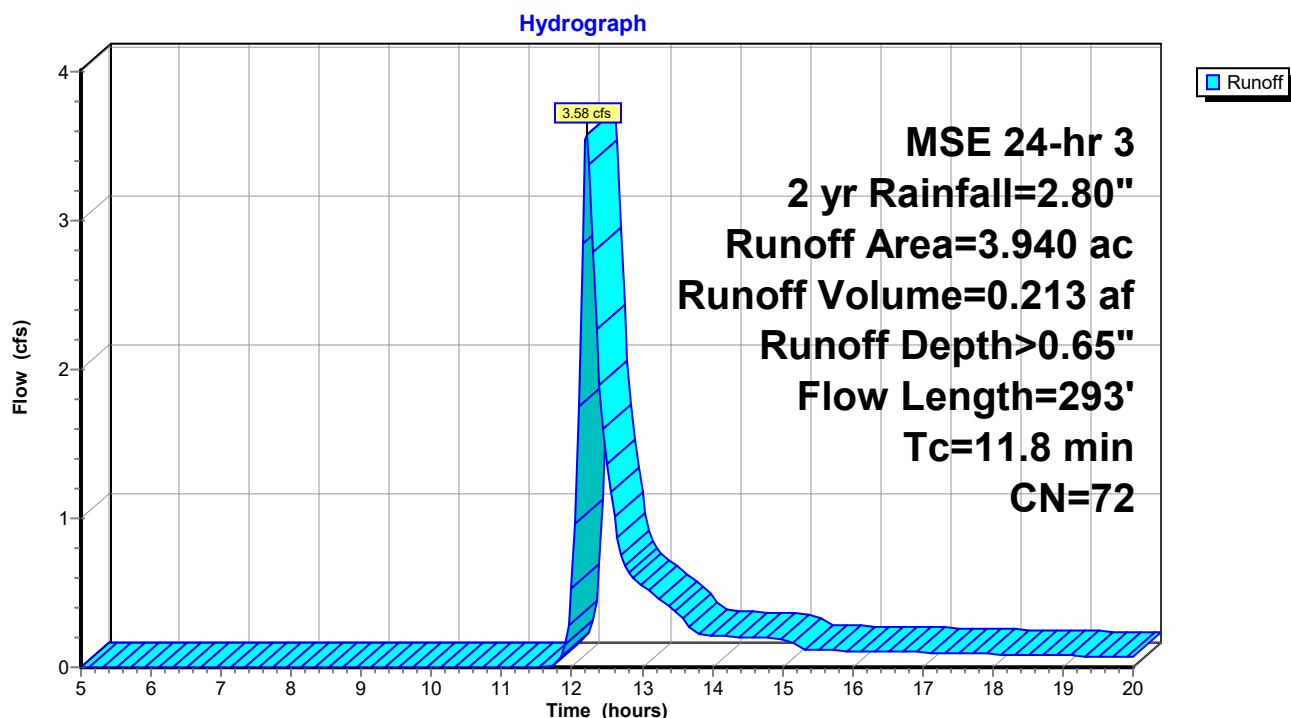
Runoff = 3.58 cfs @ 12.22 hrs, Volume= 0.213 af, Depth> 0.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
MSE 24-hr 3 2 yr Rainfall=2.80"

Area (ac)	CN	Description
0.350	98	Roofs, HSG B
* 0.330	98	Driveways, HSG B
0.490	98	Water Surface, HSG B
2.770	61	>75% Grass cover, Good, HSG B
3.940	72	Weighted Average
2.770		70.30% Pervious Area
1.170		29.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.3	16	0.0300	1.05		Sheet Flow, Sheet Flow Pavement Smooth surfaces n= 0.011 P2= 2.80"
11.3	84	0.0300	0.12		Sheet Flow, Sheet Flow-Grass Grass: Dense n= 0.240 P2= 2.80"
0.2	193	0.0850	14.44	288.83	Channel Flow, Channel-Grass Swale Area= 20.0 sf Perim= 20.0' r= 1.00' n= 0.030 Short grass
11.8	293	Total			

Subcatchment 2S: Onsite



Summary for Reach 1R: Proposed Crossing Culvert

Inflow Area = 3.000 ac, 30.00% Impervious, Inflow Depth > 0.65" for 2 yr event
Inflow = 2.59 cfs @ 12.23 hrs, Volume= 0.162 af
Outflow = 2.58 cfs @ 12.23 hrs, Volume= 0.162 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 8.41 fps, Min. Travel Time= 0.1 min

Avg. Velocity= 3.62 fps, Avg. Travel Time= 0.2 min

Peak Storage= 10 cf @ 12.23 hrs

Average Depth at Peak Storage= 0.41'

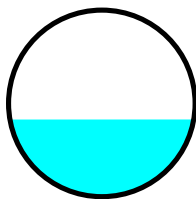
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.22 cfs

12.0" Round Pipe

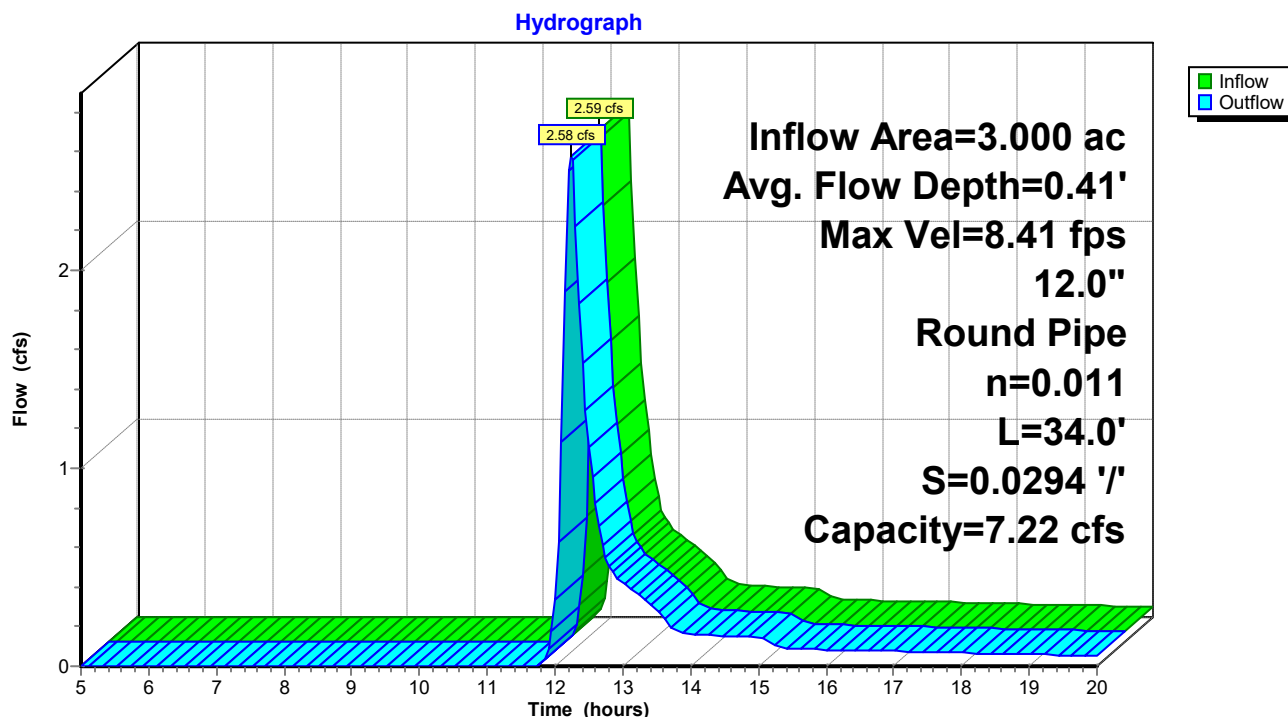
n= 0.011 Concrete pipe, straight & clean

Length= 34.0' Slope= 0.0294 '/'

Inlet Invert= 925.00', Outlet Invert= 924.00'



Reach 1R: Proposed Crossing Culvert



23.0023 Post-Development Proposed Parcels Including Off MSE 24-hr 3 2 yr Rainfall=2.80"

Prepared by {enter your company name here}

Printed 4/18/2025

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Page 10

Summary for Pond 1P: Wet Pond

Inflow Area = 6.940 ac, 29.83% Impervious, Inflow Depth > 0.65" for 2 yr event
 Inflow = 6.17 cfs @ 12.22 hrs, Volume= 0.375 af
 Outflow = 0.20 cfs @ 15.79 hrs, Volume= 0.121 af, Atten= 97%, Lag= 214.1 min
 Primary = 0.20 cfs @ 15.79 hrs, Volume= 0.121 af
 Secondary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 905.52' @ 15.79 hrs Surf.Area= 0.527 ac Storage= 0.266 af

Plug-Flow detention time= 242.8 min calculated for 0.121 af (32% of inflow)
 Center-of-Mass det. time= 159.3 min (975.2 - 815.9)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	1.785 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.490	0.000	0.000
906.00	0.560	0.525	0.525
907.00	0.630	0.595	1.120
908.00	0.700	0.665	1.785

Device	Routing	Invert	Outlet Devices
#1	Primary	905.00'	12.0" Round RCP_Round 12" L= 22.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 905.00' / 904.00' S= 0.0455 ' / Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf
#2	Device 1	905.00'	3.5" Vert. Orifice/Grate C= 0.600
#3	Device 1	906.90'	36.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Secondary	907.50'	10.0' long x 7.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.40 2.52 2.70 2.68 2.68 2.67 2.66 2.65 2.65 2.65 2.66 2.65 2.66 2.68 2.70 2.73 2.78

Primary OutFlow Max=0.20 cfs @ 15.79 hrs HW=905.52' (Free Discharge)

↑ **1=RCP_Round 12"** (Passes 0.20 cfs of 0.91 cfs potential flow)

↑ **2=Orifice/Grate** (Orifice Controls 0.20 cfs @ 2.96 fps)

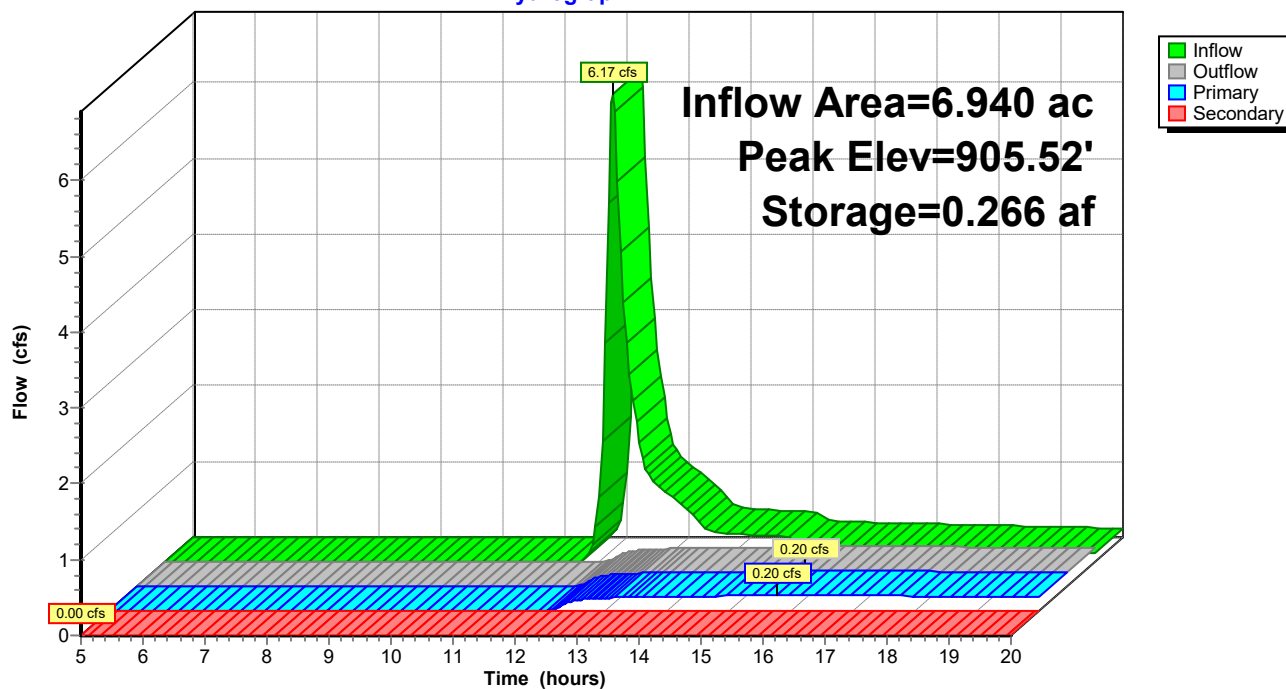
↑ **3=Orifice/Grate** (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=905.00' (Free Discharge)

↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond 1P: Wet Pond

Hydrograph



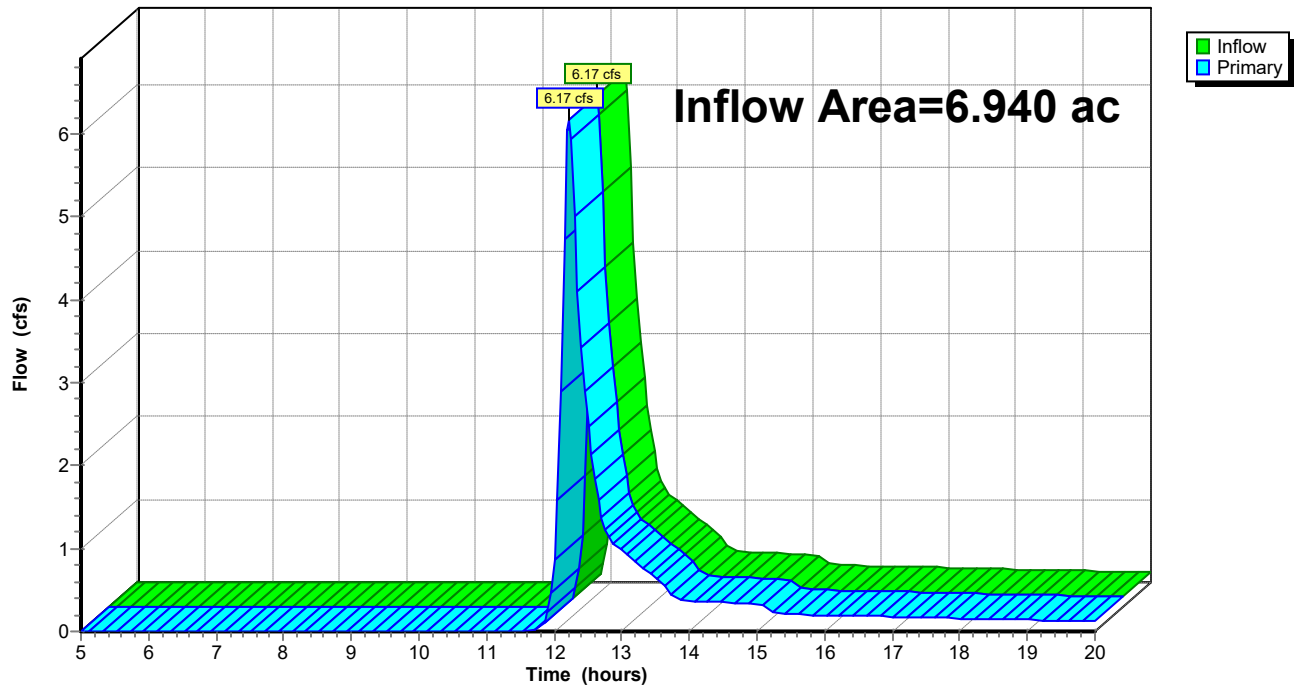
Summary for Link 1L: (new Link)

Inflow Area = 6.940 ac, 29.83% Impervious, Inflow Depth > 0.65" for 2 yr event
Inflow = 6.17 cfs @ 12.22 hrs, Volume= 0.375 af
Primary = 6.17 cfs @ 12.22 hrs, Volume= 0.375 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link 1L: (new Link)

Hydrograph



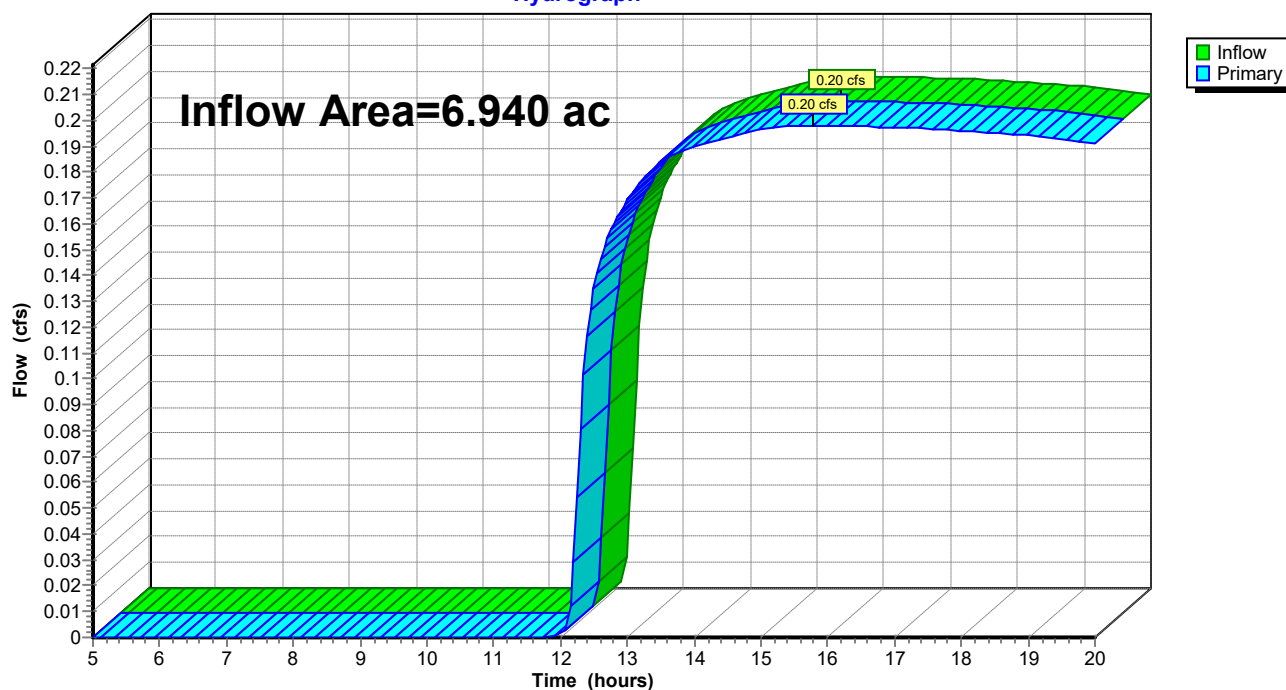
Summary for Link Post-Dev.: Post-Dev

Inflow Area = 6.940 ac, 29.83% Impervious, Inflow Depth > 0.21" for 2 yr event
Inflow = 0.20 cfs @ 15.79 hrs, Volume= 0.121 af
Primary = 0.20 cfs @ 15.79 hrs, Volume= 0.121 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Post-Dev.: Post-Dev

Hydrograph



23.0023 Post-Development Proposed Parcels Including O MSE 24-hr 3 10 yr Rainfall=3.97"

Prepared by {enter your company name here}

Printed 4/18/2025

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Page 14

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Offsite from South

Runoff Area=3.000 ac 30.00% Impervious Runoff Depth>1.36"
Flow Length=595' Tc=12.9 min CN=72 Runoff=5.84 cfs 0.341 af

Subcatchment 2S: Onsite

Runoff Area=3.940 ac 29.70% Impervious Runoff Depth>1.36"
Flow Length=293' Tc=11.8 min CN=72 Runoff=7.97 cfs 0.447 af

Reach 1R: Proposed Crossing Culvert

Avg. Flow Depth=0.68' Max Vel=10.21 fps Inflow=5.84 cfs 0.341 af
12.0" Round Pipe n=0.011 L=34.0' S=0.0294 '/' Capacity=7.22 cfs Outflow=5.83 cfs 0.341 af

Pond 1P: Wet Pond

Peak Elev=906.13' Storage=0.601 af Inflow=13.78 cfs 0.788 af
Primary=0.32 cfs 0.201 af Secondary=0.00 cfs 0.000 af Outflow=0.32 cfs 0.201 af

Link 1L: (new Link)

Inflow=13.78 cfs 0.788 af
Primary=13.78 cfs 0.788 af

Link Post-Dev.: Post-Dev

Inflow=0.32 cfs 0.201 af
Primary=0.32 cfs 0.201 af

Total Runoff Area = 6.940 ac Runoff Volume = 0.788 af Average Runoff Depth = 1.36"
70.17% Pervious = 4.870 ac 29.83% Impervious = 2.070 ac

Summary for Subcatchment 1S: Offsite from South

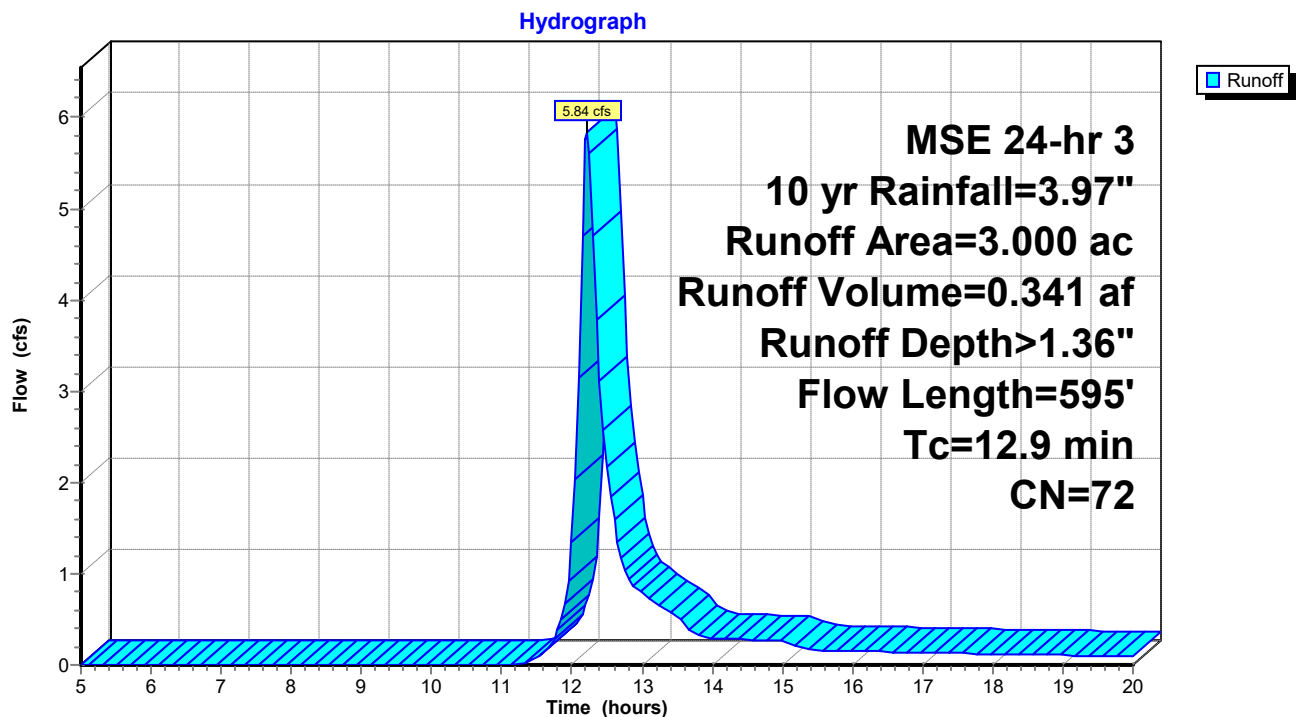
Runoff = 5.84 cfs @ 12.22 hrs, Volume= 0.341 af, Depth> 1.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
MSE 24-hr 3 10 yr Rainfall=3.97"

Area (ac)	CN	Description
3.000	72	1/3 acre lots, 30% imp, HSG B
2.100		70.00% Pervious Area
0.900		30.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.2	100	0.0700	0.18		Sheet Flow, Sheet Flow-Grass Grass: Dense n= 0.240 P2= 2.80"
1.8	185	0.0108	1.67		Shallow Concentrated Flow, Sh. Conc. Flow-Grass Unpaved Kv= 16.1 fps
1.4	250	0.0360	3.05		Shallow Concentrated Flow, Sh. Conc. Flow-Trees Unpaved Kv= 16.1 fps
0.5	60	0.0167	2.08		Shallow Concentrated Flow, Sh. Conc. Flow-Grass Unpaved Kv= 16.1 fps
12.9	595	Total			

Subcatchment 1S: Offsite from South



Summary for Subcatchment 2S: Onsite

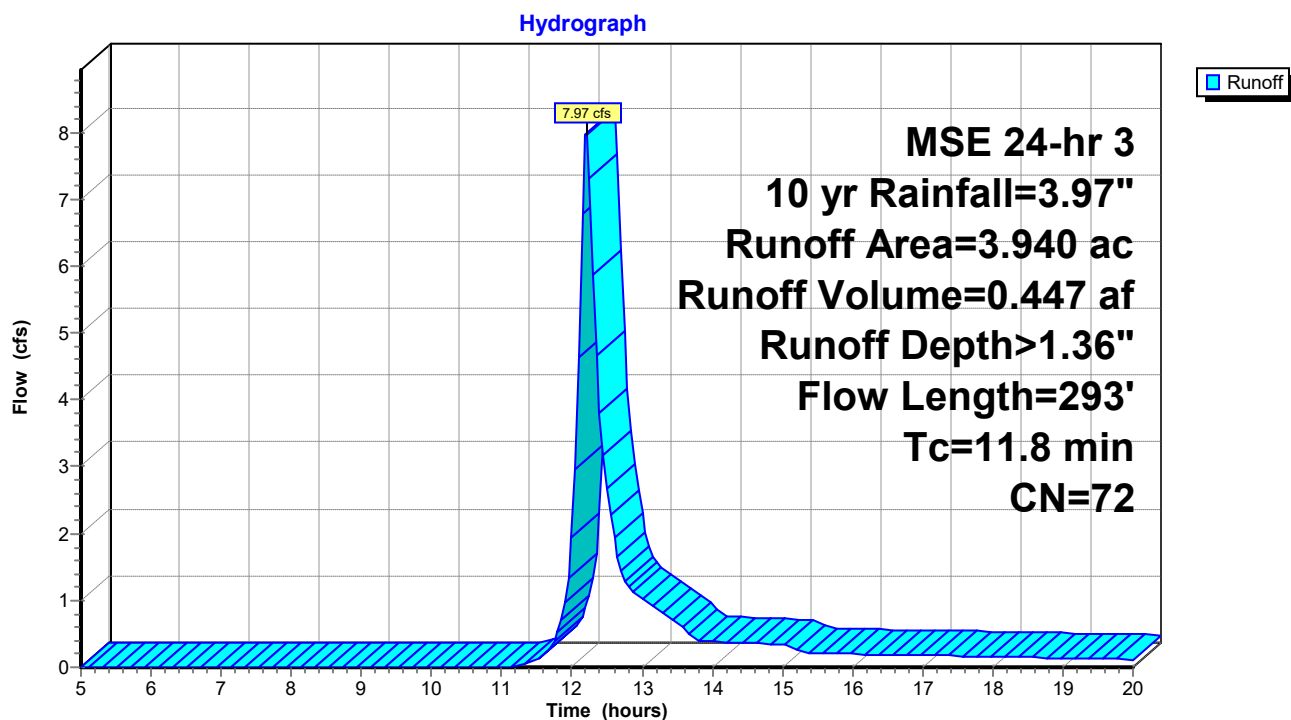
Runoff = 7.97 cfs @ 12.21 hrs, Volume= 0.447 af, Depth> 1.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 MSE 24-hr 3 10 yr Rainfall=3.97"

Area (ac)	CN	Description
0.350	98	Roofs, HSG B
* 0.330	98	Driveways, HSG B
0.490	98	Water Surface, HSG B
2.770	61	>75% Grass cover, Good, HSG B
3.940	72	Weighted Average
2.770		70.30% Pervious Area
1.170		29.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.3	16	0.0300	1.05		Sheet Flow, Sheet Flow Pavement Smooth surfaces n= 0.011 P2= 2.80"
11.3	84	0.0300	0.12		Sheet Flow, Sheet Flow-Grass Grass: Dense n= 0.240 P2= 2.80"
0.2	193	0.0850	14.44	288.83	Channel Flow, Channel-Grass Swale Area= 20.0 sf Perim= 20.0' r= 1.00' n= 0.030 Short grass
11.8	293	Total			

Subcatchment 2S: Onsite



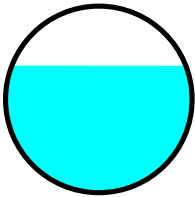
Summary for Reach 1R: Proposed Crossing Culvert

Inflow Area = 3.000 ac, 30.00% Impervious, Inflow Depth > 1.36" for 10 yr event
 Inflow = 5.84 cfs @ 12.22 hrs, Volume= 0.341 af
 Outflow = 5.83 cfs @ 12.22 hrs, Volume= 0.341 af, Atten= 0%, Lag= 0.1 min

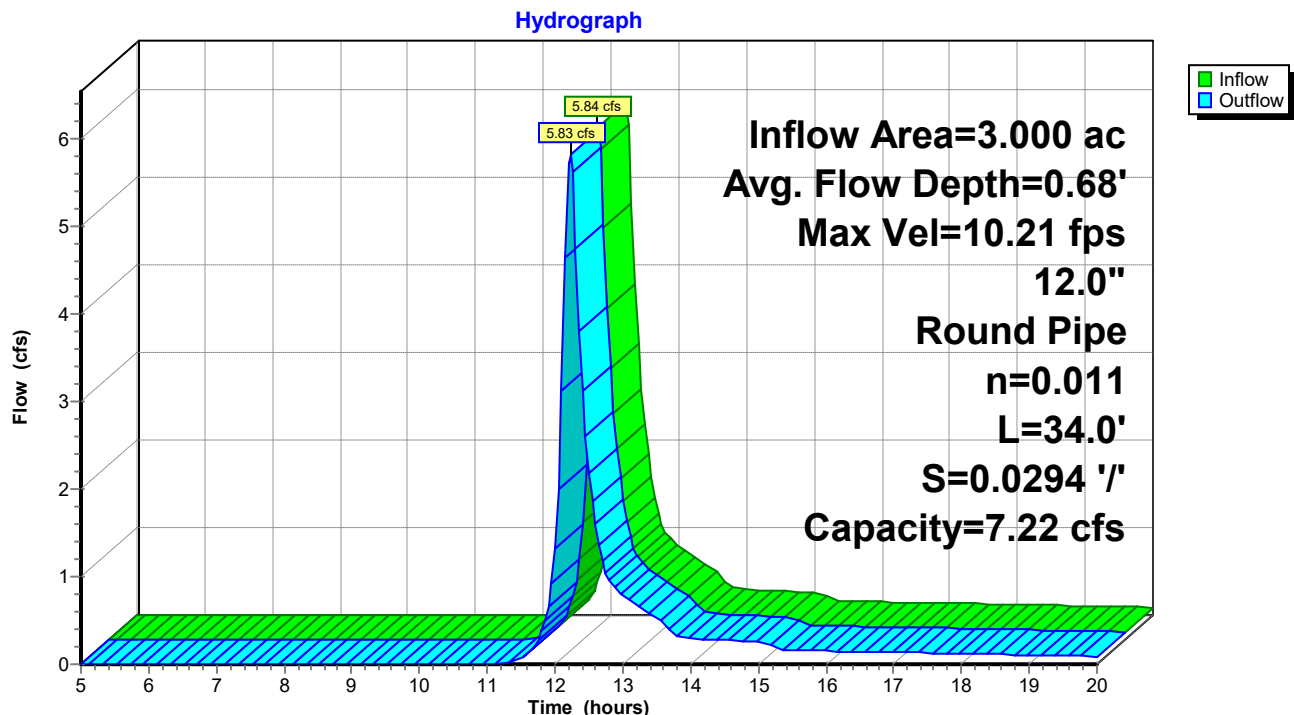
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Max. Velocity= 10.21 fps, Min. Travel Time= 0.1 min
 Avg. Velocity = 4.29 fps, Avg. Travel Time= 0.1 min

Peak Storage= 19 cf @ 12.22 hrs
 Average Depth at Peak Storage= 0.68'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.22 cfs

12.0" Round Pipe
 n= 0.011 Concrete pipe, straight & clean
 Length= 34.0' Slope= 0.0294 '/'
 Inlet Invert= 925.00', Outlet Invert= 924.00'



Reach 1R: Proposed Crossing Culvert



Summary for Pond 1P: Wet Pond

Inflow Area = 6.940 ac, 29.83% Impervious, Inflow Depth > 1.36" for 10 yr event
 Inflow = 13.78 cfs @ 12.21 hrs, Volume= 0.788 af
 Outflow = 0.32 cfs @ 16.81 hrs, Volume= 0.201 af, Atten= 98%, Lag= 276.0 min
 Primary = 0.32 cfs @ 16.81 hrs, Volume= 0.201 af
 Secondary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 906.13' @ 16.81 hrs Surf.Area= 0.569 ac Storage= 0.601 af

Plug-Flow detention time= 246.1 min calculated for 0.201 af (25% of inflow)
 Center-of-Mass det. time= 168.7 min (971.3 - 802.6)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	1.785 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.490	0.000	0.000
906.00	0.560	0.525	0.525
907.00	0.630	0.595	1.120
908.00	0.700	0.665	1.785

Device	Routing	Invert	Outlet Devices
#1	Primary	905.00'	12.0" Round RCP_Round 12" L= 22.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 905.00' / 904.00' S= 0.0455 ' / Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf
#2	Device 1	905.00'	3.5" Vert. Orifice/Grate C= 0.600
#3	Device 1	906.90'	36.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Secondary	907.50'	10.0' long x 7.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.40 2.52 2.70 2.68 2.68 2.67 2.66 2.65 2.65 2.65 2.66 2.65 2.66 2.68 2.70 2.73 2.78

Primary OutFlow Max=0.32 cfs @ 16.81 hrs HW=906.13' (Free Discharge)

↑ **1=RCP_Round 12"** (Passes 0.32 cfs of 2.66 cfs potential flow)

↑ **2=Orifice/Grate** (Orifice Controls 0.32 cfs @ 4.79 fps)

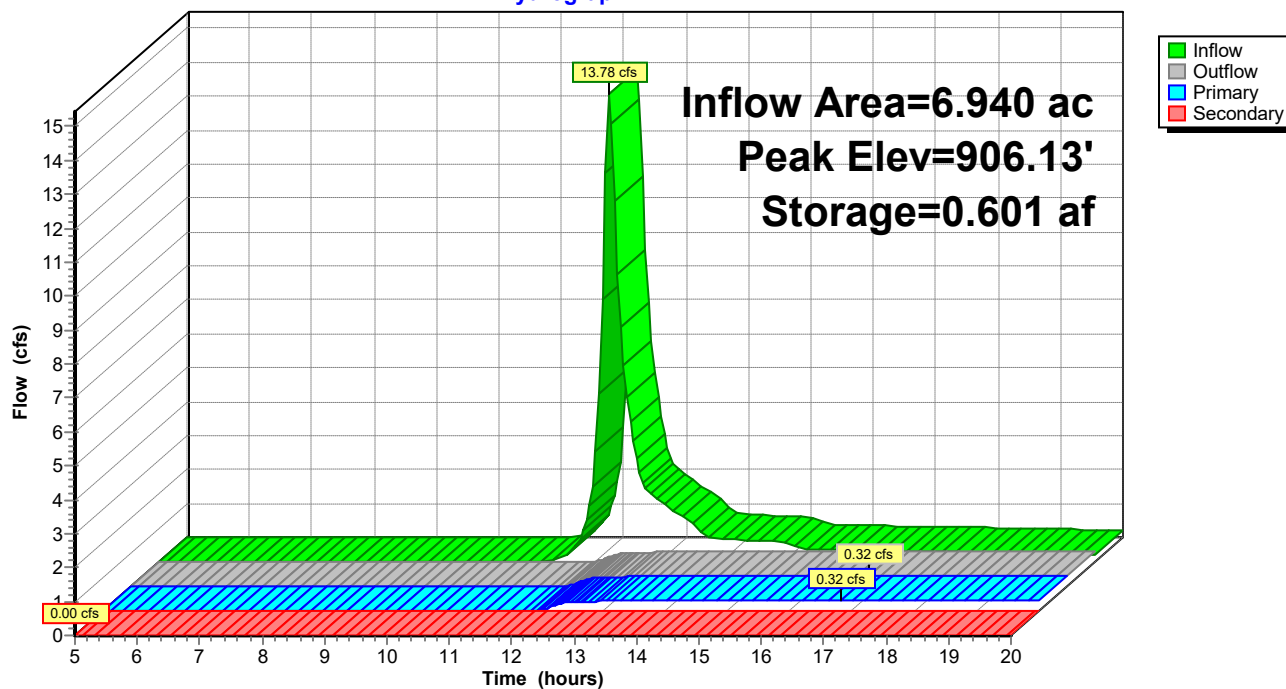
↑ **3=Orifice/Grate** (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=905.00' (Free Discharge)

↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond 1P: Wet Pond

Hydrograph



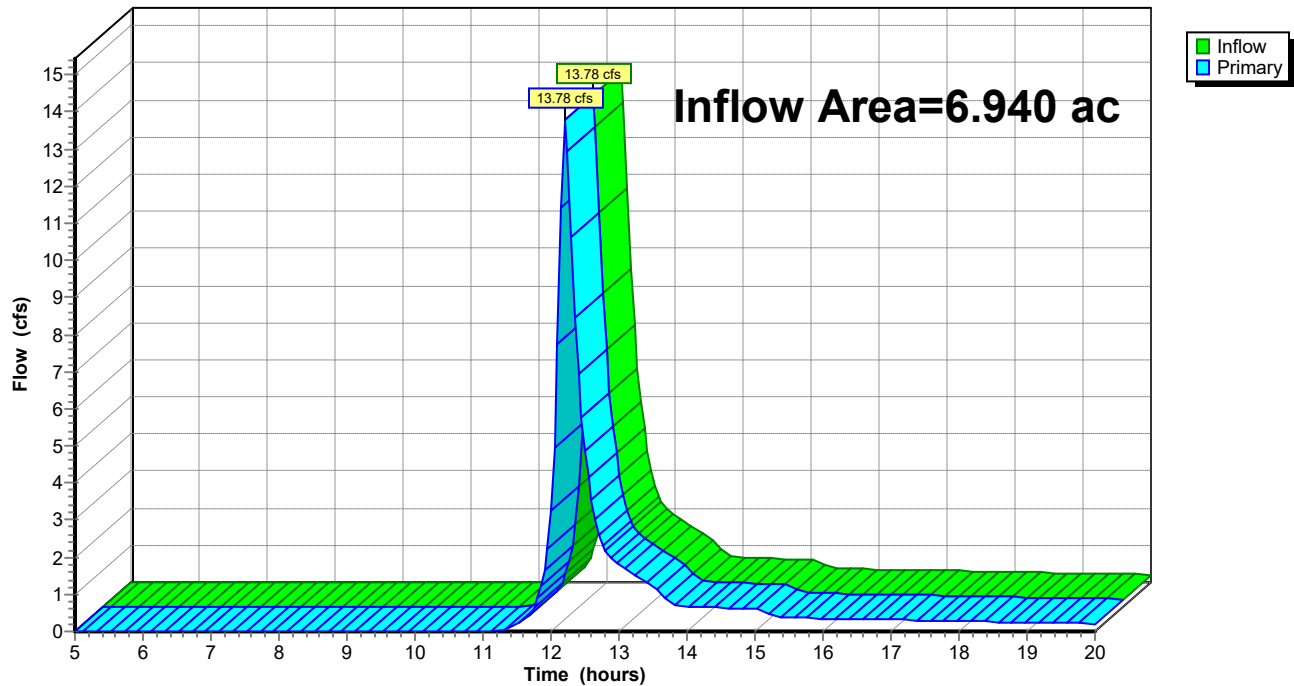
Summary for Link 1L: (new Link)

Inflow Area = 6.940 ac, 29.83% Impervious, Inflow Depth > 1.36" for 10 yr event
 Inflow = 13.78 cfs @ 12.21 hrs, Volume= 0.788 af
 Primary = 13.78 cfs @ 12.21 hrs, Volume= 0.788 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link 1L: (new Link)

Hydrograph

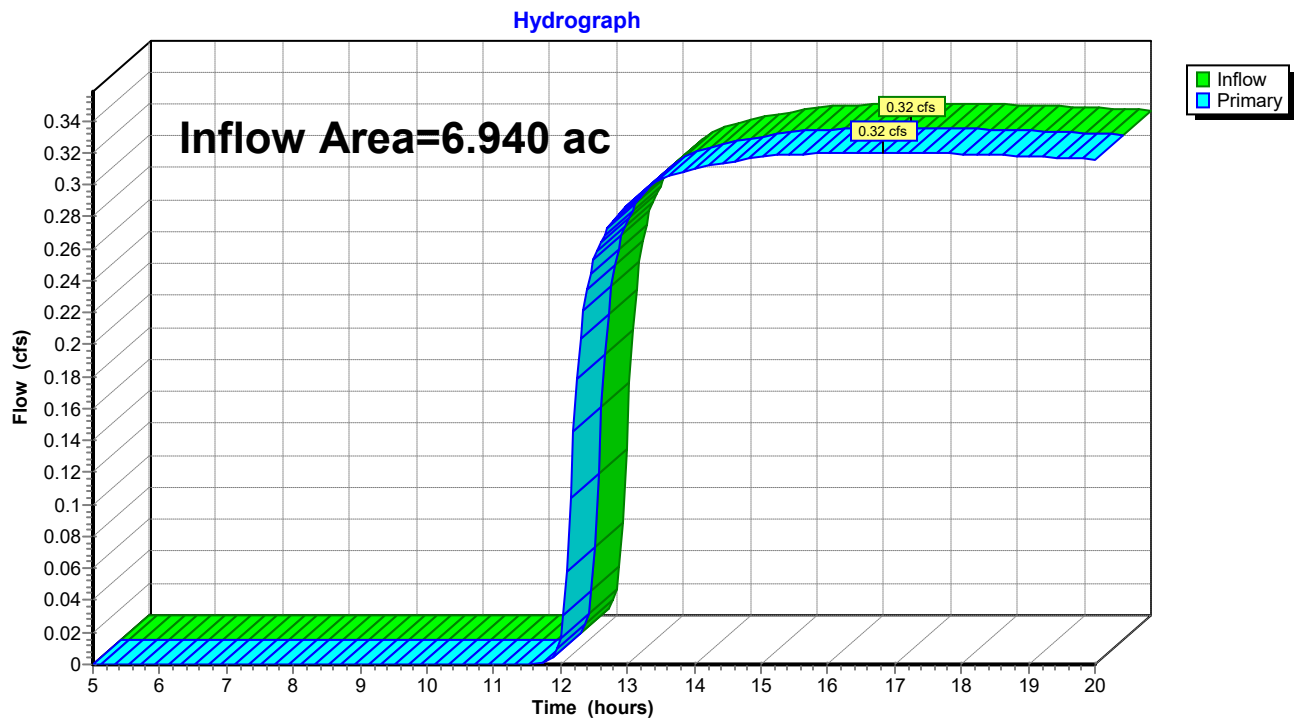


Summary for Link Post-Dev.: Post-Dev

Inflow Area = 6.940 ac, 29.83% Impervious, Inflow Depth > 0.35" for 10 yr event
Inflow = 0.32 cfs @ 16.81 hrs, Volume= 0.201 af
Primary = 0.32 cfs @ 16.81 hrs, Volume= 0.201 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Post-Dev.: Post-Dev



23.0023 Post-Development Proposed Parcels Including MSE 24-hr 3 100 yr Rainfall=6.22"

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Page 22

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Offsite from South Runoff Area=3.000 ac 30.00% Impervious Runoff Depth>3.03"
Flow Length=595' Tc=12.9 min CN=72 Runoff=13.17 cfs 0.758 af

Subcatchment 2S: Onsite Runoff Area=3.940 ac 29.70% Impervious Runoff Depth>3.03"
Flow Length=293' Tc=11.8 min CN=72 Runoff=17.91 cfs 0.995 af

Reach 1R: Proposed Crossing Culvert Avg. Flow Depth=1.00' Max Vel=10.09 fps Inflow=13.17 cfs 0.758 af
12.0" Round Pipe n=0.011 L=34.0' S=0.0294 ' Capacity=7.22 cfs Outflow=7.22 cfs 0.758 af

Pond 1P: Wet Pond Peak Elev=907.08' Storage=1.171 af Inflow=25.13 cfs 1.753 af
Primary=2.80 cfs 0.690 af Secondary=0.00 cfs 0.000 af Outflow=2.80 cfs 0.690 af

Link 1L: (new Link) Inflow=25.13 cfs 1.753 af
Primary=25.13 cfs 1.753 af

Link Post-Dev.: Post-Dev Inflow=2.80 cfs 0.690 af
Primary=2.80 cfs 0.690 af

Total Runoff Area = 6.940 ac Runoff Volume = 1.753 af Average Runoff Depth = 3.03"
70.17% Pervious = 4.870 ac 29.83% Impervious = 2.070 ac

Summary for Subcatchment 1S: Offsite from South

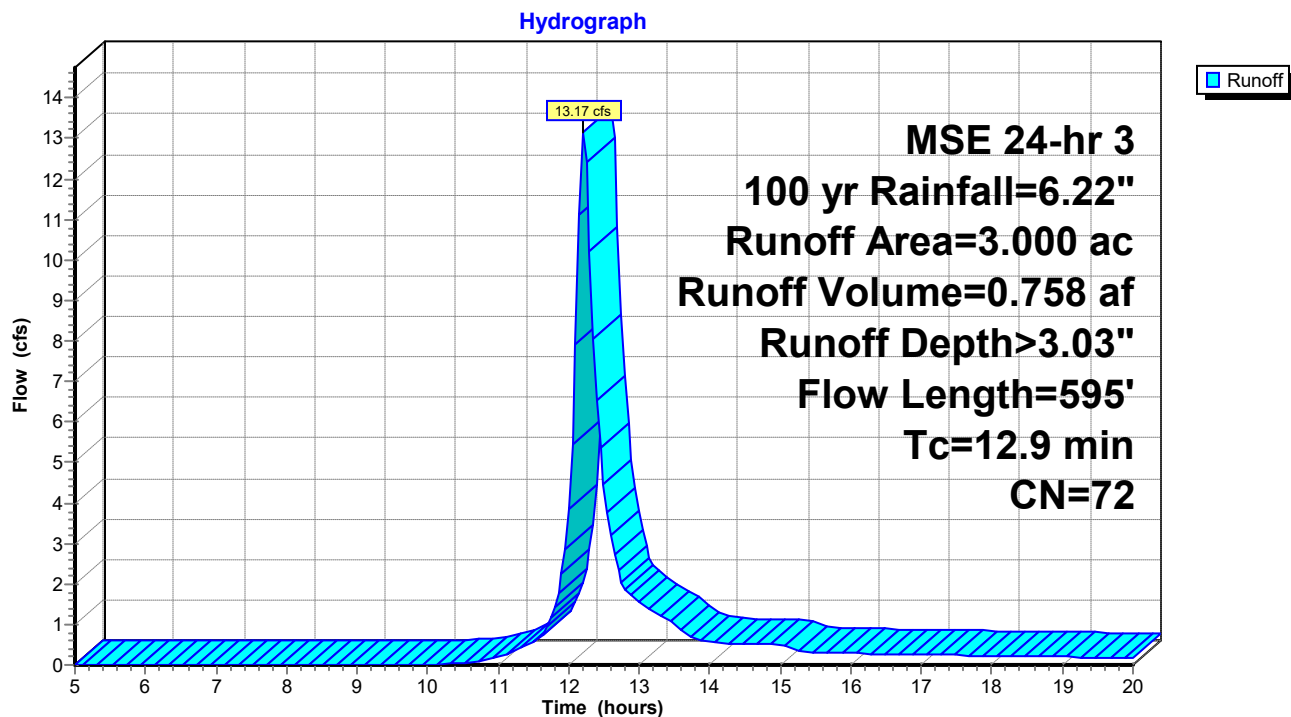
Runoff = 13.17 cfs @ 12.21 hrs, Volume= 0.758 af, Depth> 3.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 MSE 24-hr 3 100 yr Rainfall=6.22"

Area (ac)	CN	Description
3.000	72	1/3 acre lots, 30% imp, HSG B
2.100		70.00% Pervious Area
0.900		30.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.2	100	0.0700	0.18		Sheet Flow, Sheet Flow-Grass Grass: Dense n= 0.240 P2= 2.80"
1.8	185	0.0108	1.67		Shallow Concentrated Flow, Sh. Conc. Flow-Grass Unpaved Kv= 16.1 fps
1.4	250	0.0360	3.05		Shallow Concentrated Flow, Sh. Conc. Flow-Trees Unpaved Kv= 16.1 fps
0.5	60	0.0167	2.08		Shallow Concentrated Flow, Sh. Conc. Flow-Grass Unpaved Kv= 16.1 fps
12.9	595	Total			

Subcatchment 1S: Offsite from South



Summary for Subcatchment 2S: Onsite

Runoff = 17.91 cfs @ 12.20 hrs, Volume= 0.995 af, Depth> 3.03"

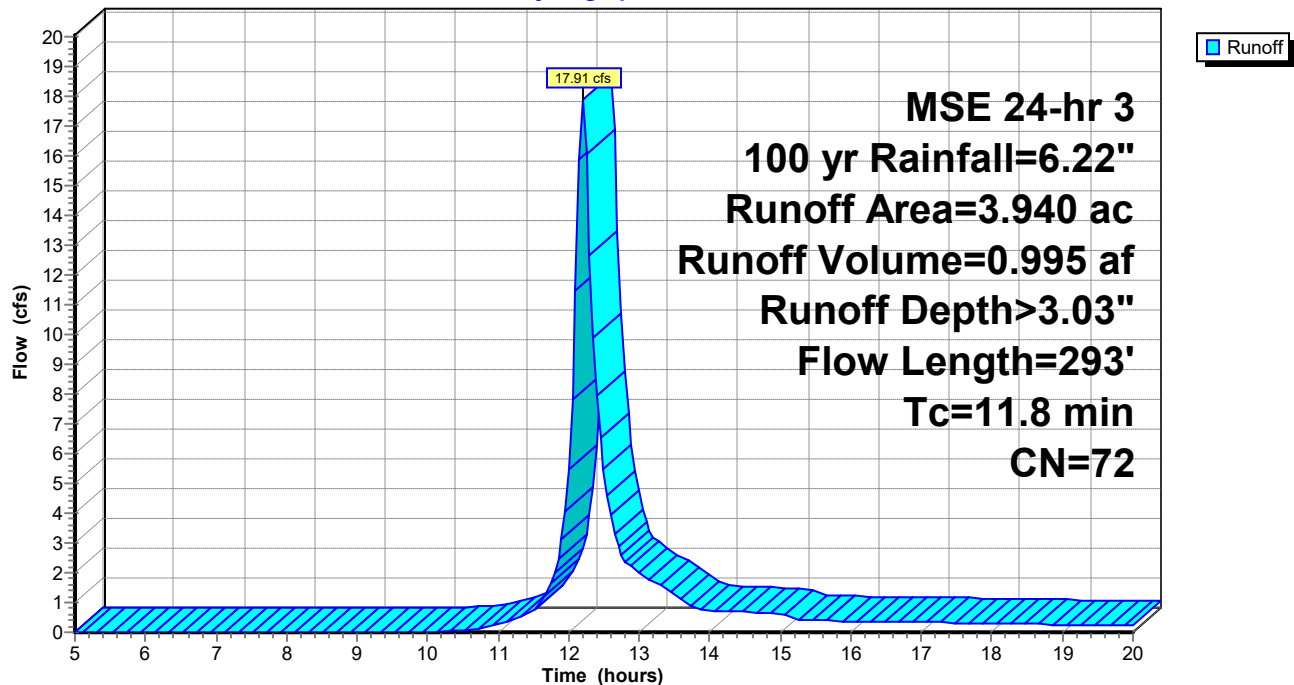
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 MSE 24-hr 3 100 yr Rainfall=6.22"

Area (ac)	CN	Description
0.350	98	Roofs, HSG B
* 0.330	98	Driveways, HSG B
0.490	98	Water Surface, HSG B
2.770	61	>75% Grass cover, Good, HSG B
3.940	72	Weighted Average
2.770		70.30% Pervious Area
1.170		29.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.3	16	0.0300	1.05		Sheet Flow, Sheet Flow Pavement Smooth surfaces n= 0.011 P2= 2.80"
11.3	84	0.0300	0.12		Sheet Flow, Sheet Flow-Grass Grass: Dense n= 0.240 P2= 2.80"
0.2	193	0.0850	14.44	288.83	Channel Flow, Channel-Grass Swale Area= 20.0 sf Perim= 20.0' r= 1.00' n= 0.030 Short grass
11.8	293	Total			

Subcatchment 2S: Onsite

Hydrograph



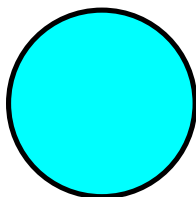
Summary for Reach 1R: Proposed Crossing Culvert

Inflow Area = 3.000 ac, 30.00% Impervious, Inflow Depth > 3.03" for 100 yr event
 Inflow = 13.17 cfs @ 12.21 hrs, Volume= 0.758 af
 Outflow = 7.22 cfs @ 12.15 hrs, Volume= 0.758 af, Atten= 45%, Lag= 0.0 min

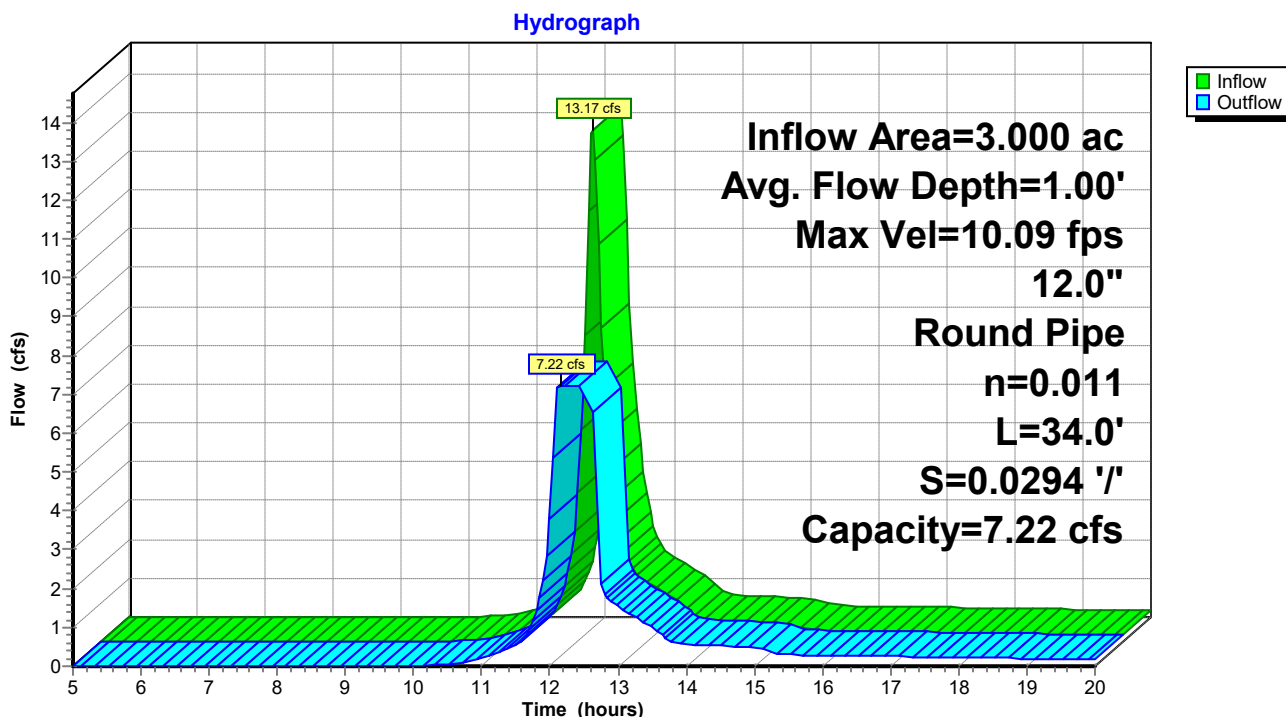
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Max. Velocity= 10.09 fps, Min. Travel Time= 0.1 min
 Avg. Velocity= 4.94 fps, Avg. Travel Time= 0.1 min

Peak Storage= 27 cf @ 12.10 hrs
 Average Depth at Peak Storage= 1.00'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.22 cfs

12.0" Round Pipe
 n= 0.011 Concrete pipe, straight & clean
 Length= 34.0' Slope= 0.0294 '/
 Inlet Invert= 925.00', Outlet Invert= 924.00'



Reach 1R: Proposed Crossing Culvert



Summary for Pond 1P: Wet Pond

Inflow Area = 6.940 ac, 29.83% Impervious, Inflow Depth > 3.03" for 100 yr event
 Inflow = 25.13 cfs @ 12.20 hrs, Volume= 1.753 af
 Outflow = 2.80 cfs @ 13.30 hrs, Volume= 0.690 af, Atten= 89%, Lag= 66.1 min
 Primary = 2.80 cfs @ 13.30 hrs, Volume= 0.690 af
 Secondary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 907.08' @ 13.30 hrs Surf.Area= 0.636 ac Storage= 1.171 af

Plug-Flow detention time= 183.4 min calculated for 0.687 af (39% of inflow)
 Center-of-Mass det. time= 115.4 min (905.8 - 790.4)

Volume	Invert	Avail.Storage	Storage Description
#1	905.00'	1.785 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
905.00	0.490	0.000	0.000
906.00	0.560	0.525	0.525
907.00	0.630	0.595	1.120
908.00	0.700	0.665	1.785

Device	Routing	Invert	Outlet Devices
#1	Primary	905.00'	12.0" Round RCP_Round 12" L= 22.0' RCP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 905.00' / 904.00' S= 0.0455 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf
#2	Device 1	905.00'	3.5" Vert. Orifice/Grate C= 0.600
#3	Device 1	906.90'	36.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Secondary	907.50'	10.0' long x 7.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.40 2.52 2.70 2.68 2.68 2.67 2.66 2.65 2.65 2.65 2.66 2.65 2.66 2.68 2.70 2.73 2.78

Primary OutFlow Max=2.80 cfs @ 13.30 hrs HW=907.08' (Free Discharge)

↑ **1=RCP_Round 12"** (Passes 2.80 cfs of 4.19 cfs potential flow)

↑ **2=Orifice/Grate** (Orifice Controls 0.45 cfs @ 6.70 fps)

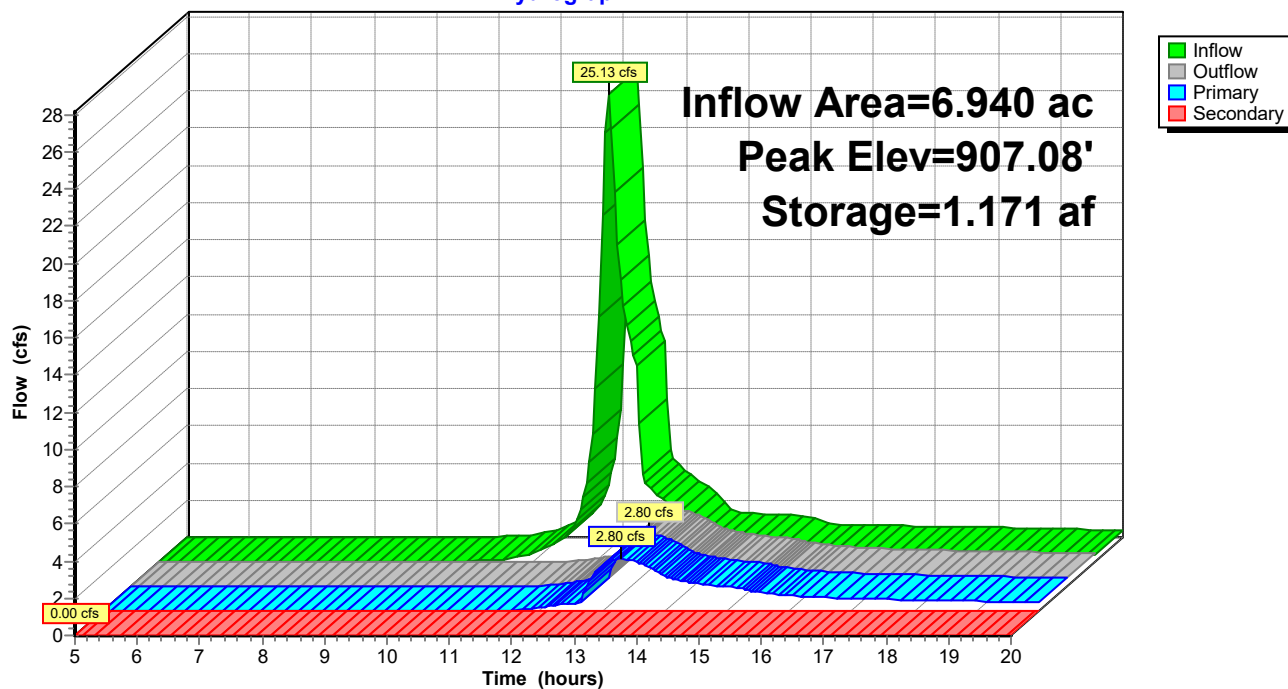
↑ **3=Orifice/Grate** (Weir Controls 2.35 cfs @ 1.39 fps)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=905.00' (Free Discharge)

↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond 1P: Wet Pond

Hydrograph



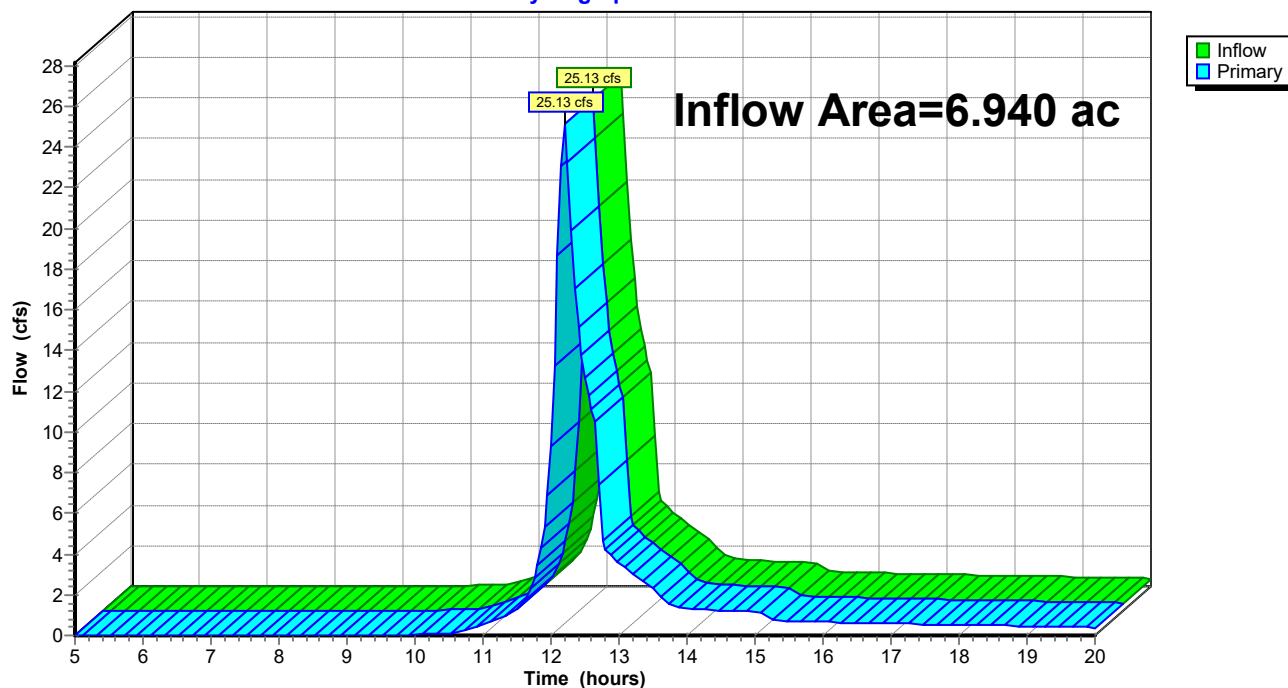
Summary for Link 1L: (new Link)

Inflow Area = 6.940 ac, 29.83% Impervious, Inflow Depth > 3.03" for 100 yr event
 Inflow = 25.13 cfs @ 12.20 hrs, Volume= 1.753 af
 Primary = 25.13 cfs @ 12.20 hrs, Volume= 1.753 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link 1L: (new Link)

Hydrograph

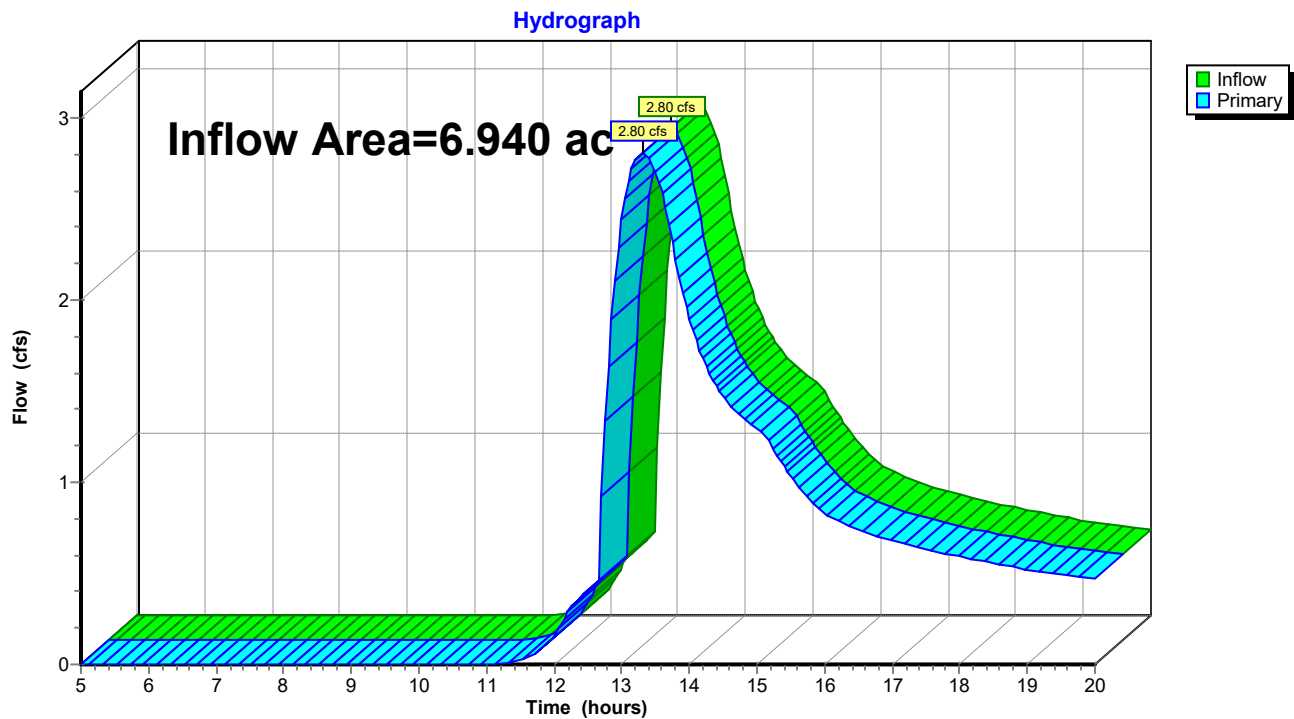


Summary for Link Post-Dev.: Post-Dev

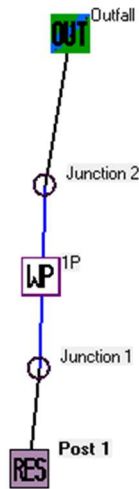
Inflow Area = 6.940 ac, 29.83% Impervious, Inflow Depth > 1.19" for 100 yr event
Inflow = 2.80 cfs @ 13.30 hrs, Volume= 0.690 af
Primary = 2.80 cfs @ 13.30 hrs, Volume= 0.690 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link Post-Dev.: Post-Dev



Appendix 4 POST DEVELOPMENT CONDITIONS (WATER QUALITY)



Canyon Development-Williams Bay-WinSLAMM Water Quality Calculations-Project Parcels

Data file name: P:\Shared\1 - Projects\2023\23.0023 - Canyon Development - WB Residential - 6 Parcels - WHA-00001 - 00006\Hydrology\Slamm\5-5-25 Final Stormwater Management\5-5-25 Proposed Parcels 5-5-25.mdb

WinSLAMM Version 10.5.0

Rain file name: C:\WinSLAMM Files\Rain Files\WisReg - Madison WI 1981.RAN

Particulate Solids Concentration file name: C:\WinSLAMM Files\v10.1 WI_AVG01.pscx

Runoff Coefficient file name: C:\WinSLAMM Files\WI_SL06 Dec06.rsvx

Residential Street Delivery file name: C:\WinSLAMM Files\WI_Res and Other Urban Dec06.std

Institutional Street Delivery file name: C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std

Commercial Street Delivery file name: C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std

Industrial Street Delivery file name: C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std

Other Urban Street Delivery file name: C:\WinSLAMM Files\WI_Res and Other Urban Dec06.std

Freeway Street Delivery file name: C:\WinSLAMM Files\Freeway Dec06.std

Apply Street Delivery Files to Adjust the After Event Load Street Dirt Mass Balance: False

Pollutant Relative Concentration file name: C:\WinSLAMM Files\WI_GEO03.ppdx

Source Area PSD and Peak to Average Flow Ratio File: C:\WinSLAMM Files\NURP Source Area PSD Files.csv

Cost Data file name:

Seed for random number generator: -42

Study period starting date: 01/01/81 Study period ending date: 12/31/81

Start of Winter Season: 12/02 End of Winter Season: 03/12

Date: 04-17-2025 Time: 13:07:10

Site information:

Canyon Development-Williams Bay, Wisconsin

LU# 1 - Residential: 2S Onsite Total area (ac): 3.940

1 - Roofs 1: 0.350 ac. Pitched Disconnected Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz

25 - Driveways 1: 0.330 ac. Disconnected Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz

45 - Large Landscaped Areas 1: 2.770 ac. Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz

70 - Water Body Areas: 0.490 ac. Source Area PSD File:

Control Practice 1: Wet Detention Pond CP# 1 (DS) - 1P

Particle Size Distribution file name: Not needed - calculated by program

Initial stage elevation (ft): 3

Peak to Average Flow Ratio: 3.8

Maximum flow allowed into pond (cfs): No maximum value entered

Outlet Characteristics:

Outlet type: Orifice 1

1. Orifice diameter (ft): 0.29
2. Number of orifices: 1
3. Invert elevation above datum (ft): 3

Outlet type: Broad Crested Weir

1. Weir crest length (ft): 10
2. Weir crest width (ft): 7
3. Height from datum to bottom of weir opening: 5.5

Outlet type: Vertical Stand Pipe

1. Stand pipe diameter (ft): 3
2. Stand pipe height above datum (ft): 4.9

Pond stage and surface area

Entry Number	Stage (ft)	Pond Area (acres)	Natural Seepage (in/hr)	Other Outflow (cfs)
0	0.00	0.0000	0.00	0.00
1	0.01	0.1500	0.00	0.00
2	1.00	0.2100	0.00	0.00
3	2.00	0.2700	0.00	0.00
4	3.00	0.4900	0.00	0.00
5	4.00	0.5600	0.00	0.00
6	5.00	0.6300	0.00	0.00
7	6.00	0.7000	0.00	0.00

File Name:

P:\Shared\1 - Projects\2023\23.0023 - Canyon Development - WB Residential - 6 Parcels - WHA-00001 - 00006\Hydrology\Slamm\5-5-25 Final Stormwater Management\5-5-25 Proposed Parcels 5-5-25.mdb

Outfall Output Summary

	Runoff Volume (cu. ft.)	Percent Runoff Reduction	Runoff Coefficient (Rv)	Particulate Solids Conc. (mg/L)	Particulate Solids Yield (lbs)	Percent Particulate Solids Reduction
Total of All Land Uses without Controls	72900		0.16	59.63	271.4	
Outfall Total with Controls	73030	-0.18 %	0.16	2.291	10.44	96.15 %
Current File Output: Annualized Total After Outfall Controls	73230		Years in Model Run: 1.00		10.47	

Pollutant	Concen- tration - No Controls	Concen- tration - With Controls	Concen- tration Units	Pollutant Yield - No Controls	Pollutant Yield - With Controls	Pollutant Yield Units	Percent Yield Reduction
Particulate Solids	59.63	2.291	mg/L	271.4	10.44	lbs	96.15 %
Total Solids	91.47	33.97	mg/L	416.3	154.9	lbs	62.79 %
Total Phosphorus	0.4087	0.1593	mg/L	1.860	0.7264	lbs	60.95 %

Print Output Summary to .csv File

Print Output Summary to Text File

Print Output Summary to Printer

Total Area Modeled (ac)

3.940

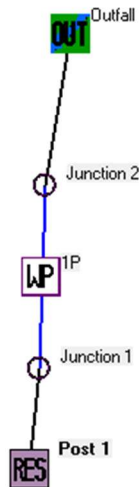
Total Control Practice Costs

Capital Cost	N/A
Land Cost	N/A
Annual Maintenance Cost	N/A
Present Value of All Costs	N/A
Annualized Value of All Costs	N/A

Perform Outfall
Flow Duration
Curve Calculations

Receiving Water Impacts Due To Stormwater Runoff (CWP Impervious Cover Model)

	Calculated Rv	Approximate Urban Stream Classification
Without Controls	0.16	Fair
With Controls	0.16	Fair



Canyon Development-Williams Bay-WinSLAMM Water Quality Calculations-Project Parcels including Offsite

Data file name: P:\Shared\1 - Projects\2023\23.0023 - Canyon Development - WB Residential - 6 Parcels - WHA-00001 - 00006\Hydrology\Slamm\5-5-25 Final Stormwater Management\5-5-25 Proposed Parcels including Offsite 5-5-25.mdb

WinSLAMM Version 10.5.0

Rain file name: C:\WinSLAMM Files\Rain Files\WisReg - Madison WI 1981.RAN

Particulate Solids Concentration file name: C:\WinSLAMM Files\v10.1 WI_AVG01.pscx

Runoff Coefficient file name: C:\WinSLAMM Files\WI_SL06 Dec06.rsvx

Residential Street Delivery file name: C:\WinSLAMM Files\WI_Res and Other Urban Dec06.std

Institutional Street Delivery file name: C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std

Commercial Street Delivery file name: C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std

Industrial Street Delivery file name: C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std

Other Urban Street Delivery file name: C:\WinSLAMM Files\WI_Res and Other Urban Dec06.std

Freeway Street Delivery file name: C:\WinSLAMM Files\Freeway Dec06.std

Apply Street Delivery Files to Adjust the After Event Load Street Dirt Mass Balance: False

Pollutant Relative Concentration file name: C:\WinSLAMM Files\WI_GEO03.ppdx

Source Area PSD and Peak to Average Flow Ratio File: C:\WinSLAMM Files\NURP Source Area PSD Files.csv

Cost Data file name:

Seed for random number generator: -42

Study period starting date: 01/01/81 Study period ending date: 12/31/81

Start of Winter Season: 12/02 End of Winter Season: 03/12

Date: 04-17-2025 Time: 13:07:45

Site information:

Canyon Development-Williams Bay, Wisconsin

LU# 1 - Residential: 2S Onsite Total area (ac): 6.940

1 - Roofs 1: 0.800 ac. Pitched Disconnected Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz

25 - Driveways 1: 0.780 ac. Disconnected Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz

45 - Large Landscaped Areas 1: 4.870 ac. Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz

70 - Water Body Areas: 0.490 ac. Source Area PSD File:

Control Practice 1: Wet Detention Pond CP# 1 (DS) - 1P

Particle Size Distribution file name: Not needed - calculated by program

Initial stage elevation (ft): 3

Peak to Average Flow Ratio: 3.8

Maximum flow allowed into pond (cfs): No maximum value entered

Outlet Characteristics:

Outlet type: Orifice 1

1. Orifice diameter (ft): 0.29
2. Number of orifices: 1
3. Invert elevation above datum (ft): 3

Outlet type: Broad Crested Weir

1. Weir crest length (ft): 10
2. Weir crest width (ft): 7
3. Height from datum to bottom of weir opening: 5.5

Outlet type: Vertical Stand Pipe

1. Stand pipe diameter (ft): 3
2. Stand pipe height above datum (ft): 4.9

Pond stage and surface area

Entry Number	Stage (ft)	Pond Area (acres)	Natural Seepage (in/hr)	Other Outflow (cfs)
0	0.00	0.0000	0.00	0.00
1	0.01	0.1500	0.00	0.00
2	1.00	0.2100	0.00	0.00
3	2.00	0.2700	0.00	0.00
4	3.00	0.4900	0.00	0.00
5	4.00	0.5600	0.00	0.00
6	5.00	0.6300	0.00	0.00
7	6.00	0.7000	0.00	0.00

File Name:

P:\Shared\1 - Projects\2023\23.0023 - Canyon Development - WB Residential - 6 Parcels - WHA-00001 - 00006\Hydrology\Slamm\5-5-25 Final Stormwater Management\5-5-25 Proposed Parcels including Offsite 5-5-25.mdb

Outfall Output Summary

	Runoff Volume (cu. ft.)	Percent Runoff Reduction	Runoff Coefficient (Rv)	Particulate Solids Conc. (mg/L)	Particulate Solids Yield (lbs)	Percent Particulate Solids Reduction
Total of All Land Uses without Controls	91731		0.11	85.89	491.9	
Outfall Total with Controls	91887	-0.17 %	0.11	4.387	25.17	94.88 %
Current File Output: Annualized Total After Outfall Controls	92139		Years in Model Run: 1.00		25.23	

Pollutant	Concen- tration - No Controls	Concen- tration - With Controls	Concen- tration Units	Pollutant Yield - No Controls	Pollutant Yield - With Controls	Pollutant Yield Units	Percent Yield Reduction
Particulate Solids	85.89	4.387	mg/L	491.9	25.17	lbs	94.88 %
Total Solids	132.0	50.15	mg/L	755.7	287.7	lbs	61.93 %
Total Phosphorus	0.5798	0.2293	mg/L	3.320	1.315	lbs	60.39 %

Print Output Summary to .csv File

Print Output Summary to Text File

Print Output Summary to Printer

Total Area Modeled (ac)

6.940

Total Control Practice Costs

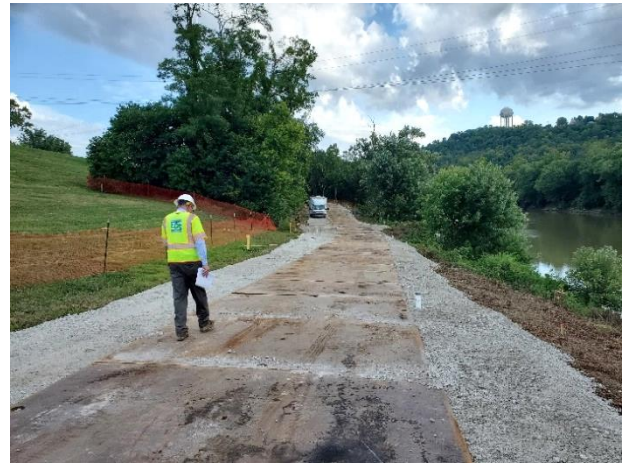
Capital Cost	N/A
Land Cost	N/A
Annual Maintenance Cost	N/A
Present Value of All Costs	N/A
Annualized Value of All Costs	N/A

Perform Outfall
Flow Duration
Curve Calculations

Receiving Water Impacts Due To Stormwater Runoff (CWP Impervious Cover Model)

	Calculated Rv	Approximate Urban Stream Classification
Without Controls	0.11	Good
With Controls	0.11	Good

Appendix 5 GEOTECHNICAL REPORT



ECS Midwest, LLC

Geotechnical Engineering Report Geneva Street Infiltration

Geneva Street
Williams Bay, Wisconsin

ECS Project Number 42:2648

October 28, 2024





ECS MIDWEST, LLC

Geotechnical • Construction Materials • Environmental • Facilities

October 28, 2024

Ms. Jennifer Wiese
Treasurer
Village of Williams Bay
PO BOX 580
Williams Bay, WI 53191

ECS Project No. 42:2648

Reference: Geotechnical Engineering Report
Geneva Street Infiltration
Geneva Street
Williams Bay, Wisconsin

Dear Ms. Wiese:

ECS Midwest, LLC (ECS) has completed the subsurface exploration, field and laboratory testing, and geotechnical engineering analyses for the above-referenced project. Our services were performed in general accordance with our agreed scope of services. This report presents our understanding of the geotechnical aspects of the project, the results of the field exploration and laboratory testing conducted, and our design and construction recommendations.

It has been our pleasure to be of service to you on this project. ECS would like to continue its services during design and provide our services during construction to check the assumptions of subsurface conditions made for this report. Please contact us should you have questions about the information contained in this report, or if ECS can be of further assistance to you.

Respectfully submitted,

ECS Midwest, LLC

Lauren McGovern
Geotechnical Staff Project Manager
LMcGovern@ecslimited.com



Darin Maciolek, P.E.
Principal, Geotechnical Dept. Manager
DMaciolek@ecslimited.com

TABLE OF CONTENTS

1.0	INTRODUCTION	2
2.0	PROJECT INFORMATION.....	3
2.1	Project Location/Current Site Use/Past Site Use.....	3
2.2	Proposed Construction	4
3.0	FIELD EXPLORATION AND LABORATORY TESTING	4
3.1	Field Exploration	4
3.2	Subsurface Characterization	4
3.3	Groundwater Observations	5
3.4	Laboratory Testing	5
4.0	DESIGN RECOMMENDATIONS	6
4.1	Infiltration Basin Design Considerations.....	6
4.2	Wet Detention Pond Design Considerations	7
5.0	SITE CONSTRUCTION CONSIDERATIONS.....	9
5.1	Subgrade Preparation	9
5.1.1	Stripping and Grubbing.....	9
5.1.2	Site Temporary Dewatering.....	9
5.2	Earthwork Operations.....	11
5.2.1	Engineered Fill	11
5.2.2	Compaction.....	12
5.3	General Considerations	13
6.0	CLOSING	13

APPENDICES

Appendix A – Drawings & Reports

- Site Location Diagram
- Boring Location Diagram
- Soil Survey Map

Appendix B – Field Operations

- Reference Notes for Boring Logs
- Subsurface Exploration Procedures: SPT
- Boring Logs

Appendix C – Laboratory Testing

- Laboratory Testing Procedures: Index Testing
- USDA Classification System
- Plasticity Chart(s)
- Particle Size Distribution

Appendix D – Supplemental Report Documents

- Soil Evaluation – Storm Form

Appendix E – Other Information

- Important Information about This Geotechnical-Engineering Report

EXECUTIVE SUMMARY

This Executive Summary is intended as a very brief overview of the primary geotechnical conditions expected to affect design and construction. Information gleaned from this Executive Summary should not be utilized in lieu of reading the entire geotechnical report.

- The main site development challenges are expected to include moisture control of the site soils.
- Because of density of the woods, which included downed trees, two of the planned boring locations could not be accessed.
- The site soils encountered at the borings located within the proposed stormwater management (SWM) area are predominately clayey and are anticipated to have low infiltration rates. Based on the soil types encountered, exemption from infiltration is expected.

1.0 INTRODUCTION

The purpose of this study was to provide geotechnical information for the design of stormwater management basin. The recommendations developed for this report are based on project information supplied by Baxter & Woodman.

Our services were provided in accordance with ECS Proposal No. 42:3888-GP, dated August 7, 2024, as authorized by Mr. William Duncan of the Village of Williams Bay on September 17, 2024, which includes the Terms and Conditions of Service included therein.

This report contains the procedures and results of the subsurface exploration and laboratory testing programs, a description of existing site conditions, engineering analyses, and geotechnical recommendations for the design and construction.

The report includes the following items:

- A brief description of the field and laboratory test procedures and results.
- A description of the observed surface topographical features and site conditions.
- A description of area and site geologic conditions.
- A description of the interpreted subsurface soil stratigraphy with pertinent available physical properties.
- Records of the subsurface exploration(s) (test boring log).
- Evaluation and recommendations for wet detention pond design in accordance with the Wisconsin Department of Natural Resources Conservation Practice Standard 1001 and NRCS.
- Evaluation of infiltration in accordance with the Wisconsin Department of Natural Resources Conservation Practice Standard 1002 and NRCS.
- Recommendations for site preparation and construction of engineered fills, including an evaluation of on-site soils for use as engineered fills, and delineation of potentially unsuitable soils and/or soils exhibiting excessive moisture at the time of sampling.
- General considerations relative to groundwater.

The scope of services for this project did not include an environmental assessment for determining the presence or absence of wetlands, or corrosive, hazardous or toxic materials in the soil, bedrock, surface water, groundwater, or air on or below, or around this site. Statements in this report or on the boring logs regarding odors, colors, and unusual or suspicious items or conditions are strictly for informational purposes.

2.0 PROJECT INFORMATION

2.1 PROJECT LOCATION/CURRENT SITE USE/PAST SITE USE

The project site is located at the north side of East Geneva Street in Williams Bay, Walworth County, Wisconsin. The subject property is bounded to the north, east and west by residential developments, and to the south by Geneva Street. The site location is shown in Figure 2.1.1 below, and on the Site Location Diagram in Appendix A.



Figure 2.1.1 Site Location (approximate site outlined in red)

The site is currently undeveloped, wooded land. At the time this report was written, a site-specific topographic survey was not available. The ground surface elevations, estimated from the topographic information on the Walworth County, WI – One View website, vary from approximately EL. 898 to EL. 930 feet.

2.2 PROPOSED CONSTRUCTION

We understand the proposed project will include the construction of a stormwater management area. The information in the table below summarizes our understanding.

DESIGN INFORMATION			
Subject		Expectation	
SWM Device	Type	Infiltration	B
	Bottom Depth/Elevation	4 to 8 feet below site grade	B
Grading	Cut	Less than 8 feet	B
	Fill	Less than 2 feet	B

R: Reported by client and/or design team

B: Based on ECS estimates in absence of information from client and/or design team

If ECS' understanding of the project is not correct or the design changes, especially if elevations are different, please contact ECS so that we may review these changes and revise our recommendations, as appropriate.

3.0 FIELD EXPLORATION AND LABORATORY TESTING

3.1 FIELD EXPLORATION

The exploration procedures are described in greater detail in Appendix B in the insert titled Subsurface Exploration Procedures: SPT.

ECS used a handheld GPS unit to field locate the borings prior to mobilization of the drilling equipment. The approximate boring location is shown on the Boring Location Diagram in Appendix A. Ground surface elevations at the boring locations were estimated from the topographic information on the Walworth County, WI – One View website. Ground surface elevations and boring locations determined without professional survey are approximate and may not be appropriate for final design and construction.

Our scope of services included three borings. However, due to the density of trees and presence of fallen trees only one of the planned borings was drilled at the site on October 9, 2024, by a subcontracted driller. The boring was advanced to an approximate depth of 20 feet. The drill crew utilized a 7822DT Geoprobe track-mounted rotary drilling rig equipped with continuous flight, hollow stem augers.

The drill crew backfilled the boreholes at completion of drilling. Settlement or expansion of borehole backfill can occur over time resulting in a trip hazard. Monitoring the boreholes after initial drilling activities is not within our scope but should be done by the client or property owner.

3.2 SUBSURFACE CHARACTERIZATION

The observed subsurface conditions were generally consistent with published geological mapping. Information regarding the site geology (Soil Survey Mapping) is included in Appendix A. Below is a generalized characterization of the subsurface materials observed at the boring locations. Please refer to the Boring Logs in Appendix B.

GENERALIZED SUBSURFACE STRATIGRAPHY					
Approx. Depth to Stratum Bottom (feet)	Strata	Description	SPT ⁽¹⁾ N-value Range (bpf)	Calibrated Penetrometer Resistance (tsf)	Moisture Content (%)
Surface	--	Topsoil (6 inches ±)	--	--	--
6	I	(CL) LEAN CLAY, stiff to hard	7 - 17	1.5 - 4.5	12 - 18
12	II	(GC) CLAYEY GRAVEL WITH SAND, very stiff to hard	12	2.5 - 4.5	8 - 9
20	III	(CL) SANDY LEAN CLAY WITH GRAVEL, stiff to very stiff	8 - 11	1.25 - 3	10 - 15

1. Standard Penetration Test.

The soil stratification shown on the boring logs represents the interpreted soil conditions at the boring locations. Variations in the stratification can occur between sample intervals and between boring locations. The subsurface conditions at other times and locations on the site may differ from those found at the boring locations. If different site conditions are encountered during construction, ECS should be contacted to review our recommendations relative to the new information.

Because the split-spoon sampler has a 1⅜-inch inside diameter, the soil classifications noted on the boring logs may not be representative of the entire soil matrix. Material larger than the 1⅜-inch inside diameter of the split-spoon sampler cannot be collected and observed directly.

3.3 GROUNDWATER OBSERVATIONS

The drillers observed the boreholes for the presence of measurable free water during drilling and at the completion of drilling. The approximate depths and elevations of measurable water observed at the boring locations are listed on the boring logs in Appendix B.

Because mostly poor draining soils were encountered at the boring locations, the static groundwater level may take days or weeks to stabilize. Therefore, the lack of observed water may not provide a reasonable indication of the static groundwater conditions at the time of drilling.

Based upon our interpretation of the subsurface data, ECS estimates the static groundwater level to be at a depth deeper than the explored depth of approximately 20 feet below the site surface at the boring location.

Variations in the long-term water table elevation may result because of changes in precipitation, evaporation, surface water runoff, construction activities, and other factors. Perched water conditions may also develop and/or exist at shallower or variable depths seasonally, particularly within more permeable soil underlain by less permeable soil.

3.4 LABORATORY TESTING

The laboratory services performed by ECS for this project included classification and index property tests on select representative soil samples.

Each soil sample was visually classified based on texture and plasticity using ASTM D2488, Standard Practice for Description and Identification of Soils (Visual-Manual Procedures) and ASTM D2487 Standard Practice for Classification for Engineering Purposes (Unified Soil Classification System (USCS)) as a general guideline. After classification, the samples were grouped into the major zones noted on the boring logs in Appendix B. The USCS group symbols for each soil type are indicated in parentheses along

with the soil descriptions. The stratification lines between strata on the boring logs are approximate; in-situ, the transitions may be gradual.

The soils were also classified using the U.S. Department of Agriculture (USDA) Soil Classification System. The USDA classification system is briefly described in Appendix C. The USDA classifications can be found on the "Soil and Site Evaluation – Storm" form included in Appendix D of this report.

The results of the laboratory tests are included on the boring logs in Appendix B or separate test sheets in Appendix C. These tests included:

- Moisture content (ASTM D2216)
- Calibrated Penetrometer Resistance
- Atterberg Limits (ASTM D4318)
- Particle Size Distribution (ASTM D6913)
- Organic content by Loss on Ignition (LOI) (ASTM D2974)

The laboratory procedures are described in greater detail in Appendix C including the insert titled Laboratory Procedures.

The soil samples will be retained in our laboratory for a period of 60 days, after which, the samples will be discarded, unless other instructions are received as to their disposal.

4.0 DESIGN RECOMMENDATIONS

The following sections contain geotechnical design recommendations for stormwater management devices.

4.1 INFILTRATION BASIN DESIGN CONSIDERATIONS

The recommendations presented in this section are limited to stormwater infiltration considerations, which follows the general guidelines of WDNR Conservation Practice Standard 1002, Site Evaluation for Stormwater Infiltration. The recommendations in this section are based on the soil and groundwater conditions observed at the boring.

Specific details of the proposed infiltration device were not available at the time of this report preparation; it is recommended ECS be provided the plans, when available, to check that the recommendations provided herein are applicable. ECS should be retained to provide observation and testing during construction.

Our scope of services is not inclusive of each step involved in the initial site screening (Part A) of the WDNR Technical Standard 1002. Therefore, other conditions may exist at, or near the site that could exclude or exempt the site, or portions of the site from the infiltration requirements.

Infiltration Rates: Based on the soil types encountered, exemption from infiltration is expected. Estimated soil infiltration rates based on the textural classification of the soils encountered and the guidelines provided in the WDNR Conservation Practice Standard 1002 ranged from 0.03 to 0.11 inches/hour and are listed on the Soil and Site Evaluation - Storm form included in Appendix D. Infiltration rates based on soil textural classification and the guidelines provided in Table 2 of the WDNR Conservation Practice Standard 1002 should be adjusted for the least permeable soil layer within 5 feet of each of the listed intervals. These infiltration rates are estimates only. If better estimates of infiltration are required, ECS recommends field double ring infiltrometer tests be conducted.

Estimation of the final design infiltration rate should consider the effects of engineered fill placed, surface vegetation, erosion control devices, and potential groundwater mounding. Prior to and during construction, the design infiltration rate of the soil at the SWM device bottom should be verified. Compaction of the basin bottom subgrade during and following construction should be prevented as this may reduce the infiltration rate of the soil. This may require exclusion of construction traffic from the infiltration bottom, or loosening of the subgrade soil, such as by raking or discing. Sediment allowed to accumulate at the basin bottom will reduce infiltration. Measures should be taken to reduce accumulation of sediment. Periodic removal of sediment should be expected.

Seasonal High Water Table: Intermittent wet and dry periods may lead to soil mottling. The NRCS considers soil matrix or mottle colors of chroma 2 or lower (gray colors) as a potential indication of horizons that are saturated for at least part of the year. The upper limits of these mottles have been considered in the estimate of the seasonal high water table. Based on the boring information, the seasonal high water level is estimated to be at a depth deeper than the explored depth of 20 feet. Estimation of the seasonal high water level based on colors (redoximorphic features) may not be reliable. Redox features can be difficult to distinguish in sandy soils. Also, we often don't know how often or long the soil is saturated at a given depth from color alone. A better estimate of the seasonal high groundwater table would require installation of groundwater observation wells along with observing the wells over a long time period.

Bedrock: Bedrock is estimated to be deeper than the depth explored of 20 feet.

4.2 WET DETENTION POND DESIGN CONSIDERATIONS

The recommendations presented in this section are limited to a wet detention pond, which follows the general guidelines of Wisconsin Department of Natural Resources (WDNR) Conservation Practice Standard 1001, Wet Detention Ponds.

The predominant site clayey soils are estimated to have relatively low infiltration rates. The construction and use of a wet detention basin is considered to be feasible at this site provided any sands and silts are lined with clay.

Given the predominate clay soils at the site, scarification and recompaction of the basin sidewalls and bottom, to help disrupt fissures or more permeable thin soil seams is expected to be adequate. More gravelly layers, and permeable soil layers, seams or lenses where present are recommended to be undercut at least 2 feet and at least 2 feet of compacted clay liner placed to reduce lateral and/or vertical movement of water through the basin walls and bottom, particularly if a stable pool elevation is desired. Based on the boring information the lean clay excavated to construct the pond is expected to be adequate for use as a clay liner. The gravelly layer encountered between approximately 6 and 12 feet may have too much coarse material to meet liner material requirements. Testing of the soil to be used as liner material is recommended to check its adequacy for use as a clay liner. Moisture content adjustment of the soils prior to placement and compaction may be needed depending on the source. More moisture adjustment of the soils borrowed from the upper few feet than deeper is expected based on the borings. Scarification and recompaction of basin walls and bottom, and construction of the clay liner are recommended to be observed and tested by ECS.

Liner material is recommended to conform to WDNR Conservation Practice Standard 1001, Type A or Type B Liner requirements, as appropriate, which include the properties listed below.

RECOMMENDED CLAY LINER MATERIAL PROPERTIES		
Material Characteristic	Material Values	
	Type A Liner	Type B Liner
USCS Classification	CL or CH	CL or CH
Percentage Passing a No. 200 Sieve	50% minimum	50% minimum
In-Place Hydraulic Conductivity (cm/sec)	1×10^{-7} or less	1×10^{-6} or less
Liquid Limit (%)	25 or greater average, no value less than 20	16 or greater average, no value less than 14
Plasticity Index (%)	12 or greater average, no value less than 10	7 or greater average, no value less than 5
Organic Content per ASTM D2487	Less than 3% loss on ignition	Less than 3% loss on ignition

Clay liner material is recommended to be compacted at a moisture content 0 to 4 percent above the optimum moisture content and to a minimum of 95 percent of the maximum dry density determined by Standard Proctor (ASTM D698). Compact with a footed compactor weighing at least 25,000 pounds, operated continuously, in loose lift thicknesses not to exceed the smaller of 6 inches or the length of the teeth on the footed compactor used. The liner material must not contain deleterious materials. Materials considered deleterious include but are not limited to miscellaneous construction material, rubble fragments, wood fragments, material greater than 3 inches in nominal diameter, and hazardous materials.

A higher density than the minimum required by Natural Resources Conservation Practice Standard 1001 may be required depending on permeability requirements and should be determined based on laboratory testing of compacted samples of the liner material to be used. USDA-NRCS Wisconsin Construction Specification 300 Clay Liner gives additional information pertaining to selection and compaction of engineered fill for the clay liner. In addition to the USDA-NRCS Wisconsin Construction Specification 300 Clay Liner requirement that the water content of fill material be above the optimum moisture content, it is recommended the water content of fill material be no more than 4 percent above the optimum moisture content. The clay is recommended to be mechanically processed, such as with discing, prior to compaction to break up clods of clay to no more than half the loose lift thickness in size. The clay liner, once placed, should be protected from desiccation caused by drying of the soil. The formation of desiccation cracks will increase the risk of water loss through the clay liner.

As an alternative to a compacted clay liner, Appendix D of the WDNR Conservation Practice Standard 1001 outlines other acceptable liner materials, which include high density polyethylene (HDPE) and Geosynthetic Clay (GCL) liners.

If water control measures require a dewatering system, then it should be installed outside of the perimeter of the pond to avoid liner penetrations that could compromise the integrity of the liner system.

Remove water that accumulates in excavations and other construction areas, along with unstable soil as soon as possible.

Take measures to reduce seepage along conduits where inlet and outlet pipes are constructed. Typically accepted measures may include the use of granular filters, anti-seepage collars or bentonite.

Provide erosion protection to protect the pond side slopes and embankments. The placement of a granular riprap and/or establishment of vegetation can be considered for erosion control within the pond and surrounding areas. The interior slopes of the ponds should be constructed at an inclination of

3:1 (Horizontal: Vertical) or flatter to help reduce the potential for instability related problems. If steeper slopes are required, then slope stability analysis is recommended to be conducted.

5.0 SITE CONSTRUCTION CONSIDERATIONS

5.1 SUBGRADE PREPARATION

The method of site preparation will be influenced by factors unknown at the time this report was prepared, which may include weather before and during construction, the possibility of subsurface conditions not revealed by the boring(s), and the final project details.

5.1.1 Stripping and Grubbing

The subgrade preparation should include stripping pavement, vegetation, rootmat, topsoil, and other soft or otherwise unsuitable materials from the 10-foot expanded building limits, 5-foot expanded pavement limits, and 5 feet beyond the toe of engineered fills, where feasible. Topsoil deposits (approximately 6 inches thick) was observed at the boring location. Deeper topsoil or organic laden soils may be present, particularly in wet, low-lying, and poorly drained areas. In wooded areas, root masses may extend deeper and require additional localized excavation depth to completely remove the organics (root mass). The project team should be prepared to remove unsuitable material in its entirety within the limits noted above. Because topsoil depths can vary greatly across a site the contractor should verify topsoil depths as part of their bidding process. Note topsoil removal should not be based on soil coloration alone. After removal of the rootmat, it may be possible to leave some darker soils in place provided the soil contains no more than 5 percent organic matter, by dry weight, as determined by Loss on Ignition ASTM D2974, has the recommended strength characteristics and is stable under proofroll. ECS should be retained to observe and document that topsoil and other unsuitable surficial materials have been removed prior to the placement of engineered fill or construction of structures.

The subgrade should not be left exposed to the elements or construction traffic for a prolonged time as the subgrade may become disturbed and/or softened. If pavement or other protective coverings are not placed within a few days after exposing the final design subgrade, consideration should be given to leaving the subgrade approximately 1 foot above the final design subgrade, where feasible, to help reduce softening of the design subgrade soil.

5.1.2 Site Temporary Dewatering

The contractor shall make their own assessment of temporary dewatering needs based upon the limited subsurface groundwater information presented in this report. Soil and groundwater conditions may vary between sampling intervals. If the contractor believes additional subsurface information is needed to assess dewatering needs, the contractor should obtain such information at their own expense. ECS makes no warranties or guarantees regarding the adequacy of the provided information to determine dewatering requirements; such recommendations are beyond our scope of services.

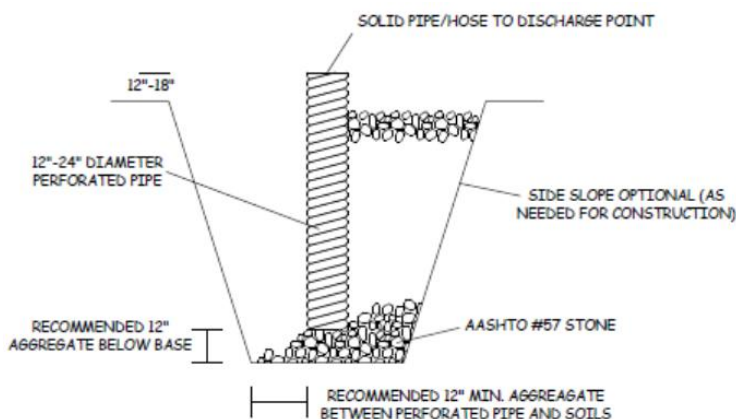
Dewatering systems are a critical component of many construction projects. Dewatering systems must be selected, designed, and maintained by a qualified and experienced (specialty or other) contractor familiar with the geotechnical and other aspects of the project. The failure to properly design and maintain a dewatering system for a given project can result in delayed construction, unnecessary undercuts, detrimental phenomena such as ‘running sand’ conditions, heaved subgrades, internal erosion (i.e., ‘piping’), the migration of ‘fines’ down-gradient towards the dewatering system, localized settlement of nearby infrastructure, foundations, slabs-on-grade and pavements, etc. Water discharged

from site dewatering systems are recommended to be discharged in accordance with local, state and federal requirements.

Surface Water: The surface of the site should be kept properly graded to enhance drainage of the surface water to appropriate discharge or storage areas during construction. Effort should be made to enhance the natural drainage without interrupting its pattern.

Subsurface Water: Groundwater observations are described in the **Groundwater Observations** section of this report. It appears the hydrostatic groundwater level at this site may be deeper than the explored depth of approximately 20 feet below grade. Based upon the results of the subsurface exploration and proposed construction, ECS' opinion is that construction dewatering at this site will be mainly to remove accumulated runoff water and perched water. Strategies for addressing groundwater are discussed below.

Strategies for Addressing Perched Groundwater: The typical strategy for addressing perched groundwater seepage into excavations and where excavations extend typically less than 1 to 2 feet below the water level, especially in areas containing primarily clay soils, is pumping from trench (or French) drains and sump pits with sump pumps which are backfilled with drainage aggregate such as AASHTO Size No. 57 Stone or open-graded bedding material. A typical sump pump drain (found in a sump pit or along a French drain) is depicted below. The inlet of the sump pump is placed at the bottom of the corrugated pipe and the discharge end of the sump is directed to an appropriate stormwater drain.



Sump Pit/Pump Conceptual Sketch

A typical French drain consists of an 18 to 24-inch wide by 18- to 24-inch-deep bed of AASHTO No. 57 aggregate (or similar open graded aggregate) wrapped in a medium duty, non-woven geotextile and (sometimes) containing a 6-inch diameter, Schedule 40 PVC perforated or slotted pipe. Actual dimensions should be determined during construction. After installation, the geotextile should be wrapped over the top of the aggregate and pipe followed by placement of backfill. The top of the drain should be positioned at least 3½ feet below the design subgrade elevation. Drains should not be routed within the expanded building limits.

Pumping wells or a vacuum system could also be used to address perched groundwater. These techniques often are only effective during the initial depletion of the perched water quantity and may quickly be ineffective at addressing accumulation of water from rain, snow, etc.

Install required sump pits and other dewatering systems outside the perimeter of the pond liner to reduce the potential for the formation of a detrimental preferential flow path through the liner.

5.2 EARTHWORK OPERATIONS

The following sections describe existing fill, soil reusability, and recommendations for engineered fill.

5.2.1 Engineered Fill

Product Submittals: Prior to placement of engineered fill, representative bulk samples (typically at least 50 to 100 pounds) of on-site and off-site borrow per material type should be submitted to ECS for laboratory testing, which may include natural moisture content, organic content, grain-size distribution, Atterberg limits, and moisture-density relationships for compaction. Import material should be tested prior to being hauled to the site to determine if it complies with project specifications. Alternatively, Proctor data from other accredited laboratories can be submitted if the test results are within the last 90 days.

Satisfactory Engineered Fill Materials: Engineered fills should consist of materials free of debris with the following engineering properties.

ENGINEERED FILL INDEX PROPERTIES		
Subject		Property
Plasticity	Upper 4 feet in Structure Areas and Upper 2 feet in Pavement Areas	LL ≤ 40, PI ≤ 15
	Below 4 feet in Structure Areas and Below 2 feet in Pavement Areas	LL ≤ 50, PI ≤ 20
Max. Particle Size		3 inches
Max. Organic Content (by dry weight)		5 percent

Open-graded materials, such as coarser sands, and gravels (SP and GP), which contain increased void space in their mass may need to be encapsulated within a filter geotextile. If the engineered fill is to provide low-frost susceptible characteristics, the engineered fill must be classified as a clean GP or GW (or clean coarser SW or SP) per Unified Soil Classification System (ASTM D-2487) and must be properly drained.

Unsatisfactory Materials: Unsatisfactory fill materials, which do not satisfy the requirements for suitable materials, include topsoil and organic materials (PT, OH, OL), silt (ML) within the frost zone, and high plasticity soils elastic silt (MH) and fat clay (CH).

Pea gravel is not recommended to be used as engineered fill. Pea gravel has round/smooth characteristics, no fines and does not interlock when compacted, which makes it more susceptible to future movement and instability resulting in excessive and variable settlement.

On-Site Borrow Suitability: Natural deposits of soils that meet the definition of Satisfactory Engineered Fill Materials are expected to be present at the site. Site soil used as engineered fill must not contain more than 5 percent organic matter as determined by ASTM D2974, and must be free of frozen matter, deleterious materials, over-sized material (maximum 3-inch particle diameter), or chemicals that may result in the material being classified as “contaminated.” The on-site soil may be feasible to use as engineered fill with proper moisture conditioning during favorable conditions. Site soil considered for use as engineered fill should be further evaluated by ECS prior to its use.

Depending on the conditions at the time of construction some drying of site soil prior to reuse as engineered fill may be needed. This may involve spreading and disking to aerate, chemical modification or other methods to reduce the soil moisture to within a narrow range of the soil's optimum moisture content to facilitate placement and compaction. Reuse of soils having elevated moisture content is often a point of contention and a source of delays and change orders. From a technical standpoint, soils with moisture contents wet of optimum can be reused provided the moisture is properly adjusted to within a workable range of the material optimum moisture content. From a practical standpoint, wet soils can be difficult to dry within small or congested areas or during unfavorable weather, and such difficulties should be considered during planning and budgeting. A clear understanding by the general contractor and grading subcontractor regarding the reuse of excavated soils will be important to reduce delays and unexpected cost overruns. It is important to establish as part of the contract whether soils having elevated moisture content will be considered suitable for reuse. Some conditions at the time of construction, such as wet or freezing weather, may preclude the use of on-site soil.

5.2.2 Compaction

Subgrade Benching: Fill material should be placed in horizontal lifts. Where fill materials will be placed to widen existing embankment fills, or placed up against sloping ground, the soil subgrade should be scarified, and the new fill benched and keyed into the existing material. Placement and compaction of fill is recommended to be on a 5 (H):1 (V) or flatter slope, or stepped or benched as required to flatten.

Engineered Fill Compaction: Engineered fill is recommended to be placed and compacted in appropriate thickness loose lifts as recommended below. Give as much importance to the moisture content requirements of the material as the density requirements during placement and compaction considering the moisture sensitivity of the soil.

ENGINEERED FILL COMPACTION RECOMMENDATIONS		
Subject		Recommendation
Compaction Standard		Standard Proctor, ASTM D698
Recommended Compaction	Structural Areas	≥ 98 percent of Maximum Dry Density
	Landscape/Greenspace Areas	≥ 90 percent of Maximum Dry Density
Moisture Content	Fine-grained	-1 to +3 % points of the material's optimum value
	Coarse-grained	-3 to +3 % points of the material's optimum value
Maximum Loose Thickness		8 inches prior to compaction ⁽¹⁾

1. The maximum loose lift thickness depends upon the type of compaction equipment used and material being compacted. Density testing during fill placement is important to check and document that the specified compaction is being achieved. In some cases, thinner lifts than noted above and/or more compaction energy may be needed to achieve the required degree of compaction.

Compaction equipment appropriate for the material type being compacted should be used. Sheepfoot compaction equipment is typically used for the fine-grained soils (clays). A vibratory steel drum roller is typically used for compaction of coarse-grained soils (sands and gravels) as well as to help seal compacted surfaces.

5.3 GENERAL CONSIDERATIONS

Fill Placement Considerations: Fill materials should not be placed on frozen soils, on frost-heaved soils, on excessively wet soils, or soils that are otherwise unstable. Borrow fill materials should not contain frozen materials at the time of placement, and frozen or frost-heaved soils should be removed prior to placement of engineered fill or other fill soils and aggregates. Excessively wet soils or aggregates should be scarified, aerated, and moisture conditioned.

Excavation Safety: Excavations and slopes should be maintained in accordance with OSHA excavation safety standards. The Contractor is solely responsible for designing and constructing stable, temporary excavations and slopes and should shore, slope, or bench the sides of the excavations and slopes as required to maintain stability of both the excavation sides and bottom. The Contractor's responsible person, as defined in 29 CFR Part 1926, should evaluate the soil exposed in the excavations as part of the contractor's safety procedures. In no case should slope height, slope inclination, or excavation depth, including utility trench excavation depth, exceed those specified in local, state, and federal safety regulations. In some cases, the use of shoring, bracing, or trench boxes may be required. ECS is providing this information solely as a service to our client. ECS is not assuming responsibility for construction site safety or the contractor's activities; such responsibility is not being implied and should not be inferred.

Bidding/Estimating Considerations: Contractors bidding or undertaking work at the site should examine the results of the subsurface exploration, satisfy themselves as to the adequacy of the information for bidding and construction, make their own interpretation of the data, and consider the effect it may have on their cost proposal, construction techniques, schedule, and equipment capabilities. Furthermore, it is recommended contractors complete additional fieldwork and exploration they deem necessary to properly prepare a cost proposal for the site work. Soil borings do not provide the same wide-scale view of the subsurface conditions that is obtained during site grading, excavation or other aspects of earthwork construction. Additional scope may be required to obtain more detailed subsurface information needed for earthwork bid preparation, which could include test pits to better understand the lateral and vertical extents of subsurface materials of concern, particularly topsoil and existing undocumented fill, which may not be as easily identifiable from disturbed auguring and/or with spilt-spoon samples. Even with this additional information, budget contingencies should be carried in construction to help cover potential variations in subsurface conditions.

6.0 CLOSING

ECS has prepared this report to guide geotechnical-related design and construction aspects of the project. We performed these services in accordance with the standard of care expected of professionals in the industry performing similar services on projects of like size and complexity at this time in the region. No other representation, express or implied, and no warranty or guarantee is included or intended in this report.

The description of the proposed project is based on information provided to ECS. If this information is inaccurate, either because of our interpretation of the documents provided, or site or design changes that may occur later, ECS should be contacted so that we can review our recommendations and provide additional or alternate recommendations as may be required to reflect the proposed construction.

We recommend ECS review the project's plans and specifications so that we may evaluate consistency of those plans/specifications with the intent of the geotechnical report recommendations.

Field observations and quality assurance testing during earthwork and foundation installation are an extension of, and integral to, the geotechnical design recommendations. We recommend that ECS be retained to apply our expertise throughout the geotechnical phases of construction, and to provide consultation and recommendations should issues arise.

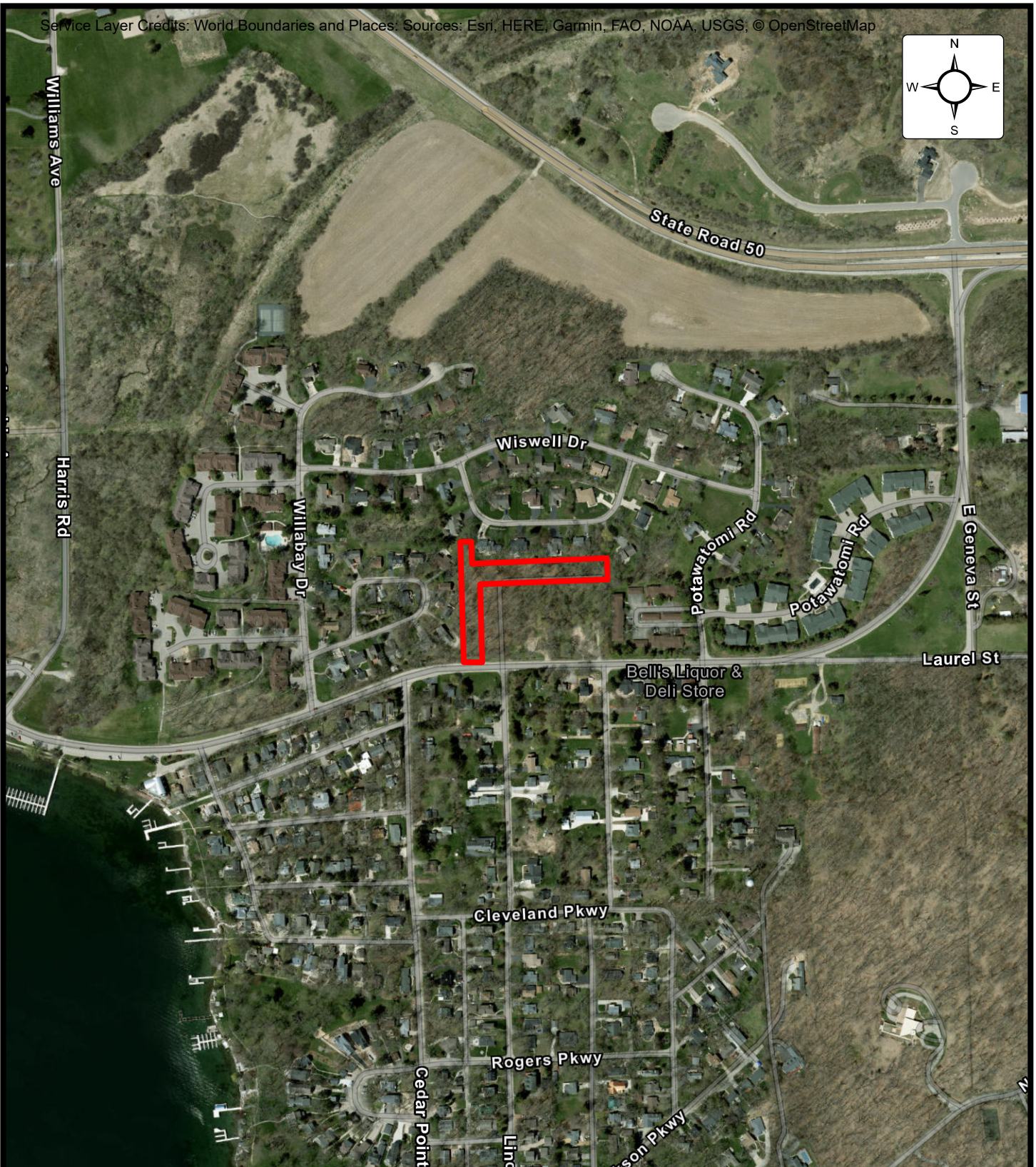
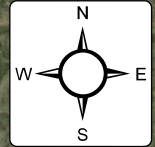
ECS is not responsible for the conclusions, opinions, or recommendations of others based on the data in this report.

Appendix A - Drawings and Reports

Site Location Diagram

Boring Location Diagram(s)

Soil Survey Map



SITE LOCATION DIAGRAM Geneva Street Infiltration

E Geneva Street, Williams Bay, Wisconsin

Village of Williams Bay

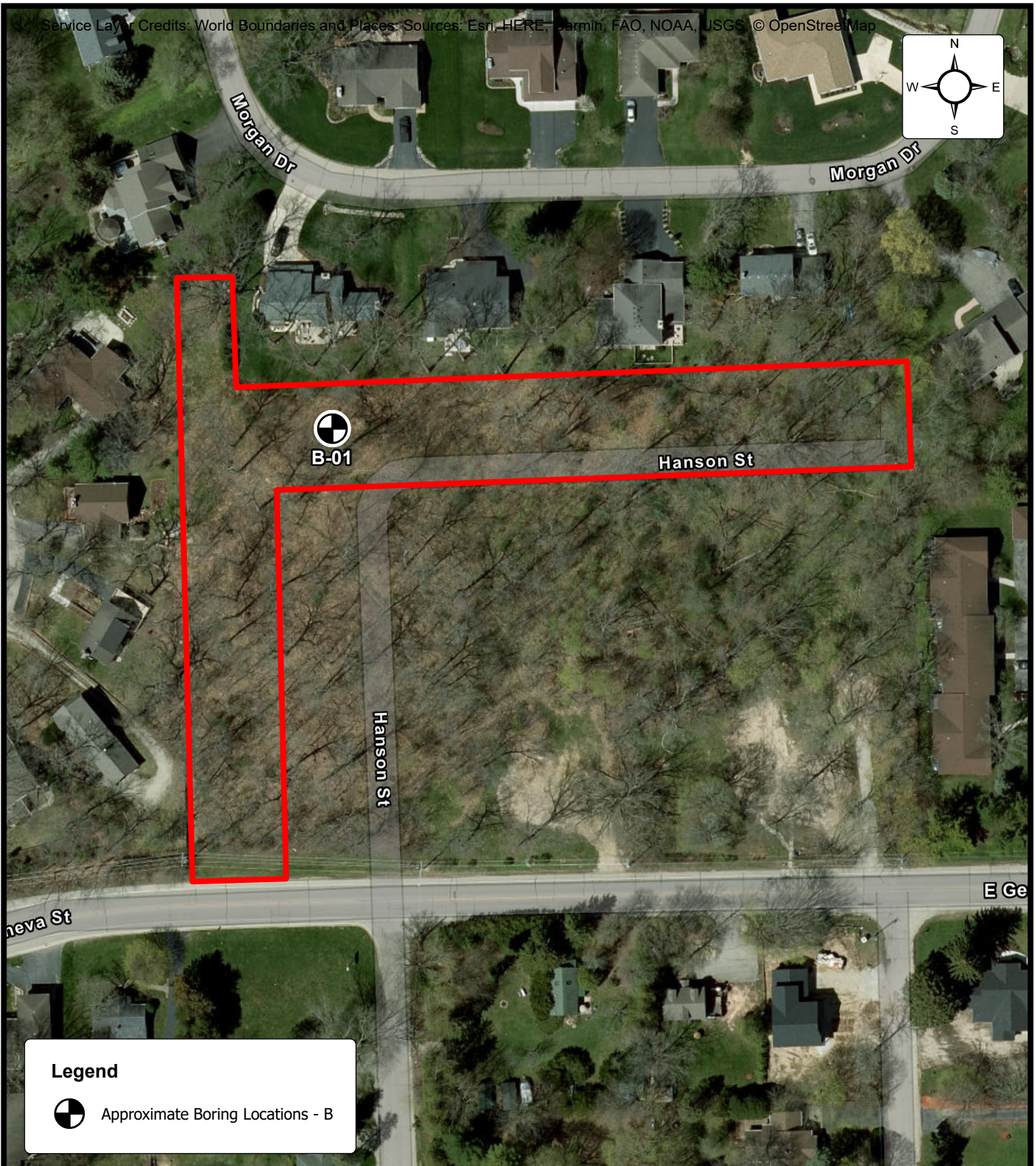
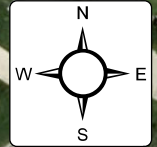
ENGINEER
DM1

SCALE
1" = 500'

PROJECT NO.
42:2648

SHEET
SLD

DATE
10/25/2024



Legend



Approximate Boring Locations - B



BORING LOCATION DIAGRAM Geneva Street Infiltration

E Geneva Street, Williams Bay, Wisconsin

Village of Williams Bay

ENGINEER
DM1

SCALE
1" = 100'

PROJECT NO.
42:2648

SHEET
BLD

DATE
10/25/2024



SOIL SURVEY MAPPING
Geneva Street Infiltration

Geneva Street, Williams Bay, Wisconsin
Village of Williams Bay

Engineer	DM
Scale	AS SHOWN
Project No.	42:2648
Figure	SS
Date	10/28/2024

SUMMARY OF SOIL SURVEY MAP UNIT CHARACTERISTICS			
Item	Map Unit		
Symbol	CyA	MyB	MyC
Landforms	Drainageways	Till plains	Till plains
Parent Material	Thin loess over loamy till	Thin loess over loamy lodgment till	Thin loess over loamy lodgment till
Typical Soil Profile	silt loam, clay loam, loam	silt loam, clay loam, loam	silt loam, clay loam, loam
Shrink-Swell Potential	Low	Low	Low
Potential for Frost Action	High	Moderate	Moderate
Corrosion Risk–Uncoated Steel	High	Low	Low
Corrosion Risk – Concrete	Low	Moderate	Moderate
Drainage	Somewhat poorly drained	Well drained	Well drained
Hydrologic Group	C/D	B	B
Hydric Soil	No	No	No
Ksat (in/hr)	0.14 to 0.57	0.57 to 1.98	0.57 to 1.98
Depth to Water Table (inches)	About 0 to 24	About 60 to 80	About 60 to 80

Appendix B – Field Operations

Reference Notes

Exploration Procedures

Boring Logs



REFERENCE NOTES FOR BORING LOGS

MATERIAL ^{1,2}	
	ASPHALT
	CONCRETE
	GRAVEL
	TOPSOIL
	VOID
	BRICK
	AGGREGATE BASE COURSE
	GW WELL-GRADED GRAVEL gravel-sand mixtures, little or no fines
	GP POORLY-GRADED GRAVEL gravel-sand mixtures, little or no fines
	GM SILTY GRAVEL gravel-sand-silt mixtures
	GC CLAYEY GRAVEL gravel-sand-clay mixtures
	SW WELL-GRADED SAND gravelly sand, little or no fines
	SP POORLY-GRADED SAND gravelly sand, little or no fines
	SM SILTY SAND sand-silt mixtures
	SC CLAYEY SAND sand-clay mixtures
	ML SILT non-plastic to medium plasticity
	MH ELASTIC SILT high plasticity
	CL LEAN CLAY low to medium plasticity
	CH FAT CLAY high plasticity
	OL ORGANIC SILT or CLAY non-plastic to low plasticity
	OH ORGANIC SILT or CLAY high plasticity
	PT PEAT highly organic soils

DRILLING SAMPLING SYMBOLS & ABBREVIATIONS			
SS	Split Spoon Sampler	PM	Pressuremeter Test
ST	Shelby Tube Sampler	RD	Rock Bit Drilling
WS	Wash Sample	RC	Rock Core, NX, BX, AX
BS	Bulk Sample of Cuttings	REC	Rock Sample Recovery %
PA	Power Auger (no sample)	RQD	Rock Quality Designation %
HSA	Hollow Stem Auger		

PARTICLE SIZE IDENTIFICATION		
DESIGNATION	PARTICLE SIZES	
Boulders	12 inches (300 mm) or larger	
Cobbles	3 inches to 12 inches (75 mm to 300 mm)	
Gravel: Coarse	¾ inch to 3 inches (19 mm to 75 mm)	
Fine	4.75 mm to 19 mm (No. 4 sieve to ¾ inch)	
Sand: Coarse	2.00 mm to 4.75 mm (No. 10 to No. 4 sieve)	
Medium	0.425 mm to 2.00 mm (No. 40 to No. 10 sieve)	
Fine	0.074 mm to 0.425 mm (No. 200 to No. 40 sieve)	
Silt & Clay ("Fines")	<0.074 mm (smaller than a No. 200 sieve)	

COHESIVE SILTS & CLAYS		
UNCONFINED COMPRESSIVE STRENGTH, QP ⁴	SPT ⁵ (BPF)	CONSISTENCY ⁷ (COHESIVE)
<0.25	<2	Very Soft
0.25 - <0.50	2 - 4	Soft
0.50 - <1.00	5 - 8	Firm
1.00 - <2.00	9 - 15	Stiff
2.00 - <4.00	16 - 30	Very Stiff
4.00 - 8.00	31 - 50	Hard
>8.00	>50	Very Hard

RELATIVE AMOUNT ⁷	COARSE GRAINED (%) ⁸	FINE GRAINED (%) ⁸
Trace	≤5	≤5
With	10 - 20	10 - 25
Adjective (ex: "Silty")	25 - 45	30 - 45

GRAVELS, SANDS & NON-COHESIVE SILTS	
SPT ⁵	DENSITY
<5	Very Loose
5 - 10	Loose
11 - 30	Medium Dense
31 - 50	Dense
>50	Very Dense

WATER LEVELS ⁶	
	WL (First Encountered)
	WL (Completion)
	WL (Seasonal High Water)
	WL (Stabilized)

FILL AND ROCK			
FILL	POSSIBLE FILL	PROBABLE FILL	ROCK

¹Classifications and symbols per ASTM D 2488-17 (Visual-Manual Procedure) unless noted otherwise.

²To be consistent with general practice, "POORLY GRADED" has been removed from GP, GP-GM, GP-GC, SP, SP-SM, SP-SC soil types on the boring logs.

³Non-ASTM designations are included in soil descriptions and symbols along with ASTM symbol [Ex: (SM-FILL)].

⁴Typically estimated via pocket penetrometer or Torvane shear test and expressed in tons per square foot (tsf).

⁵Standard Penetration Test (SPT) refers to the number of hammer blows (blow count) of a 140 lb. hammer falling 30 inches on a 2 inch OD split spoon sampler required to drive the sampler 12 inches (ASTM D 1586). "N-value" is another term for "blow count" and is expressed in blows per foot (bpf). SPT correlations per 7.4.2 Method B and need to be corrected if using an auto hammer.

⁶The water levels are those levels actually measured in the borehole at the times indicated by the symbol. The measurements are relatively reliable when augering, without adding fluids, in granular soils. In clay and cohesive silts, the determination of water levels may require several days for the water level to stabilize. In such cases, additional methods of measurement are generally employed.

⁷Minor deviation from ASTM D 2488-17 Note 14.

⁸Percentages are estimated to the nearest 5% per ASTM D 2488-17.



SUBSURFACE EXPLORATION PROCEDURE: STANDARD PENETRATION TESTING (SPT) ASTM D 1586 Split-Barrel Sampling

Standard Penetration Testing, or **SPT**, is the most frequently used subsurface exploration test performed worldwide. This test provides samples for identification purposes, as well as a measure of penetration resistance, or N-value. The N-Value, or blow counts, when corrected and correlated, can approximate engineering properties of soils used for geotechnical design and engineering purposes.

SPT Procedure:

- Involves driving a hollow tube (split-spoon) into the ground by dropping a 140-lb hammer a height of 30-inches at desired depth
- Recording the number of hammer blows required to drive split-spoon a distance of 18-24 inches (in 3 or 4 Increments of 6 inches each)
- Auger is advanced* and an additional SPT is performed
- One SPT typically performed for every two to five feet. An approximate 1.5 inch diameter soil sample is recovered.



**Drilling Methods May Vary—* The predominant drilling methods used for SPT are open hole fluid rotary drilling and hollow-stem auger drilling.

CLIENT: Village of Williams Bay				PROJECT NO.: 42:2648		BORING NO.: B-01		SHEET: 1 of 1		
PROJECT NAME: Geneva Street Infiltration				DRILLER/CONTRACTOR: Professional Testing Service						
SITE LOCATION: E Geneva Street, Williams Bay, Wisconsin, 53191								LOSS OF CIRCULATION 100		
LATITUDE: 42.579763		LONGITUDE: -88.525617		STATION:		SURFACE ELEVATION: 907.0		BOTTOM OF CASING 		

DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	WATER LEVELS	ELEVATION (FT)	BLOWS/6" (TCP/MC/SPT-N value) *	STANDARD PENETRATION BLOWS/FT		ROCK QUALITY DESIGNATION & RECOVERY		LIQUID LIMIT X PLASTIC LIMIT		CALIBRATED PENETROMETER TSF					WATER CONTENT % [FINES CONTENT] %				
									10 20 30 40 50		10 20 30 40 50		1 2 3 4 5		10 20 30 40 50									
									RQD		REC		MC SAMPLER BLOWS/FT		TEXAS CONE PENETRATION BLOWS/FT									
									10 20 30 40 50		10 20 30 40 50													
	S-1	SS	24	24	Topsoil Thickness[6"]			4-4-5-6 (9)																
	S-2	SS	24	20	(CL) LEAN CLAY, trace sand, contains slight roots, brown, moist, stiff to hard, contains some SILT lenses (organic content = 2.8%)			4-5-5-7 (10)																
5	S-3	SS	24	10	(CL) LEAN CLAY WITH SAND, trace sand, contains slight roots, brown, moist, hard, contains SILT lenses		902	3-5-7-7 (12)																
	S-4	SS	13	13	(GC) CLAYEY GRAVEL WITH SAND, brown, moist, hard (organic content = 0.8%) (possible cobbles at 7+/- feet)			6-7-50/1" (57/7")																
10	S-5	SS	24	16				9-10-7-8 (17)																
	S-6	SS	24	20				5-6-6-7 (12)																
	S-7	SS	24	16	(CL) SANDY LEAN CLAY WITH GRAVEL, brown, moist, stiff to very stiff			7-5-4-4 (9)																
15	S-8	SS	24	18				4-4-4-4 (8)																
	S-9	SS	24	24				3-4-6-6 (10)																
20	S-10	SS	24	9				5-5-6-7 (11)																
					END OF BORING AT 20 FT			887																
25																								
30																								

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL

WL (First Encountered) Dry	BORING STARTED: Oct 09 2024	CAVE IN DEPTH:
WL (Completion) 	BORING COMPLETED: Oct 09 2024	HAMMER TYPE: Auto
WL (Seasonal High Water) 	EQUIPMENT: Geoprobe 7822DT	DRILLING METHOD: Hollow Stem Auger
WL (Stabilized) 	LOGGED BY: LEM1	

GEOTECHNICAL BOREHOLE LOG

Appendix C – Laboratory Testing

Other Laboratory Test Results
Textural Triangle USDA Test(s)
Grain Size Analysis/Analyses
Plasticity Chart(s)



LABORATORY PROCEDURES:

Index Testing

Moisture content determination was performed on select fine-grained soil samples in accordance with ASTM D 2216.

Calibrated hand penetrometer tests (Qp) were performed on select cohesive soil samples. In the hand penetrometer test, the unconfined compressive strength of a soil sample is estimated, to a maximum of 4.5 or 6 tons per square foot (tsf), depending on the penetrometer model, by measuring the resistance of a soil sample to penetration by a small, calibrated, spring-loaded cylinder.

Atterberg limits determination was performed on select fine-grained soil samples in accordance with ASTM D 4319. The Atterberg limits are a basic measure of the critical water contents of a fine-grained soil: its **liquid limit**, **plastic limit**, and **shrinkage limit**. Depending on its water content, a soil may appear in one of four states: solid, semi-solid, plastic and liquid. In each state, the consistency and behavior of a soil is different and consequently so are its engineering properties. Atterberg limits can also be used to help distinguish between silt and clay, and to distinguish between different types of silts and clays.

Particle size distribution, also referred to as gradation or sieve analysis, refers to the proportions by dry mass of a soil particles distributed over specified particle-size ranges. The particle size distribution is used to help classify soils for engineering purposes. Particle size distribution determination was performed on select fine-grained soil samples in accordance with ASTM D 421, D 422 and/or D 1140.

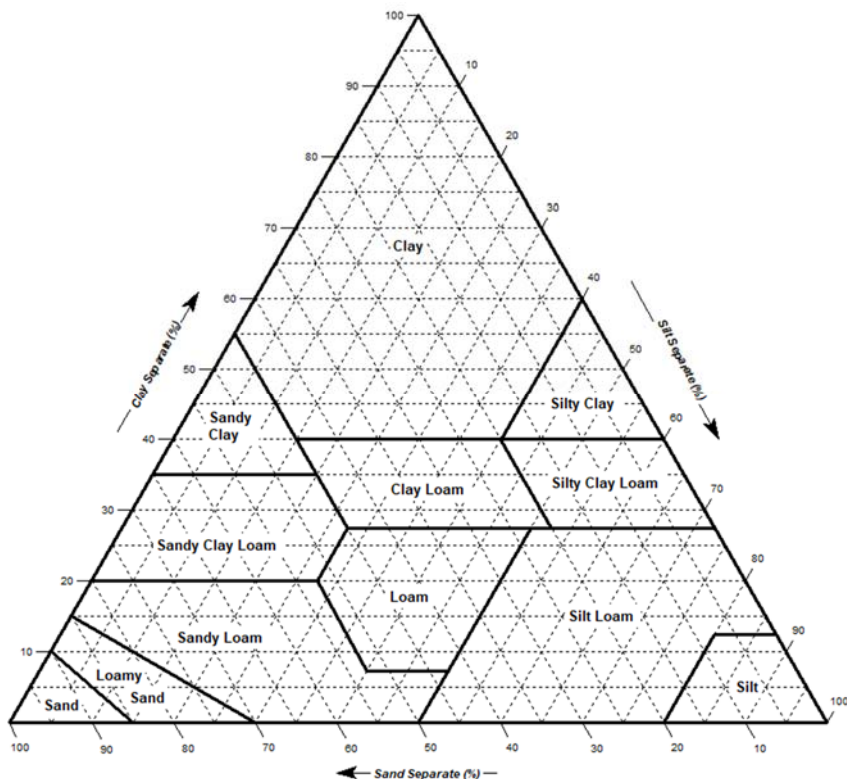
A **loss on ignition (LOI)** test is used to estimate the organic content of the soil. In the LOI test a dry sample is heated to 440° C to burn off organic matter within the sample. The lost weight is compared to the initial dry weight to estimate the percentage of organics in the material. LOI determination was performed in accordance with ASTM D 2974.



U.S. Department of Agriculture (USDA) Soil Classification System

Texture Triangle

Fine Earth Texture Classes (—)



Texture Class

Texture Class or Subclass	Code	
	Conv.	NASIS
Coarse Sand	cos	COS
Sand	s	S
Fine Sand	fs	FS
Very Fine Sand	vfs	VFS
Loamy Coarse Sand	lcos	LCOS
Loamy Sand	ls	LS
Loamy Fine Sand	lfs	LFS
Loamy Very Fine Sand	lvfs	LVFS
Coarse Sandy Loam	cosl	COSL
Sandy Loam	sl	SL
Fine Sandy Loam	fsl	FSL
Very Fine Sandy Loam	vfsl	VFSL
Loam	l	L
Silt Loam	sil	SIL
Silt	si	SI
Sandy Clay Loam	scl	SCL
Clay Loam	cl	CL
Silty Clay Loam	sicl	SICL
Sandy Clay	sc	SC
Silty Clay	sic	SIC
Clay	c	C

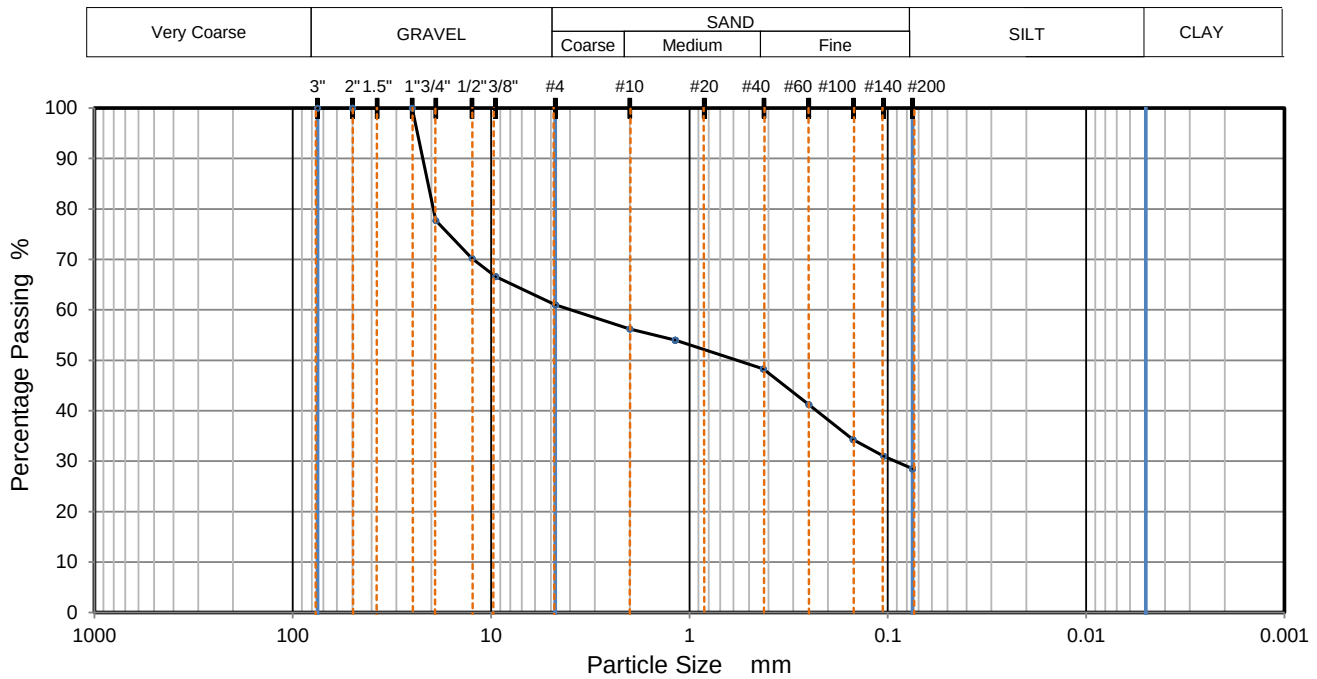
Texture Modifiers – Conventions for using “Rock Fragment Texture Modifiers” and for using textural adjectives that convey the “% volume” ranges for **Rock Fragments – Size and Quantity**.

Fragment Content % By Volume	Rock Fragment Modifier Usage
< 15	No texture adjective is used (noun only; e.g., <i>loam</i>).
15 to < 35	Use adjective for appropriate size; e.g., <i>gravelly</i> .
35 to < 60	Use “very” with the appropriate size adjective; e.g., <i>very gravelly</i> .
60 to < 90	Use “extremely” with the appropriate size adjective; e.g., <i>extremely gravelly</i> .
≥ 90	No adjective modifier. If ≤ 10% fine earth, use the appropriate noun for the dominant size class; e.g., <i>gravel</i> . Use terms in lieu of texture.

Texture Modifiers – (Adjectives)

Rock Fragments: Size and Quantity	Code		Criteria: Percent (by volume) of total rock fragments and dominated by (<i>name size</i>):
	Conv.	PDP/ NASIS	
<i>Rock Fragments (> 2mm; ≥ Strongly Cemented)</i>			
Gravelly	GR	GR	≥ 15% but < 35% gravel
Fine Gravelly	FGR	GRF	≥ 15% but < 35% fine gravel
Medium Gravelly	MGR	GRM	≥ 15% but < 35% med. gravel
Coarse Gravelly	CGR	GRC	≥ 15% but <35% coarse gravel
Very Gravelly	VGR	GRV	≥ 35% but < 60% gravel
Extremely Gravelly	XGR	GRX	≥ 60% but < 90% gravel
Cobbly	CB	CB	≥ 15% but < 35% cobbles
Very Cobbly	VCB	CBV	≥ 35% but < 60% cobbles
Extremely Cobbly	XCB	CBX	≥ 60% but < 90% cobbles
Stony	ST	ST	≥ 15% but < 35% stones
Very Stony	VST	STV	≥ 35% but < 60% stones
Extremely Stony	XST	STX	≥ 60% but < 90% stones
Bouldery	BY	BY	≥ 15% but < 35% boulders
Very Bouldery	VBY	BYV	≥ 35% but < 60% boulders
Extremely Bouldery	XBY	BYX	≥ 60% but < 90% boulders
Channery	CN	CN	≥ 15% but < 35% channers
Very Channery	VCN	CNV	≥ 35% but < 60% channers
Extremely Channery	XCN	CNX	≥ 60% but < 90% channers
Flaggy	FL	FL	≥ 15% but < 35% flagstones
Very Flaggy	VFL	FLV	≥ 35% but < 60% flagstones
Extremely Flaggy	XFL	FLX	≥ 60% but < 90% flagstones

PARTICLE SIZE DISTRIBUTION



TEST RESULTS ()

Sieving		Hydrometer Sedimentation	
Particle Size	% Passing	Particle Size mm	% Passing
3"	100.0		
2"	100.0		
1"	100.0		
3/4"	77.7		
1/2"	70.2		
3/8"	66.6		
#4	61.0		
#10	56.2		
#16	54.0		
#40	48.3		
#60	41.2		
#100	34.3		
#140	31.0		
#200	28.5		

Dry Mass of sample, g

536.6

Sample Proportions	% dry mass
Very coarse, >3" sieve	0.0
Gravel, 3" to # 4 sieve	39.0
Coarse Sand, #4 to #10 sieve	4.8
Medium Sand, #10 to #40	7.9
Fine Sand, #40 to #200	19.8
Fines <#200	28.5

USCS	GC	Liquid Limit	D90	22.110	D50	0.576	D10	
AASHTO		Plastic Limit	D85	20.790	D30	0.092	Cu	
USCS Group Name	Clayey Gravel with Sand	Plasticity Index	D60	3.967	D15		Cc	

Project: Geneva Street Infiltration
 Client: Village of Williams Bay
 Sample Description: Clayey Gravel with Sand
 Sample Source: B-01

Project No.: 42:2648
 Depth (ft): 8.0 - 10.0
 Sample No.: S-5
 Date Reported: 10/25/2024



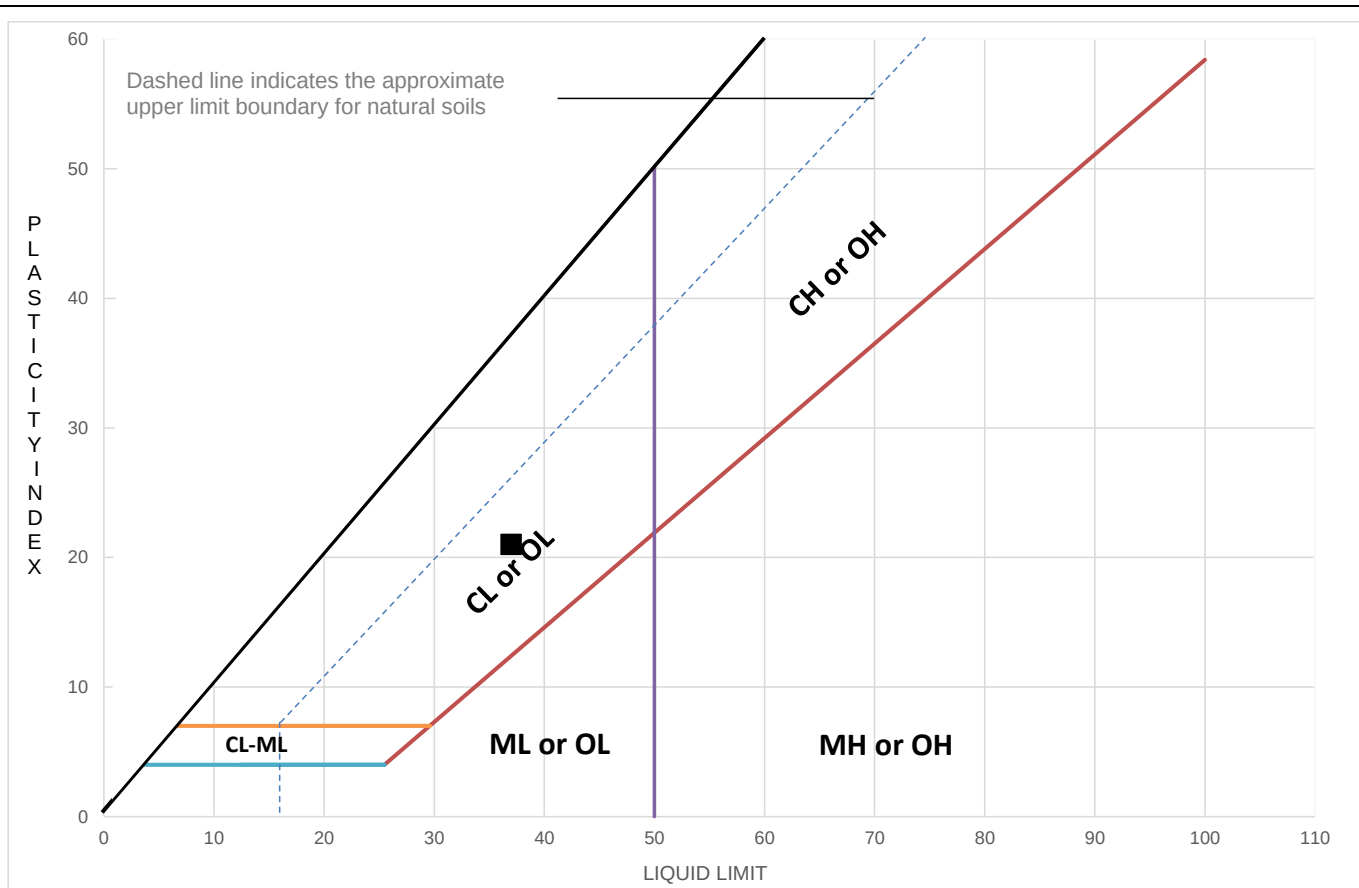
Office / Lab
 ECS Midwest LLC - Milwaukee

Address
 3695 N. 126TH Street
 Unit C
 Brookfield, WI 53005

Office Number / Fax
 (414)286-7557
 (262)886-1988

Tested by	Checked by	Approved by	Date Received	Remarks
LMcGovern	LMcGovern	LMcGovern		USDA Classification: Very Gravely Sandy Clay Loam

LIQUID AND PLASTIC LIMITS TEST REPORT



TEST RESULTS (ASTM D4318-10 (MULTIPOINT TEST))

	Sample Location	Sample Number	Sample Depth (ft)	LL	PL	PI	%<#40	%<#200	AASHTO	USCS	Material Description
■	B-01	S-3	4.00-6.00	37	16	21					

Project: Geneva Street Infiltration
Client: Village of Williams Bay

Project No.: 42:2648
Date Reported: 10/22/2024



Office / Lab
ECS Midwest LLC - Milwaukee

Address
3695 N. 126TH Street Unit C
Brookfield, WI 53005

Office Number / Fax
(414)286-7557
(262)886-1988

Tested by	Checked by	Approved by	Date Received
LMcGovern	LMcGovern	LMcGovern	

Appendix D – Supplemental Documents

Other Supplemental Documents



Soil and Site Evaluation – Stormwater Infiltration

In accordance with SPS 382.365, 385, Wis. Adm. Code, and WDNR Standard 1002

Personal information you provide may be used for secondary purposes [Privacy Law, s. 15.04(1)(m)]

Page 1 of 2

Attach a complete site plan on paper not less than 8 ½ x 11 inches in size. Plan must include, but is not limited to: vertical and horizontal reference point (BM); direction and percent of slope; scale or dimensions; north arrow; and BM referenced to nearest road.		COUNTY Walworth
PLEASE PRINT ALL INFORMATION		PARCEL ID
PROPERTY OWNER Village of Williams Bay	PROPERTY LOCATION Govt. Lot ____, NE ¼, NE ¼, S 6 ____, T 1 ____, R 17 E Lot #, Block #, Subd. Name or CSM #: _____	
PROPERTY OWNER'S MAILING ADDRESS	Municipality: Williams Bay ☐ City ☒ Village ☐ Town	
CITY, STATE, ZIP CODE	PHONE	Nearest Road: E. Geneva Street
Drainage area _____ ☐ sq. ft. ☐ acres	HYDRAULIC APPLICATION TEST METHOD ☒ Morphological Evaluation ☐ Double Ring Infiltrometer ☐ Other: (specify) _____	SOIL MOISTURE Date of soil borings: 10/9/2024 USDA-NRCS WETS VALUE: ☐ Dry = 1 ☒ Normal = 2 ☐ Wet = 3
Test site suitable for (check all that apply): ☐ Site not suitable ☐ Bioretention ☐ Reuse ☐ Subsurface Dispersal System ☐ Irrigation ☐ Other _____		

B-01 #OBS. ☐ Pit ☒ Boring Ground Surface Elevation 9007 ft. Elevation of Limiting Factor -- ft.

Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frags.	% Fines	Hydraulic App. Rate Inches/Hr.
A	0-6	--	Topsoil	--	--	--	--	--	--	--
B	6-48	10YR 5/3	No Redox Features	c	1 vf abk	mvfi	g	< 5	85-95	0.07
C	48-72	10YR 5/3	No Redox Features	c	1 vf abk	mvfi	g	< 5	80-90	0.07
C	72-144	10YR 5/3	No Redox Features	vgr-scl	1 vf abk	mfi	g	44	28	0.11
C	144-240	10YR 5/3	No Redox Features	gr-cl	1 vf abk	mfi	g	15	60-70	0.03

Comments:

No water encountered.
No bedrock encountered

#OBS. ☐ Pit ☐ Boring Ground Surface Elevation _____ ft. Elevation of Limiting Factor _____ ft.

Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frags.	% Fines	Hydraulic App. Rate Inches/Hr.

Comments:

#OBS. ☐ Pit ☐ Boring Ground Surface Elevation _____ ft. Elevation of Limiting Factor _____ ft.

Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frags.	% Fines	Hydraulic App. Rate Inches/Hr.

Comments:

Overall Site Comments:

Matthew Meyer		1053414 (SP-110700001)
_____ Name (Please Print)	_____ Signature	_____ Credential Number
1060 Breezewood Ln, Suite 102 Neenah, WI 54956	10/25/2024	920.886.1406
_____ Address	_____ Date Evaluation Conducted	_____ Phone Number

Appendix E – Other Information

GBA - Geotechnical Engineering Report Information Sheet

Important Information about This Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you – assumedly a client representative – interpret and apply this geotechnical-engineering report as effectively as possible. In that way, you can benefit from a lowered exposure to problems associated with subsurface conditions at project sites and development of them that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed herein, contact your GBA-member geotechnical engineer. Active engagement in GBA exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

Understand the Geotechnical-Engineering Services Provided for this Report

Geotechnical-engineering services typically include the planning, collection, interpretation, and analysis of exploratory data from widely spaced borings and/or test pits. Field data are combined with results from laboratory tests of soil and rock samples obtained from field exploration (if applicable), observations made during site reconnaissance, and historical information to form one or more models of the expected subsurface conditions beneath the site. Local geology and alterations of the site surface and subsurface by previous and proposed construction are also important considerations. Geotechnical engineers apply their engineering training, experience, and judgment to adapt the requirements of the prospective project to the subsurface model(s). Estimates are made of the subsurface conditions that will likely be exposed during construction as well as the expected performance of foundations and other structures being planned and/or affected by construction activities.

The culmination of these geotechnical-engineering services is typically a geotechnical-engineering report providing the data obtained, a discussion of the subsurface model(s), the engineering and geologic engineering assessments and analyses made, and the recommendations developed to satisfy the given requirements of the project. These reports may be titled investigations, explorations, studies, assessments, or evaluations. Regardless of the title used, the geotechnical-engineering report is an engineering interpretation of the subsurface conditions within the context of the project and does not represent a close examination, systematic inquiry, or thorough investigation of all site and subsurface conditions.

Geotechnical-Engineering Services are Performed for Specific Purposes, Persons, and Projects, and At Specific Times

Geotechnical engineers structure their services to meet the specific needs, goals, and risk management preferences of their clients. A geotechnical-engineering study conducted for a given civil engineer

will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client.

Likewise, geotechnical-engineering services are performed for a specific project and purpose. For example, it is unlikely that a geotechnical-engineering study for a refrigerated warehouse will be the same as one prepared for a parking garage; and a few borings drilled during a preliminary study to evaluate site feasibility will not be adequate to develop geotechnical design recommendations for the project.

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project or purpose;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, the reliability of a geotechnical-engineering report can be affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If you are the least bit uncertain about the continued reliability of this report, contact your geotechnical engineer before applying the recommendations in it. A minor amount of additional testing or analysis after the passage of time – if any is required at all – could prevent major problems.*

Read this Report in Full

Costly problems have occurred because those relying on a geotechnical-engineering report did not read the report in its entirety. Do not rely on an executive summary. Do not read selective elements only. *Read and refer to the report in full.*

You Need to Inform Your Geotechnical Engineer About Change

Your geotechnical engineer considered unique, project-specific factors when developing the scope of study behind this report and developing the confirmation-dependent recommendations the report conveys. Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the elevation, configuration, location, orientation, function or weight of the proposed structure and the desired performance criteria;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project or site changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept*

responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.

Most of the “Findings” Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site’s subsurface using various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing is performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgement to form opinions about subsurface conditions throughout the site. Actual site-wide subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team through project completion to obtain informed guidance quickly, whenever needed.

This Report’s Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, they are not final, because the geotechnical engineer who developed them relied heavily on judgement and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* exposed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.*

This Report Could Be Misinterpreted

Other design professionals’ misinterpretation of geotechnical-engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a continuing member of the design team, to:

- confer with other design-team members;
- help develop specifications;
- review pertinent elements of other design professionals’ plans and specifications; and
- be available whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction-phase observations.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note*

conspicuously that you’ve included the material for information purposes only. To avoid misunderstanding, you may also want to note that “informational purposes” means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. This happens in part because soil and rock on project sites are typically heterogeneous and not manufactured materials with well-defined engineering properties like steel and concrete. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled “limitations,” many of these provisions indicate where geotechnical engineers’ responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a “phase-one” or “phase-two” environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnical-engineering report does not usually provide environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures.* If you have not obtained your own environmental information about the project site, ask your geotechnical consultant for a recommendation on how to find environmental risk-management guidance.

Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, the engineer’s services were not designed, conducted, or intended to prevent migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, *proper implementation of the geotechnical engineer’s recommendations will not of itself be sufficient to prevent moisture infiltration.* Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. *Geotechnical engineers are not building-envelope or mold specialists.*



GEOPROFESSIONAL
BUSINESS
ASSOCIATION

Telephone: 301/565-2733

e-mail: info@geoprofessional.org www.geoprofessional.org

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Appendix 6 MAINTENANCE AGREEMENT

Storm Water Management Practice Maintenance Agreement

Document Number

THIS AGREEMENT, made and entered into this _____ day of _____, 2025, by and between Canyon Development, hereinafter called the "Owner", and the Village of Williams Bay, hereinafter called the "Village".

WITNESSETH:

WHEREAS, the Owner is the owner of the following described lands situated in the Village of Williams Bay, County of Walworth County, State of Wisconsin, to-wit:

Tax Parcel Number: WHA 00001 and WHA 00006

hereinafter called the "Property".

WHEREAS, the Owner is developing the property; and

WHEREAS, the Site Plan known as Canyon Development-Residential Site, hereinafter called the "Plan", which is expressly made a part hereof, as approved or to be approved by the Village, provides for on-site storm water management practices within the confines of the Property; and

Name and Return Address:

Attn: Wayne Edwards
Village of Williams Bay
250 Williams Bay
Williams Bay, Wisconsin 53191

WHEREAS, the Village and the Owner, its successors and assigns, including any homeowners association, agree that the health, safety and welfare of the residents of the Village of Williams Bay, require that on-site storm water management practices as defined in the Williams Bay Municipal Code be constructed and maintained on the Property; and

WHEREAS, the Village requires that on-site storm water management practices as shown on the Plan be constructed and adequately maintained by the Owner, its successors, and assigns, including any homeowners association.

NOW, THEREFORE, in consideration of the foregoing premises, the mutual covenants contained herein, and the following terms and conditions, the parties hereto agree as follows:

1. The on-site storm water management practices shall be constructed by the Owner, its successors, and assigns, including any homeowners association, in accordance with the plans and specifications indicated in the Plan and applicable statutes, ordinances and rules. The storm water management practices shall serve the drainage area designated in the Plan.
2. The Owner, its successors and assigns including but not limited to any homeowners association, shall regularly inspect the storm water management practices and specifically the function of the approved storm water management system as often as conditions require, but in any event at least once each year. The standard Operation and Maintenance Report attached to this agreement as Exhibit A and by this reference made a part hereof shall be used for the purpose of the regular inspections of the storm water management practices. The Owner, its successors and assigns shall keep the Operation and Maintenance Reports from past inspections as well as a log of maintenance activity indicating the date and type of maintenance completed. The Reports and maintenance log shall be made available to the Village for review. The purpose of the inspections is to assure safe and proper functioning of the facilities. The inspections shall cover all facilities including but not limited to storm sewer, underdrains, inlets/grates, and wet detention basins. Deficiencies shall be noted in the Operation and Maintenance Report.
3. The Owner, its successors, and assigns, including any homeowners association, shall adequately maintain the storm water management practices, including but not limited to all pipes and channels built to convey storm

water to the facility, as well as all structures, improvements, and vegetation provided to control the quantity and quality of the storm water. Adequate maintenance is herein defined as keeping the storm water management facilities in good working condition so that these facilities are performing their design functions and are in accordance with the Operation and Maintenance Standards attached to this agreement as Exhibit B and by this reference made a part hereof.

4. The Owner, its successors, and assigns, including any homeowners Association, hereby grant permission to the Village, its authorized agents, and employees, to enter upon the Property and to inspect the storm water management practices whenever the Village deems necessary. The purpose of inspection is to investigate reported deficiencies and/or to respond to citizen complaints. The Village shall provide the Owner, its successors, and assigns, including any homeowners association, copies of the inspection findings and a directive to commence with the repairs if necessary. Corrective actions shall be taken within a reasonable timeframe as established by the Village Engineer.
5. If the Owner, its successors and assigns, including any homeowners Association, fails to maintain the storm water management practices in good working condition, consistent with the terms of the approved plans and specifications approved by the Village and does not perform the required corrective actions and inspections in the specified time, the Village may:
 - a. Issue a citation to the Owner, its successors, and assigns. Each day that the violation exists shall constitute a separate offense and shall be considered a continuing violation.
 - b. Perform the corrective actions identified in the inspection report and assess the Owner, its successors and assigns for the cost of such work. The cost of such work may be specially assessed against the property pursuant to Wisconsin Statutes Section 66.0703. If the facilities are located on an outlot owned collectively by homeowners association, the Village may assess each member of the homeowners association according to the ownership interest in the facilities located on the property. This provision shall not be construed to allow the Village to erect any structure of permanent nature on the land of the Owner outside of the easement for the storm water management practices, and in no event shall this Agreement be construed to impose any such obligation on the Village.
6. The Owner, its successors, and assigns, including any homeowners Association, will perform the work necessary to keep these facilities in good working order as appropriate. In the event a maintenance schedule for the storm water management practices (including sediment removal) is outlined on the approved plans, the schedule will be followed. The minimal amount of maintenance on the storm water management practices shall be in accordance with the Operation and Maintenance Standards (Exhibit B).
7. In the event the Village pursuant to this Agreement, performs work of any nature, or expends any funds in performance of said work for labor, use of equipment, supplies, materials, and the like, the Owner, its successors and assigns, including any homeowners association, shall reimburse the Village upon demand, within thirty (30) days of receipt for all actual costs incurred by the Village hereunder.
8. This Agreement imposes no liability of any kind whatsoever on the Village, its officers, agents and employees, and the owner agrees to indemnify and hold the Village harmless as and against any and all claims, actions, causes of action, demands, including attorney fees which the Village may incur as a result of the failure of the storm water management system and/or actions taken or not taken by the Village to enforce the terms of this agreement including, but not limited to, the performance of maintenance activities.
9. This Agreement shall be attached as an exhibit to any document which creates a homeowners association that is responsible for maintenance of the storm water management practices and be recorded at the Walworth County Register of Deeds, and shall constitute covenant running with the land, and shall be binding on the Owner, its administrators, executors, assigns, heirs and any other successors in interests, including any homeowners association. The owner shall provide the Village with a copy of any document which creates a homeowner's association that is responsible for the storm water management practices.
10. Notwithstanding anything in this Agreement to the contrary, in the event the Owner, or the Owner's successors and assigns, sell or otherwise transfer ownership in the Property, the Owner and the Owners successors and assigns, are hereby released from any and all liabilities and obligations under the terms of this Agreement.
11. This Agreement shall be governed and construed in accordance with the laws of the State of Wisconsin and may only be modified in a writing executed by both parties.
12. The Owner stipulates that this agreement permits the Village to have access to his or her property for purposes of storm water inspection and maintenance and that the Owner waives the right to an appraisal or to

pursue any other rights it may have pursuant to Ch 32 of the Wisconsin statutes.

13. Any notice, request or demand required or permitted under this Agreement shall be in writing and shall be deemed given when personally served or three (3) days after the same has been deposited with the United States Post Office, registered or certified mail, return receipt requested, postage prepaid and addressed as follows:

If to Owner: Canyon Development
937 Tarrant Drive
Fontana, WI 53125
Contact: Stephen Greenburg
Phone: 815-451-2276
Email: steve@cdgrp.net

If to City: Village of Williams Bay
250 Williams Bay Williams
Bay, WI 53191
Contact: Wayne Edwards Phone:
262-245-2706
Email: wedwards@willisamsbay.wi.gov

Any party may change its address for the receipt of notice by written notice to the other.

IN WITNESS WHEREOF, the parties have executed this agreement as of the date first above written.

PROPERTY OWNER

Canyon Development

Canyon Development Representative

The foregoing Agreement was acknowledged before me this ____ day of
_____, 2025, by _____.

NOTARY PUBLIC

My Commission Expires: _____

This document was drafted by:
Lynch & Associates, LLC. Contact:
Ryan W. Kloth, P.E., CFM
Email: rkloth@lynch-engineering.com

Approved for recording: _____

EXHIBIT A

Wet Detention Basin

Operation, Maintenance & Management Report

Project: Canyon Development Location Site Status:

 Inspector: Date:

 Time:

Item	Satisfactory (YES/NO)	Comments
<i>Inspection - required monthly for first 12 months, annually thereafter, and after every major storm event</i>		
Vegetation and ground cover adequate		
Signs of embankment erosion		
Animal burrows		
Emergency spillway stabilized		
Surrounding areas stabilized		
Basin free of trash and debris		
Signs of sediment buildup		
Inlet clean and working		
Outlet pipe and outlet control structure clean and working		
Other (describe)		
<i>Maintenance</i>		
Inlet cleaning - once per year		
Outlet pipe and outlet control structure – once per year		
Repair eroding areas and replant - as needed		
Replace plants or reseed – as needed		
Trash and debris removal - as needed		
Other (describe)		

Additional Comments - Actions to be Taken:

EXHIBIT B

STORMWATER WET DETENTION BASIN OPERATION AND MAINTENANCE STANDARDS

I. ROUTINE MAINTENANCE

A. Inspection

1. Side slopes, embankments, and emergency spillways that are not rock lined which have been planted with turf grasses should be mowed at least twice a year to prevent woody growth and control noxious weeds.
2. Adjacent to the residential areas, more frequent mowing, typically once a week during a normal growing season, is recommended for aesthetic and allergy control purposes.
3. Native grasses should be mowed to a height of 6" in mid to late summer or after they have achieved a height of 1-1/2 feet during the first growing season. Further mowing in subsequent growing seasons will not be required.
4. If possible, the native grass area should be burned off every three to four years in the spring of the year. Check local burning regulations as permits may be required.
5. If burning of the native grass areas is not possible, a 5 to 8" mowing every 3 to 4 years, may suffice as a substitute management technique. The mowed area should be raked and performed in the spring.

B. Inspections

1. Inspections of the ponds shall be completed on an annual basis or after significant rainfall events.
2. The inspections should be completed during wet weather conditions to determine if the ponds are functioning properly.
3. Inspection priorities shall be as follows:
 - a. Inspect the embankments for subsidence, erosion, cracking, and tree growth.
 - b. Inspect the condition of the emergency spillway and overland flow path.
 - c. Inspect the pond for accumulation of sediment.
 - d. Inspect the outlet control structure for clogs, debris, and material failures.
 - e. Inspect upstream and downstream channels from an erosion perspective.
 - f. Inspect any modifications that may have been made to the ponds following their initial construction.
 - g. Inspect the side slopes of the pond for erosion, slumping, cracking, or woody plant materials.
4. As-built plans shall accompany the person responsible for the pond inspections.
5. Documentation of the inspections should be completed and filed. Documentation should include as a minimum:
 - a. Inspectors name, affiliation and professional credentials if applicable.
 - b. Date, time and weather conditions.
 - c. Approximate rainfall total over a 24-hour period if applicable.
 - d. Existing embankment, outlet and inlet conveyance systems and vegetation condition.
 - e. Sediment depth at the outlet control structure and at a minimum one other location.
 - f. Identification of potential structural failures and repair needs.

- g. Other pond conditions such as vegetation growth, algae growth, and emergency spillway conditions.
- h. Repair recommendations.

C. Debris and Litter Removal.

1. Debris and litter removal from the pond surface shall be completed at least once a month.
2. Particular attention should be paid to debris accumulating around the riser pipe to prevent potential clogging.

D. Erosion Control.

1. The pond side slopes, embankments and emergency spillways may suffer from periodic slumpage and erosion.
2. Corrective measures shall include regrading, filling, and revegetation of the eroded or slumping areas.
3. Rip rap at the pond outlet and emergency spillways should be inspected for displacement or undermining. Repairs shall be made upon discovery.

E. Nuisance Control.

1. Biological control of algae and mosquitoes is preferred over chemical control. Consultation with local WDNR officials is recommended prior to the introduction of any biological control.
2. Maintaining the native grass perimeter will aid in the control of geese.
3. Mechanical controls should be used when feasible.

II. NON-ROUTINE MAINTENANCE

A. Structural Repairs and Replacement.

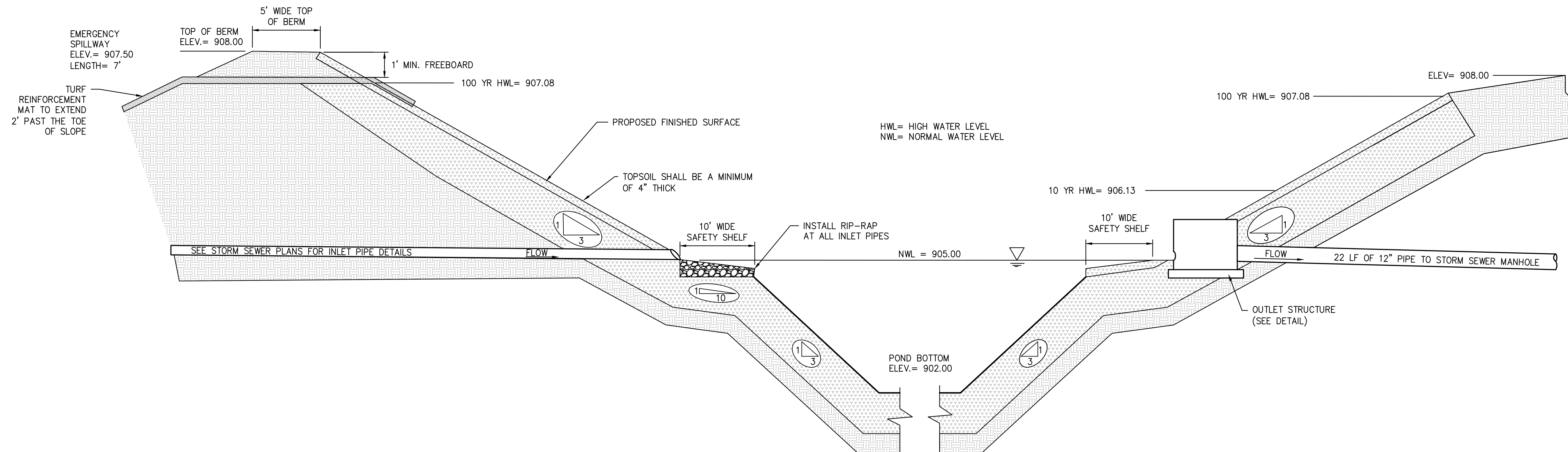
1. The outlets of the pond have been constructed utilizing concrete pipe. The estimate life of these structures is 75 to 100 years. Annual inspection of the structures will disclose any potential structural problems. If structural problems appear, repair, or replace the outlet.
2. Excessive or chronic drawdowns of the ponds may cause leaks or seepage through the embankments. Excessive drawdowns should be avoided and thus corrective measures for leakage and seepage can be avoided.

B. Sediment Removal.

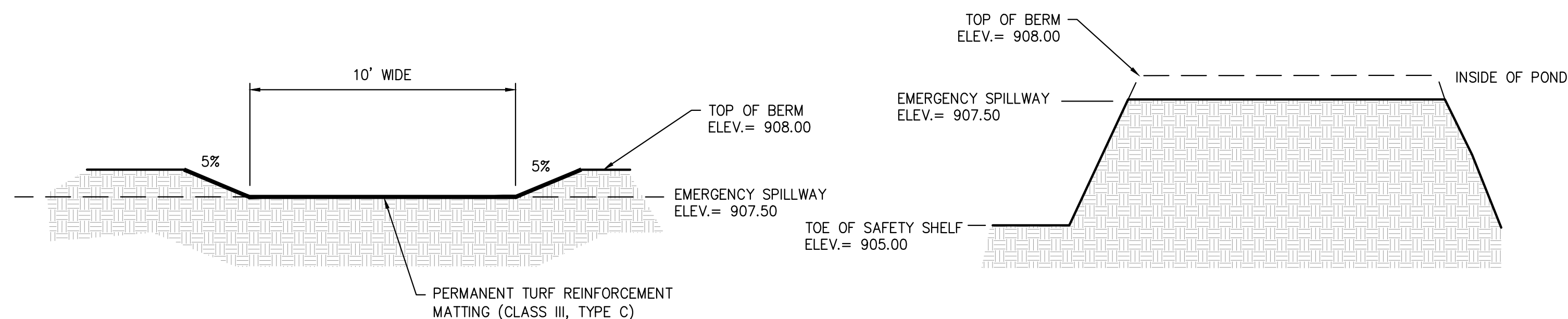
1. A sediment clean out cycle of 10 to 15 years is recommended. Sediment removal is required when the average depth of the permanent pool is 3.5 feet. Annual inspections shall be made to ensure that the design depth of the permanent water pool is maintained.
2. Sediment removed from the ponds shall be hauled to an upland area, spread and stabilized with vegetative material.
3. It is recommended that the sediment be tested to determine if land filling is necessary. Contact the local DNR prior to sediment sampling and testing to ensure compliance with State standards and regulations.
4. Surveyed depths of the sediment storage area and permanent pool elevations shall be made immediately following the construction of the ponds and recorded on the as-built plans. Annual inspections shall include measure downs to determine sediment elevations in relation to the permanent pool elevation.

Appendix 7 CONSTRUCTION DETAILS

1. ALL CONSTRUCTION SHALL MEET THE SPECIFICATIONS OF WDNW WET DETENTION BASIN TECHNICAL STANDARDS 1001.
2. ALL WORK TO BE CONDUCTED IN CONFORMANCE WITH THE STORMWATER MANAGEMENT PLAN FOR THE PROJECT SITE AS APPROVED BY THE REGULATORY ENGINEER OF RECORD.
3. RCP PIPING & INSTALLATION WITHIN THE BASIN SHALL BE IN ACCORDANCE WITH (ASTM C-76/C443) WITH RUBBER GASKETS.
4. FOR CONSTRUCTED EMBANKMENTS WHERE THE PERMANENT POOL IS PONDED 3 FEET OF MORE AGAINST THE EMBANKMENT. THERE SHALL BE A CORE TRENCH OR KEYWAY ALONG THE CENTERLINE OF THE EMBANKMENT UP TO THE PERMANENT POOL ELEVATION. THE CORE TRENCH OR KEYWAY SWALE SHALL BE A MINIMUM OF 2 FEET DEEP AND 8 FEET WIDE WITH SIDE SLOPE OF 1:1 OR FLATTER.
5. CONTRACTOR SHALL POSITION ANTI-SEEP COLLAR SUCH THAT THE COLLARS ARE COMPLETELY CONTAINED WITHIN THE EMBANKMENT.
6. IMMEDIATELY INSTALL FILTER FABRIC OVER ALL OUTLETS TO PREVENT SEDIMENT DEPOSITION IN THE PIPING. (REMOVE FOLLOWING AREA STABILIZATION.)



1 WET DETENTION BASIN
DETAILS



1. EXTEND MATTING 2' BELOW THE ELEVATION OF THE EMERGENCY SPILLWAY ON THE INSIDE OF THE POND.
2. EXTEND MATTING TO TOE OF SLOPE ON THE OUTSIDE OF THE POND.
3. MATTING SHALL BE INSTALLED IN ACCORDANCE WITH WDNR TECH STANDARD 1052.

2 EMERGENCY SPILLWAY

STORM WATER MANAGEMENT PLAN FOR CANYON DEVELOPMENT

WILLIAMS BAY, WI

LYNCH & ASSOCIATES
ENGINEERING CONSULTANTS, LLC

PLAN DETAILS

	NO.	REVISIONS	BY	DATE

ISSUED FOR
REVIEW

PLAN DATE	4.22.2025
-----------	-----------

DESIGNED BY BRW

0 N/A
SCALE

PROJECT NO.
23.0023

SHEET NO.

DETAILS



WILLIAMS BAY, WI

LYNCH & ASSOCIATES
ENGINEERING CONSULTANTS, LLC

NO.	REVISIONS	BY	DATE
-----	-----------	----	------

DETAILS

RESOLUTION NO. R-35-25

**FINAL RESOLUTION VACATING EAST HALF OF HANSON STREET
AND ALL OF 16.5 FOOT ALLEY SHOWN ON HANSON'S ADDITION TO WILLIAMS BAY**

WHEREAS, a petition to vacate the east half of Hanson Street and all of the 16.5 foot alley as shown on Hanson's Addition to Williams Bay having been filed with the Village of Williams Bay on April 16, 2025; and

WHEREAS, due notice of the public hearing on the adoption of the resolution vacating same has been duly given according to statute as appears more fully by the proofs of service, proof of mailing and proof of publication on file in the office of the Village Clerk; and

WHEREAS, a Lis Pendens has been filed with the office of the Register of Deeds of Walworth County, to which the petition to vacate and a copy of a map showing the location of the street and alley was attached; and

WHEREAS, the public hearing on the petition to vacate having been held before the Village Board on the 19th day of May, 2025, and it appearing that no objections in writing to said proposed vacation have been filed with the Village Board, and said matter having been discussed and it appears that the public interest requires it.

NOW, THEREFORE, the Village Board of the Village of Williams Bay do resolve as follows:

BE IT RESOLVED that the Village Board of the Village of Williams Bay, since the public interest requires it, vacates, abandons and discontinues the east half of Hanson Street and all of the 16.5 foot alley as shown on Hanson's Addition to Williams Bay in the Village of Williams Bay, Walworth County, Wisconsin described as follows:

THAT PART OF HANSON'S ADDITION TO WILLIAMS BAY, ACCORDING TO THE RECORDED PLAT, BEING THE EAST HALF OF HANSON STREET, AND ALL OF THE 16.5 FOOT ALLEY, AS SHOWN ON HANSON'S ADDITION TO WILLIAMS BAY, DESCRIBED AS FOLLOWS:

BEGINNING AT THE SOUTHWEST CORNER OF LOT 8 OF HANSON'S ADDITION TO WILLIAMS BAY, A SUBDIVISION; THENCE SOUTH 88°57'36" WEST (RECORDED AS WEST) ALONG THE SOUTH LINE OF HANSON STREET, 33.00 FEET TO THE CENTERLINE OF HANSON STREET; THENCE NORTH 00°54'29" WEST (RECORDED AS NORTH), ALONG SAID CENTERLINE, 286.33 FEET, TO A POINT ON THE SOUTH LINE OF BAYVIEW AVE; THENCE NORTH 88°58'13" EAST 33.00 FEET, TO THE NORTHWEST CORNER OF LOT 7; THENCE SOUTH 00°54'29" EAST, ALONG THE WEST LINE OF LOT 7 134.87 FEET (RECORDED AS 135 FEET), TO THE SOUTH WEST CORNER OF LOT 7; THENCE NORTH 88°57'59" EAST 448.01 FEET (RECORDED AS 448 FEET), TO THE SOUTH EAST CORNER OF LOT 1 AND THE EAST END OF ALLY; THENCE SOUTH 00°54'49" EAST (RECORDED AS SOUTH), 16.50 FEET, TO THE NORTHEAST CORNER OF LOT 14; THENCE SOUTH 88°57'59" WEST, 448.01 FEET (RECORDED AS 448 FEET), TO THE NORTHWEST CORNER OF LOT 8; THENCE SOUTH 00°54'29" EAST 134.89 FEET (RECORDED AS 135 FEET) TO THE SOUTHWEST CORNER OF LOT 8 AND THE POINT OF BEGINNING, ALL IN THE NORTHEAST ¼ OF THE NORTHEAST ¼ OF SECTION 6 TOWNSHIP 1 NORTH, RANGE 17 EAST OF THE FOURTH PRINCIPAL MERIDIAN, VILLAGE OF WILLIAMS BAY, WALWORTH COUNTY, WISCONSIN.

TAX ID: WHA00001-00006

BE IT FURTHER RESOLVED that the map attached to the petition to vacate and Lis Pendens be further attached to this final resolution and that all proofs of service and mailing and publication of the notice of this proceeding incorporated herein and made a part thereof.

BE IT FURTHER RESOLVED that the above-described portions of Hanson Street and the 16.5 foot alley which have been vacated, abandoned and discontinued be attached are the parcels adjoining thereto as provided by law provided that the east one-half of Hanson Street be attached to the parcels adjoining thereto to the east of said street.

BE IT FURTHER RESOLVED that the Village Clerk certify a copy of this resolution and map and cause the same be recorded in the office of the Register of Deeds for Walworth County, Wisconsin.

CERTIFICATION

I, Tina Kolls, Clerk of the Village of Williams Bay, a municipal corporation organized and existing under the laws of the State of Wisconsin, do hereby certify that the foregoing is a true and correct copy of the resolution of the Village Board of the Village of Williams Bay adopted at a duly convened meeting of said Village Board held at the Village of Williams Bay, Walworth County, Wisconsin on the 19th day of May, 2025 as taken by me from the minutes of said meeting and compared to the original said resolution reported in said minutes.

IN WITNESS WHEREOF I have set my hand this _____ day of May, 2025.

Tina Kolls, Clerk

AUTHENTICATION

Signature of Tina Kolls
Authenticated on _____, 2025.

Mark A. Schroeder, Village Attorney
State Bar Number: 1000061

This Instrument was Drafted by:

Attorney Mark A. Schroeder
State Bar Number: 1000061
Consigny Law Firm, S.C.
303 East Court Street
Janesville, WI 53545
(608) 755-5050